

Elasticity - deals with actions of body under applied loads. we assume that when loads are removed, body returns to its original shape without hysteresis.

[Strength & materials assumption: plane sections remain plane and are $\perp$ to $\vec{E}$ of beam
its a 1-D analysis - we don't take into account contraction in the $\perp$ to the plane of motion

Assume $\lim_{\delta A \rightarrow 0} \frac{\delta \vec{P}}{\delta A} = \vec{t}_n$ $\lim_{\delta A \rightarrow 0} \frac{\delta \vec{M}}{\delta A} = 0$ $\vec{M}$ is the moment vector

where $\vec{t}_n$ is the traction on the surface whose normal is $\vec{n}$. State of stress at a point is said to be known if traction on every plane passes through that point is known

$$\vec{t}_j = \sigma_{ji} \vec{e}_i \quad \vec{\sigma} = \sigma_{ji} \vec{e}_i \vec{e}_j \quad \therefore \vec{\sigma} = \sigma_{ji} \vec{e}_j \vec{e}_i$$

tractions on body where normal is $\vec{n}$

P.L.M. $\sum$ forces acting on body = the time rate of change of momentum $\int_S \vec{t}_n dS + \int_V \vec{f} dV = \int_V \rho \frac{\partial^2 \vec{u}}{\partial t^2} dV$

P.A.M. $\sum$ moments about some point = " " " of angular momentum $\int_S \vec{r} \times \vec{t}_n dS + \int_V \vec{r} \times \vec{f} dV = \int_V \vec{r} \times \rho \frac{\partial^2 \vec{u}}{\partial t^2} dV$

P.L.M. $\nabla \cdot \vec{\sigma} + \vec{f} = \rho \frac{\partial^2 \vec{u}}{\partial t^2}$ P.A.M. $\sigma_{ij} = \sigma_{ji}$

Compatibility - depends on geometry of system; $\nabla \times \vec{\epsilon} \times \nabla = 0$ pointwise and $\oint$ relation over multiply connected regions "holes" for compatibility

(1) If stresses are given must satisfy compat. to get unique displ. field (to up to rigid body motion). (2) If displ. are given, compat are uniquely satisfied.

Constitutive Eqs needed: since ϵ_{ij} (6 unknowns) are fns of u_i (3 indep variables), must get 3 more eqns. Use conserv of energy.

$$\int_S \vec{t}_n \cdot \vec{v} dS + \int_V \vec{f} \cdot \vec{v} dV = \frac{D}{Dt} \left(\int_V \frac{1}{2} \rho \vec{v} \cdot \vec{v} dV + \int_V \rho U_0 dV \right) \quad \vec{v} = \frac{\partial \vec{u}}{\partial t}$$

surface traction work volume work KE internal energy/unit mass

using P.L.M. then we get that $\int_V (\vec{\sigma} : \dot{\vec{\epsilon}} - \dot{U}) dV = 0$ or $\int_V (\sigma_{ij} \frac{\partial \epsilon_{kj}}{\partial t} - \frac{\partial U}{\partial t}) dV = 0$ $U = \rho U_0$

postulate $U = U(\vec{\epsilon})$ only $\therefore \frac{\partial U}{\partial t} = \frac{\partial U}{\partial \epsilon} \frac{\partial \epsilon}{\partial t} = \frac{\partial U}{\partial \epsilon_{ij}} \frac{\partial \epsilon_{ij}}{\partial t}$ $\therefore \frac{\partial U}{\partial \epsilon} = \vec{\sigma}$

also $U(\vec{\epsilon}) = U(\vec{0}) + U'(\vec{0}) \cdot \vec{\epsilon} + \frac{1}{2} U''(\vec{0}) : \vec{\epsilon} : \vec{\epsilon} + \dots$ $U(\vec{\epsilon}) = \frac{1}{2} c_{ijkl} \epsilon_{ij} \epsilon_{kl}$

reference state takes as 0 $\Rightarrow$ residual stresses $\exists$ when in equil state unless we take as 0 produce non-linear terms

$\therefore U(\vec{\epsilon}) = \frac{1}{2} c_{ijkl} \epsilon_{ij} \epsilon_{kl}$ or $\sigma_{ij} = c_{ijkl} \epsilon_{kl}$ where $\frac{\partial U}{\partial \epsilon_{ij}} = \sigma_{ij}$ $i, j = 1, 2, 3$

$c_{ijkl} = 81 \text{ const}$ since $U = c_{ijkl} \epsilon_{ij} \epsilon_{kl} = c_{klij} \epsilon_{kl} \epsilon_{ij} = c_{klij} \epsilon_{ij} \epsilon_{kl} \Rightarrow \epsilon_{ijkl} = c_{klij}$
 also since $\epsilon_{ij} = \epsilon_{ji} \Rightarrow c_{ijkl} = c_{jikl} = c_{klij} = c_{klji} \Rightarrow 21 \text{ independent constants for general material}$

can reduce to 2 independent material constants for isotropic material

Bulk Mod $K = \frac{3\lambda + 2\mu}{3} = \frac{\sigma_{kk}}{3 \epsilon_{kk}} = \frac{\text{average pressure}}{\text{unit volume change}}$

average pressure needed to cause unit volume change.

