## CE 544 Open Channel Hydraulics - Solution Mid-term, Winter 2015

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- $\checkmark\,$  You will have 50 minutes to complete the exam. The exam is closed book, closed notes and open mind.
- ✓ The procedure will be graded. Please justify your answers

1. (25 points) A trapezoidal channel is to be designed to carry a discharge of 75 m<sup>3</sup>/s at **maximum hydraulic efficiency**. The side slopes of the channel are 2H:1V and the Manning's roughness n is 0.030.

- a. If the maximum allowable velocity in the channel is 1.75m/s, what should be the dimensions of the channel (bottom width and height)?
- b. What should be the longitudinal slope of the channel if the flow is uniform?

a)  

$$A = Q/V = \frac{75}{1.75} = 42.85 \text{ Tm}^{2}$$

$$b = 2y (\sqrt{1+z^{2}} - z) = 0.4724$$

$$A = by + zy^{2} = 0.472y^{2} + zy^{2} = 2.472y^{2}$$

$$A = 42.86 = 2.472y^{2}$$

$$y^{2} = 17.836 \rightarrow y = 4.16m \Rightarrow b = 0.472y = 1.96m$$

$$b) P = b + 2y\sqrt{1+z^{2}} = 1.96 + z(4.16)\sqrt{1+z^{2}} = 20.56m$$

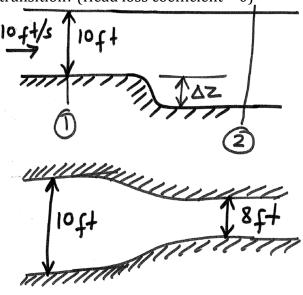
$$R = A/p = 42.86/20.56 = 2.08 \text{ M}$$

$$V = 1.75 = \frac{1}{0.030}(2.08)^{2/3} \text{ S}_{0}^{1/2}$$

$$S_{0} = 0.00104 \text{ m/m}.$$

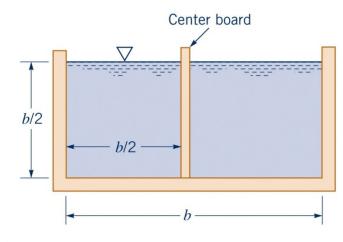
$$Q = \frac{k}{n} AR^{-5/2} = CA^{-5/3} A = by + 7y^{2}$$
Hydraulic efficiency:  $P = b + 2y\sqrt{1+7^{2}}$   
() for a given A, Qmax ? they are equivalent  
() for a given Q, min A. ] and give the same  
In our case: Q is given So A needs to be  
 $\frac{2}{3}$   $\frac{1}{3}$   $\frac{1}{3}$   $\frac{1}{3}$   $\frac{1}{3}$   $\frac{1}{3}$   
P Q = CA  
if Q is constant, A will be minimum if  
P is minimum.  
Amin  $(\frac{dA}{dy} = 0) \rightarrow \frac{db}{dy} = -\frac{b}{27} - 27$   
P min  $(\frac{dF}{dy} = 0) \rightarrow \frac{db}{dy} + 2\sqrt{1+7^{2}} = 0$   
() in (2)  $\Rightarrow -\frac{b}{y} - 27 + 2\sqrt{1+7^{2}} = 0$   
 $\frac{b}{y} = 0.44 + 1$ 

2. (25 points) A 10-ft wide rectangular channel is flowing at a depth of 10-ft with a velocity of 10 ft/s. If the channel has a smooth contraction in width from 10 ft to 8 ft, how much should the channel bottom drop to maintain a constant water surface elevation through the transition? (Head loss coefficient = 0)



$$\begin{array}{l} 9_{1} = 100 \ \frac{ft^{2}}{5}, \ y_{c} = 6.77 \ ft \end{array} (2) \\ E_{1} = 11.55 \ ft \\ Q_{1} = 100 \times 10 = 1000 \ ft^{3}/s \\ Q_{1} = Q_{2} \\ Q_{1} = Q_{2} \\ 1000 = 9_{2}b_{2} = 9_{2} \times 8 \\ (9_{2} = 125 \ ft^{2}/s) \\ \frac{y_{c_{2}}}{2} = \left(\frac{9_{2}}{9}\right)^{1/3} = \left(\frac{125^{2}}{32.2}\right)^{1/3} \\ \frac{y_{c_{2}}}{2} = \frac{9_{2}}{7.86} \ ft \\ Energy \ equetion \ between \ 0 \ and \ (2) \\ E_{1} = y_{2} + \frac{9_{2}^{2}}{9} - \Delta Z \\ \frac{29y^{2}}{2x32.2} \ y_{2}^{2} \\ 1.55 = \frac{242.624}{y_{2}^{2}} \\ \frac{y_{2}}{2} = 12.51 \ ft \\ \Delta z = y_{2} - 10 = 2.51 \ ft \end{array}$$