Beltrami condition is the stress equiv of ^{compatibility} ~~biharmonic~~ $\nabla^2 \phi = \nabla^2 (\sigma_{kk}) = 0$ or $\nabla^4 \sigma_{ij} = 0$
 if no body force or accel.

Coulomb Torsion Assume displ field $u_r = 0, u_z = 0, u_\theta = \alpha r z$ $\alpha = \text{twist}$

" torsion is end loaded.

St. Venant Principle - application of load produces local effect; far from pts of application
 two systems with equivalent static loads will have same stress field

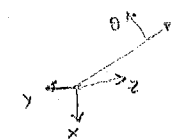
St Venant Torsion assume end loaded stress field $\sigma_{xz}, \sigma_{yz} \neq 0$ all others = 0

use equil $\Rightarrow$ define $\phi \Rightarrow$ use BC on surface to show $\phi = \text{const}$ on body $\Rightarrow F_x, F_y = 0$ on end, ^{each cavity}

Compatibility gives $\nabla^2 \phi = -2\mu\alpha$ $\alpha = \text{is the twist}$

$\nabla^2 = \frac{\partial^2}{\partial x^2} + \frac{\partial^2}{\partial y^2}$

for cavities we also have $\oint_{C_i} \frac{\partial \phi}{\partial n} ds = 2\mu\alpha A_i$ $A_i = \text{area of cavity enclosed by } C_i$
 $\mu = \text{rigidity}$

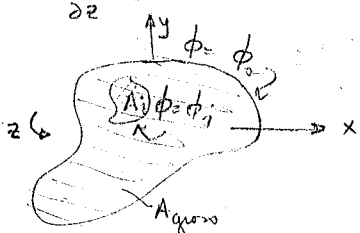


$u_x = -\alpha y z$ $u_y = \alpha x z$ $u_z = f(x, y)$

$u_r = -u_x \sin \theta + u_y \cos \theta$ $y = r \cos \theta$ $x = r \sin \theta$ $u_x = \alpha r \cos \theta z$ $u_y = -\alpha r \sin \theta z$ $\Rightarrow u_r = 0$

also $\frac{\partial u_z}{\partial x} = \frac{1}{\mu} \frac{\partial \phi}{\partial y} + \alpha y$ $\frac{\partial u_z}{\partial y} = -\frac{1}{\mu} \frac{\partial \phi}{\partial x} + \alpha x$ $\frac{\partial u_z}{\partial z} = 0$
 $u_\theta = -u_x \cos \theta + u_y \sin \theta = -\alpha r \cos^2 \theta z + \alpha r \sin^2 \theta z = -\alpha r z$

also $T = 2 \left[\iint \phi dA - \phi_0 A_{gross} + \sum \phi_i A_i \right]$
 $= \mu\alpha J$ $J = \text{polar moment of inertia of beam about z-axis}$



ME238A

PRINCIPAL STRESSES
ARE ALWAYS OF THE
FORM.

$$\begin{pmatrix} \sigma_1 & 0 & 0 \\ 0 & \sigma_2 & 0 \\ 0 & 0 & \sigma_3 \end{pmatrix}$$

shear = 0 on planes $\perp$ to principal directions

Formulas

$$t_i = \sigma_{ij} e_j \quad \text{where } \sigma_{ij} = \sigma_{ji} \quad t_i = e_i \cdot \sigma \quad \sigma = \sigma_{ij} e_i e_j \quad \text{tensor}$$

STRESS ON FACE WHOSE NORMAL IS n

$$\sigma = e_i t_i = \sigma_{ij} e_i e_j$$

$$n \frac{\partial \phi}{\partial n} = \nabla \phi \quad \text{or} \quad \frac{\partial \phi}{\partial n} = n \cdot \nabla \phi$$

$$\text{RATE OF CHANGE OF } \phi \text{ WRT } n \quad // \quad \frac{\partial \phi}{\partial x_k} = \nabla \phi \cdot e_k$$

$$\nabla \cdot \sigma = \nabla \cdot (e_j t_j) = \frac{\partial}{\partial x_j} t_j = \sigma_{jk,j} e_k$$

$$du = dr \cdot \nabla u \quad \text{and} \quad d\phi = dr \cdot \nabla \phi$$

$$\text{PLM} \quad \nabla \cdot \sigma + f = \rho \frac{\partial^2 u}{\partial t^2} \quad \text{or} \quad \sigma_{jk,j} + f_k = \rho \frac{\partial^2 u_k}{\partial t^2}$$

$$\sigma' = e'_m \sigma_{ij} e'_j \cdot e'_n$$

$$\sigma'_{mn} = \sigma_{ij} l_{mi} l_{nj} \quad \text{where } l_{mi} = e'_m \cdot e_i$$

STRESS TENSOR IN NEW COORD SYSTEM $l_{mi} \neq l_{im}$

INVARIANTS IN TERMS OF GENERAL STRESSES FROM $-\lambda^3 + I\lambda^2 - II\lambda + III = 0$ FOR PRINCIPALS $-\lambda^3 + I'\lambda^2 - II'\lambda + III' = 0$

$$I = \sigma_{kk} = \sigma_{xx} + \sigma_{yy} + \sigma_{zz}$$

$$I' = I = \sigma_1 + \sigma_2 + \sigma_3$$

$$II = \frac{1}{2} e_{mik} e_{njl} \sigma_{ij} \sigma_{kl} \delta_{mn} = \sigma_{xx}\sigma_{yy} + \sigma_{yy}\sigma_{zz} + \sigma_{zz}\sigma_{xx} - (\sigma_{xy}^2 + \sigma_{yz}^2 + \sigma_{zx}^2) \quad II' = II = \sigma_1\sigma_2 + \sigma_2\sigma_3 + \sigma_3\sigma_1$$

$$III = \frac{1}{6} e_{mik} e_{njl} \sigma_{ij} \sigma_{kl} \sigma_{mn} = \sigma_{xx}\sigma_{yy}\sigma_{zz} + 2\sigma_{xy}\sigma_{yz}\sigma_{zx} - (\sigma_{xx}\sigma_{yz}^2 + \sigma_{yy}\sigma_{xz}^2 + \sigma_{zz}\sigma_{xy}^2) \quad III' = III = \sigma_1\sigma_2\sigma_3$$

TO FIND $\lambda^{(k)}$ solve the eigenvalue equation and look at back of pg 8 notes for method to find eigenvectors (ie principal directions)

$$(\nabla u)_c = u_i \nabla = e_k e_j \frac{\partial u_k}{\partial x_j} \quad \nabla u_i = e_j e_k \frac{\partial u_k}{\partial x_j} \quad \text{and} \quad \hat{I} = \nabla u_i = (\delta_{ij} - \frac{\partial u_j}{\partial x_i}) e_i e_j$$

$$dir_o = dir \cdot [I - \nabla u] = [I - u_i \nabla] \cdot dir$$

EULERIAN DEFORMATION TENSOR of new

$$dir = dir_o \cdot [I + \nabla_o u] = [I + u_i \nabla_o] \cdot dir_o$$

LAGRANGIAN " " new length in terms of old

$$\eta_{LL} = \frac{L^2 - L_o^2}{2L_o^2} \quad \text{EULERIAN STRETCH}$$

$$\epsilon_{LL} = \frac{L^2 - L_o^2}{2L_o^2} \quad \text{LAGRANGIAN STRETCH} \quad \eta_{LL} \rightarrow \epsilon_{LL} \rightarrow \epsilon = \frac{\Delta L}{L} \text{ for small strains}$$

$$\eta_{rr} = n \cdot \Phi \cdot n \quad n = \frac{dir}{|dir|}$$

$$\Phi = \frac{1}{2} (\nabla u_i + u_i \nabla - \nabla u_i \cdot u_i \nabla) = \frac{1}{2} \left(\frac{\partial u_j}{\partial x_i} + \frac{\partial u_i}{\partial x_j} - \frac{\partial u_k}{\partial x_i} \frac{\partial u_k}{\partial x_j} \right) = \phi_{ij} e_i e_j$$

EULERIAN STRAIN TENSOR $\phi_{ij} = \phi_{ji}$ contains rotations & extensions

$$\frac{1}{2} \cos \theta = n \cdot \Phi \cdot \$ \quad \text{where } n = \frac{dir'}{|dir'|} \quad \$ = \frac{dir}{|dir|}$$

$$\text{where } n = \frac{dir'}{|dir'|} \quad \$ = \frac{dir}{|dir|}$$

$$n \cdot \Phi \cdot \$ = \frac{1}{2} \frac{dir' \cdot dir}{|dir'| |dir|}$$

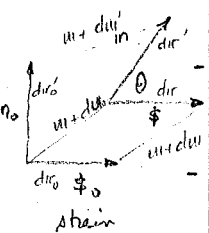
$$\epsilon_{rr} = n_o \cdot \Phi \cdot n_o$$