3. (25 points) By what percent is the flowrate reduced in the rectangular channel shown in figure below because of the addition of the thin center board? All surfaces are of the same material.



$$Q = \frac{k}{n} A R_h^{2_3} S_h^{1_2}$$

Without the center board s

$$A = b\left(\frac{b}{2}\right) = \frac{b^2}{2} ; R_h = \frac{A}{p} = \frac{\frac{b}{2}}{2b} = \frac{b}{4}$$
$$= D \quad Q_{\text{withoutcenter-board}} = \frac{K}{h} \left(\frac{b^2}{2}\right) \left(\frac{b}{4}\right)^{\frac{2}{3}} S_0^{\frac{1}{2}} \qquad \text{Eq. (1)}$$

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With the center board &

$$\begin{aligned} & \left( \begin{array}{l} Q_{\text{with.conter-board}} = 2 \left( \begin{array}{l} Q_{2} \right), \text{ where } A_{2} = \left( \begin{array}{l} \frac{b}{2} \right)^{2} \\ Rh_{2} = \frac{A_{2}}{P_{2}} = \frac{\left( \begin{array}{l} \frac{b}{2} \right)^{2}}{3 \left( \begin{array}{l} \frac{b}{2} \right)^{2}} = \frac{b}{6} \\ = 0 \quad Q_{\text{with.conter-board}} = 2 \left[ \begin{array}{l} \frac{K}{P_{1}} \left( \begin{array}{l} \frac{b}{2} \right)^{2} \left( \begin{array}{l} \frac{b}{2} \right)^{2} \gamma_{3}} S_{0}^{\frac{1}{2}} \right] \\ Devide \quad Eq.(2) \quad by \quad Eq.(1) \quad to \quad obtain \quad \frac{Q_{\text{with.conter-board}}}{Q_{\text{with.outl.conterboard}}} = \frac{2 \left( \begin{array}{l} \frac{b}{2} \right)^{2} \left( \begin{array}{l} \frac{b}{2} \right)^{2} \gamma_{3}} \\ \frac{C}{2} \right) \left( \begin{array}{l} \frac{b}{2} \right)^{2} \left( \begin{array}{l} \frac{b}{2} \right)^{2} \gamma_{3}} \\ \frac{C}{2} \right) \left( \begin{array}{l} \frac{b}{2} \right)^{2} \left( \begin{array}{l} \frac{b}{2} \right)^{2} \gamma_{3}} \\ \frac{C}{2} \right) \left( \begin{array}{l} \frac{b}{2} \right)^{2} \left( \begin{array}{l} \frac{b}{2} \right)^{2} \gamma_{3}} \\ \frac{C}{2} \right) \left( \begin{array}{l} \frac{b}{2} \right) \left( \begin{array}{l} \frac{b}{2} \right)^{2} \gamma_{3}} \\ \frac{C}{2} \right) \left( \begin{array}{l} \frac{b}{2} \right)^{2} \gamma_{3}} \\ \frac{C}{2} \right) \left( \begin{array}{l} \frac{b}{2} \right)^{2} \gamma_{3}} \\ \frac{C}{2} \right) \left( \begin{array}{l} \frac{b}{2} \right) \left( \begin{array}{l} \frac{b}{2} \right)^{2} \gamma_{3}} \\ \frac{C}{2} \right) \left( \begin{array}{l} \frac{b}{2} \right) \left( \begin{array}{l} \frac{b}{2} \right)^{2} \gamma_{3}} \\ \frac{C}{2} \right) \left( \begin{array}{l} \frac{b}{2} \right)^{2} \gamma_{3}} \\ \frac{C}{2} \right) \left( \begin{array}{l} \frac{b}{2} \right) \left( \begin{array}{l}$$

4. (25 points) A hydraulic jump at the base of a spillway of a dam is such that the depths upstream and downstream of the jump are 0.90 and 3.6 m, respectively. If the spillway is 10 m wide, what is the flowrate over the spillway?