$$\Phi = \frac{1}{2} [\nabla_o u_i + u_i \nabla_o + \nabla_o u_i \cdot u_i \nabla_o] \quad \text{LAGRANGIAN STRAIN TENSOR} \quad n_o = \frac{du}{|du|}$$

$$\frac{1}{2} \cos \theta = \frac{n_o \cdot \Phi \cdot \$}{\sqrt{1+2\epsilon_{s_o s_o}} \sqrt{1+2\epsilon_{n_o n_o}}} = \frac{du' \cdot du}{2 |du'| |du|}$$

$$= \frac{1}{2} \left[\frac{\partial u_j}{\partial x_{o i}} + \frac{\partial u_i}{\partial x_{o j}} + \frac{\partial u_k}{\partial x_{o i}} \frac{\partial u_k}{\partial x_{o j}} \right] = \epsilon_{ij}$$

relates undeformed state to deformed state



deformation

for small extensions / deformations then $\bar{\Phi} \rightarrow \Phi \rightarrow \mathbb{E}$ where $\mathbb{E} + \omega = \nabla_0 u$, $\mathbb{E} - \omega = u_1 \nabla_0$

Dilatation $\Delta = \frac{\partial u_x}{\partial x} + \frac{\partial u_y}{\partial y} + \frac{\partial u_z}{\partial z} = u_{k,k} = \epsilon_{kk} = I_\epsilon = \nabla \cdot u$

$\mathbb{E} = \frac{1}{2} (\nabla u + u_1 \nabla) = \epsilon_{ij} e_i e_j = \frac{1}{2} (u_{i,j} + u_{j,i}) e_i e_j$; $\omega_{ij} = \frac{1}{2} (u_{i,j} - u_{j,i})$ $\omega = \omega_{ij} e_i e_j$ $\nabla u = \mathbb{E} + \omega$

Incompatibility tensor $\Psi = \nabla \times \Phi \times \nabla$ should be 0 (Eqs. Pg 14) pointwise compatibility

For multiply connected regions: $\oint_{C_i} \text{div} \cdot (\Phi \times \nabla) = 0$ and $\oint_{C_i} [\Phi + (1r - 1r_0) \times (\nabla \times \Phi)] \cdot dr = 0$

assume $U(\epsilon) = U_0 + \frac{\partial U}{\partial \epsilon} \Delta \epsilon + \frac{\partial^2 U}{\partial \epsilon^2} \frac{\Delta \epsilon^2}{2} + \dots$ only
init state = 0 otherwise imply prestressed state, if no stresses

obtained from conservation of energy + PLM $\Rightarrow$

$-\frac{\partial U}{\partial t} = \sigma_{kj} \frac{\partial \epsilon_{kj}}{\partial t}$ where $U = \text{internal energy / volume}$ or $\dot{U} = \sigma : \dot{\epsilon}$ $\frac{\partial U}{\partial t} = \frac{\partial U}{\partial \epsilon_{ij}} \frac{\partial \epsilon_{ij}}{\partial t}$
if $U = U(\epsilon)$ only $\sigma_{ii} = \frac{\partial U}{\partial \epsilon_{ii}}$ (no summation) $\sigma_{ij} = \frac{1}{2} \frac{\partial U}{\partial \epsilon_{ij}}$ $i \neq j$
and $U = \frac{1}{2} c_{ijkl} \epsilon_{ij} \epsilon_{kl} > 0$ $\sigma_{ii} = \frac{\partial U}{\partial \epsilon_{ii}}$

Remember that $2\epsilon_{ij} = u_{i,j} + u_{j,i}$

for isotropic material $\sigma_{ij} = \lambda \epsilon_{kk} \delta_{ij} + 2\mu \epsilon_{ij}$ $\frac{E}{2(1+\nu)} = G = \mu$ the shear modulus of elast.

$\frac{E\nu}{(1+\nu)(1-2\nu)} = \lambda$
 $\epsilon_{ij} = \frac{1+\nu}{E} \sigma_{ij} - \frac{\nu}{E} \sigma_{kk} \delta_{ij} = \frac{1}{2\mu} \left[\sigma_{ij} - \frac{\lambda}{3\lambda+2\mu} \sigma_{kk} \delta_{ij} \right]$

See pg 18-19

For unique soln we need that from specified init cond. that $\epsilon_0 = 0$

$t_j^* = 0$ from body forces specified * is difference soln

$\int_{S_d} t_j^* \Delta u_j^* dS dt = 0$ compatibility must be specified for discontinuities in displ field

$\int_{S_v} t_j^* u_j^* dS dt = 0$ must specify singularities

must specify how $\int t_j^* u_j^* dS dt = 0$ ie $t_j^* = 0$ or $u_j^* = 0$ or combination of $t_j^*, u_j^* = 0$

if t_j are specified solution is specified up to rigid body effects for static problems

if t_j " " " is unique for dynamic case

now $\sigma_{ij} = \lambda u_{k,k} \delta_{ij} + \mu (u_{i,j} + u_{j,i})$

$\nabla \times \Phi \times \nabla = \epsilon_{jls} \epsilon_{skl} + \epsilon_{skl} \epsilon_{jls} - \epsilon_{skl} \epsilon_{jls} - \epsilon_{jls} \epsilon_{skl} = 0$

$\nabla \times \Phi \times \nabla \Rightarrow$ Beltrami-Michell Conditions on stresses.

Gwin $\Theta = \sigma_{kk}$

$\nabla^2 \sigma_{ij} + \frac{1}{1+\nu} \Theta_{,ij} = 0$ for constant body force & no accel

Pg 20 full dynamic eqs

Beltrami

when work w/ body force & accel we work w/ Beltrami-Michell

also $\nabla^2 \Theta = 0$ where Θ is harmonic

also $\nabla^4 \sigma_{ij} = 0$ equivalent to $\nabla \times \mathbf{d} \times \nabla = 0$

In orthog curvilinear $ds_{\alpha_i} = h_{\alpha_i} d\alpha_i$ where $h_{\alpha_i} = \sqrt{\sum_j \frac{\partial x_j^2}{\partial \alpha_i^2}}$

$$\nabla = \sum_i \frac{1}{h_{\alpha_i}} \mathbf{e}_{\alpha_i} \frac{\partial}{\partial \alpha_i} \quad \text{and } u_i = u_{\alpha_i} \mathbf{e}_{\alpha_i}$$

$$\text{and } \epsilon_{\alpha\alpha_i} = \mathbf{e}_{\alpha} \cdot \mathbf{d} \cdot \mathbf{e}_{\alpha_i} = \frac{1}{h_{\alpha}} \left[\frac{\partial u_{\alpha}}{\partial \alpha} + \frac{u_{\beta}}{h_{\beta}} \frac{\partial h_{\alpha}}{\partial \beta} + \frac{u_{\gamma}}{h_{\gamma}} \frac{\partial h_{\alpha}}{\partial \gamma} \right]$$

$$\epsilon_{\alpha\beta} = \mathbf{e}_{\alpha} \cdot \mathbf{d} \cdot \mathbf{e}_{\beta} = \frac{1}{2} \left[\frac{h_{\beta}}{h_{\alpha}} \frac{\partial}{\partial \alpha} \left(\frac{u_{\beta}}{h_{\beta}} \right) + \frac{h_{\alpha}}{h_{\beta}} \frac{\partial}{\partial \beta} \left(\frac{u_{\alpha}}{h_{\alpha}} \right) \right]$$

$$\Delta = \frac{1}{h_{\alpha} h_{\beta} h_{\gamma}} \left\{ \frac{\partial}{\partial \alpha} (u_{\alpha} h_{\beta} h_{\gamma}) + \frac{\partial}{\partial \beta} (u_{\beta} h_{\alpha} h_{\gamma}) + \frac{\partial}{\partial \gamma} (u_{\gamma} h_{\alpha} h_{\beta}) \right\} = \nabla \cdot \mathbf{u}$$

$$\nabla^2 \sigma_{ij} + \frac{1}{1+\nu} \Theta_{,ij} - \frac{\nu}{1+\nu} \delta_{ij} \nabla^2 \Theta = - (f_{i,j} + f_{j,i}) + \rho (\ddot{u}_{i,j} + \ddot{u}_{j,i})$$

$$- \frac{\nu}{1-\nu} f_{k,k} \delta_{ij} - (f_{i,j} + f_{j,i}) - \frac{\rho}{E} \left(\frac{\nu}{1-\nu} \ddot{\Theta} \delta_{ij} - 2(1+\nu) \ddot{\sigma}_{ij} \right)$$

Aside

$$- \frac{\nu}{1-\nu} \delta_{ij} f_{k,k} + \frac{\rho \nu}{1-\nu} \delta_{ij} \ddot{u}_{k,k} - (f_{i,j} + f_{j,i}) + \rho (\ddot{u}_{i,j} + \ddot{u}_{j,i})$$

$$- \frac{\nu}{1+\nu} \delta_{ij} \left[\frac{\nu}{1+\nu} \nabla^2 \Theta + \frac{\nu}{1-\nu} f_{k,k} - \frac{\rho \nu}{1-\nu} \ddot{u}_{k,k} \right] = 0$$

B.C. can specify either tractions and/or displ but cannot specify T_x & u_x together uniqueness
assumes finite volume
and no singularities

Since we are using linear elastic theory we can superpose solns.

** Remember always satisfy BC, Equil Eqn & Compatibility

Equil $\sigma_{ij,i} + f_j = 0$

BC $\sigma_{ij} n_j = T_i$ and/or u_i is prescribed n_j is the normal outward

Compat: $\nabla \times \epsilon \times \nabla = 0$ for simply connected region LINEAR ELASTICITY

$$\epsilon_{ij} = \frac{(1+\nu)}{E} \sigma_{ij} - \frac{\nu}{E} \sigma_{kk} \delta_{ij} \quad \epsilon_{ij} = \frac{1}{2} (u_{j,i} + u_{i,j}) \quad \omega_{ij} = \frac{1}{2} (u_{j,i} - u_{i,j})$$

PLANE STRAIN $\sigma_{ij} = \lambda \delta_{ij} \epsilon_{kk} + 2\mu \epsilon_{ij}$ only for isotropic; normally $\sigma_{ij} = C_{ijkl} \epsilon_{kl}$

$$\begin{aligned} \epsilon_{zz} = 0 \quad \sigma_{zz} = \nu (\sigma_{xx} + \sigma_{yy}) \\ \epsilon_{zx} = \epsilon_{zy} = 0 \quad u_z = 0 \\ u_x(x,y), u_y(x,y) \text{ only} \\ \sigma_{xx}, \sigma_{xy}, \sigma_{yz} \neq f(z) \quad f_1, f_2 \neq f(z) \\ \epsilon_{xx}, \epsilon_{yy}, \epsilon_{xy} \neq f(z) \quad f_3 = 0 \end{aligned} \quad \left\| \quad \begin{aligned} \epsilon_{xx} &= \frac{(1+\nu)}{E} [(1-\nu) \sigma_{xx} - \nu \sigma_{yy}] = \frac{1-\nu^2}{E} \sigma_{xx} - \frac{\nu(1+\nu)}{E} \sigma_{yy} \\ \epsilon_{yy} &= \frac{(1+\nu)}{E} [(1-\nu) \sigma_{yy} - \nu \sigma_{xx}] = \frac{1-\nu^2}{E} \sigma_{yy} - \frac{\nu(1+\nu)}{E} \sigma_{xx} \\ \epsilon_{xy} &= \frac{1}{2\mu} \sigma_{xy} = \frac{1+\nu}{E} \sigma_{xy} \end{aligned}$$

PLANE STRESS

$$\begin{aligned} \sigma_{zz} = 0, \sigma_{xz} = 0, \sigma_{yz} = 0 \\ f_3 = 0 \text{ to satisfy equil} \end{aligned} \quad \left\{ \begin{aligned} u_x(x,y), u_y(x,y) \text{ only} \\ \sigma_{xx}, \sigma_{xy}, \sigma_{yy}, f_1, f_2 \neq f(z) \end{aligned} \right. \quad \text{if very thin} \quad \text{NOT EXACT USUALLY}$$

Define $\phi(x,y)$ s.t. $\sigma_{xx} = \frac{\partial^2 \phi}{\partial y^2}$ $\sigma_{xy} = -\frac{\partial^2 \phi}{\partial x \partial y}$ $\sigma_{yy} = \frac{\partial^2 \phi}{\partial x^2}$ satisfies equil. To satisfy compatibility we get $\nabla^4 \phi = 0 = \nabla^2 \epsilon_{kk} = \nabla^2 \sigma_{kk} = 0$

if body force is included then $\sigma_{xx} = \frac{\partial^2 \phi}{\partial y^2} + V$ $\sigma_{yy} = \frac{\partial^2 \phi}{\partial x^2} + V$ when $f_j = -\frac{\partial V}{\partial x_j}$ $V = \text{potential fn}$
for weight force take $V = \pm \rho g y$ (sign depends on defn of axes) and let $\phi = 0$ also $\nabla^2 V = 0 \uparrow$
this solution is good for either plane strain or plane stress.

Gwin: Plane Stress soln replace ν by $\frac{\nu}{1-\nu}$ to get plane strain E_σ by $\frac{E}{1-\nu^2}$
Plane Strain " " ν_E by $\frac{\nu}{1+\nu}$ to get plane stress E_E by $E \left(\frac{1+\nu}{1-\nu^2} \right)$

2-D semi infinite plane w/ periodic b.c. & $|\sigma_{ij}| \rightarrow 0$ as $y \rightarrow -\infty$ SEE PG 4 my notes

a) if $\sigma_{xy} = 0$ $\sigma_{yy} = f(x) = \sum A_n \sin \frac{n\pi x}{L}$ $A_n = \frac{2}{L} \int_0^L f(x) \sin \frac{n\pi x}{L} dx$ take $\phi_n = g(y) \sin \gamma_n x$
and let $\sum \phi_n = \phi(x,y)$ where $\gamma_n = \frac{n\pi}{L}$. If $e^{\gamma y} = g(y)$ then $s = \pm \gamma_n$

Solve and get $\sigma_{xx}, \sigma_{xy}, \sigma_{yy}$ now $u_i = \int \frac{\partial u_i}{\partial x_i} dx_i + f(x_j)$ only (ODD FUNC)

b) if $\sigma_{xy} = 0$ $\sigma_{yy} = f_2(x) = \sum B_n \cos \frac{n\pi x}{L} + \frac{B_0}{2}$ $B_n = \frac{2}{L} \int_0^L f_2(x) \cos \frac{n\pi x}{L} dx$ take $\phi_n = g(y) \cos \gamma_n x$ (EVEN FUNC)
if $f(x) = f_1 + f_2$ then $\phi = \frac{B_0 x^2}{4} + \sum g_1(y) \sin \frac{n\pi x}{L} + \sum g_2(y) \cos \frac{n\pi x}{L}$ (ANY FUNC)

c) if $\sigma_{xy} = g(x) = \frac{D_0}{2} + \sum D_n \cos \frac{n\pi x}{L} + E_n \sin \frac{n\pi x}{L}$ take $\phi_n = -\frac{xy}{2} D_0 + \sum g_1(y) \sin \frac{n\pi x}{L} + g_2(y) \cos \frac{n\pi x}{L}$
 $\sigma_{yy} = 0$

ie if $\sigma_{xy} = g_1(x) = \frac{D_0}{2} + \sum D_n \cos \frac{n\pi x}{L}$ take $\phi_n = -\frac{xy}{2} D_0 + \sum g_1(y) \sin \frac{n\pi x}{L}$
 $\sigma_{xy} = g_2(x) = \sum E_n \sin \frac{n\pi x}{L}$ take $\phi_n = \sum g_2(y) \cos \frac{n\pi x}{L}$

Fourier Integrals : half space with $|a_j| \rightarrow 0$ as $y \rightarrow \infty$

$$\phi = \int_{-\infty}^{\infty} e^{-i\lambda y} [A(\lambda) e^{i\lambda x} + B(\lambda) y e^{i\lambda x}] d\lambda \quad y > 0$$

w/BC $\sigma_{yy}|_{y=0} = f(x) \Rightarrow -\infty f(x) = -\int_{-\infty}^{\infty} \lambda^2 e^{-i\lambda x} A d\lambda ;$

$\sigma_{xy}|_{y=0} = g(x) \Rightarrow g(x) = \int_{-\infty}^{\infty} -i\lambda e^{-i\lambda x} \{-\lambda A + B\} d\lambda$

define $f(\lambda) = \int_{-\infty}^{\infty} f(x) e^{i\lambda x} dx$ and $f(x) = \frac{1}{2\pi} \int_{-\infty}^{\infty} f(\lambda) e^{-i\lambda x} d\lambda$

we can use this iff

$\int_0^{\infty} |f| dx \leq M$ and f is continuous on intervals $\exists. f'(x^-), f'(x^+)$ exist

and $f(x_0) = \frac{1}{2} [f(x_0^-) + f(x_0^+)]$ if x_0 is a pt of discontinuity

$$\boxed{f(x) = \frac{1}{2\pi} \int_{-\infty}^{\infty} e^{-i\lambda x} d\lambda}$$

$\int_{-\infty}^{\infty} f(x) \uparrow f(x)$

if $\sigma_{xy} = 0$ and $\sigma_{yy} = -f(x)$ then $\sigma_{yy}(x, y; \xi) = -\frac{2}{\pi} \int_{-\infty}^{\infty} \frac{y^3 f(\xi) d\xi}{[(x-\xi)^2 + y^2]^2}$

$$\sigma_{xy}(x, y; \xi) = -\frac{2}{\pi} \int_{-\infty}^{\infty} \frac{y^2 (x-\xi) f(\xi) d\xi}{[(x-\xi)^2 + y^2]^2}$$

$$\sigma_{xx}(x, y; \xi) = -\frac{2}{\pi} \int_{-\infty}^{\infty} \frac{y (x-\xi)^2 f(\xi) d\xi}{[(x-\xi)^2 + y^2]^2}$$

if $\sigma_{xy} = g(x)$ $\sigma_{yy} = 0$ then $\sigma_{yy}(x, y; \xi) = \frac{2}{\pi} \int_{-\infty}^{\infty} \frac{y^2 (x-\xi) g(\xi) d\xi}{[(x-\xi)^2 + y^2]^2}$

$$\sigma_{xy}(x, y; \xi) = \frac{2}{\pi} \int_{-\infty}^{\infty} \frac{y (x-\xi)^2 g(\xi) d\xi}{[(x-\xi)^2 + y^2]^2}$$

$$\sigma_{xx}(x, y; \xi) = \frac{2}{\pi} \int_{-\infty}^{\infty} \frac{(x-\xi)^3 g(\xi) d\xi}{[(x-\xi)^2 + y^2]^2}$$

if $\sigma_{xy} = g(x)$ and $\sigma_{yy} = f(x)$ take linear combination of above results

If loads is applied at origin of half space $\Rightarrow \tau_{r\theta}, \sigma_{\theta\theta}, \tau_{\theta z} = 0$ on all radial planes

2-D in polar coords Plane Strain soln $\nabla^2(\sigma_{kk}) = (1+\nu) \nabla^2(\sigma_{xx} + \sigma_{yy}) = (1+\nu) \nabla^2(\sigma_{rr} + \sigma_{\theta\theta}) = 0$

take $\sigma_{rr} = \frac{1}{r} \frac{\partial \phi}{\partial r} + \frac{1}{r^2} \frac{\partial^2 \phi}{\partial \theta^2}$; $\sigma_{\theta\theta} = \frac{\partial^2 \phi}{\partial r^2}$; $\sigma_{r\theta} = -\frac{\partial}{\partial r} \left(\frac{1}{r} \frac{\partial \phi}{\partial \theta} \right)$

where $\nabla^2 \phi = \frac{1}{r} \frac{\partial}{\partial r} \left(r \frac{\partial \phi}{\partial r} \right) + \frac{1}{r^2} \frac{\partial^2 \phi}{\partial \theta^2}$

see ring results - handout

Lame Solution: only pressure on boundary of ring

both plane strain & plane stress solutions are exact.

reaction to forces applied $= \oint T_y ds = -\frac{\partial \phi}{\partial x} \int_s^F$; reaction to forces applied $= \oint T_x ds = +\frac{\partial \phi}{\partial y} \int_s^F$ $\oint M_z ds = \left(\phi - r \cdot \nabla \phi \right) \int_s^F$

STRESS FUNC for pt forces in ∞ plane are given on Pg 19 / Prelim to G_{ij}

$\phi = b' r \cos \theta \ln r$ gives no traction but does give multivalued disp. Good for disloc

$\phi^{(x)}$ = stress fn for pt load in x dir at x,y (line of force in z dir)
 $\phi^{(y)}$ = " " " " " " y dir " " (line of force in z dir) } in infinite medium Pg 19

now from $\phi^{(x)}$ we can get $\sigma_{xx}^{(x)}, \sigma_{xy}^{(x)}, \sigma_{yy}^{(x)} \Rightarrow u_x^{(x)}, u_y^{(x)} : G_{11}, G_{21}$
 $\phi^{(y)}$ " " " $\sigma_{xx}^{(y)}, \sigma_{xy}^{(y)}, \sigma_{yy}^{(y)} \Rightarrow u_x^{(y)}, u_y^{(y)} : G_{12}, G_{22}$

B.I.E

$\int_s [T_i^{(1)}(x) G_{im}(x,y) - T_i^{(m)}(x,y) u_i^{(1)}(x)] ds = u_m(y)$ for y inside body

$G_{im}(x,y)$ are green's fns ; $T_i^{(m)}(x,y) = \sigma_{ij}^{(m)} n_j$ = tractions due to the green's functions

and $T_i^{(1)}(x)$ & $u_i^{(1)}(x)$ on the boundary

$= [\lambda \delta_{ij} \epsilon_{kk}^{(m)} + 2\mu \epsilon_{ij}^{(m)}] n_j$
 $= [\lambda \delta_{ij} G_{kk,m} + \mu (G_{im,j} + G_{jm,i})] n_j$

use B.I.E. to get $u_m(y)$ inside if you know $T_i^{(m)}, G_{im}, T_i^{(1)}, u_i^{(1)}$ on bdy. Normally you don't know $T_i^{(1)}(x), u_i^{(1)}(x)$ everywhere. $\therefore$ must use to get $u_m(y)$ mbdy

$\frac{1}{2} u_m(y) + \int_s u_i^{(1)}(x) T_i^{(m)}(x,y) ds = \int_s T_i^{(1)}(x) G_{im}(x,y) ds$ for y on the surface

known: $T_i^{(m)}(x,y), G_{im}(x,y)$ given either $T_i^{(1)}(x)$ or $u_i^{(1)}(x)$ on surface

Discretiz. see pg 22 my notes.

Dislocation see pg 24 $\phi = b, r \cos \theta \ln r$ or $d, r \sin \theta \ln r$

Method of images pg 24 back etc.

Line of force solution in infinite medium

2-D

$$G_{km}(\underline{x}, \underline{x}') = \frac{1}{8\pi\mu(1-\nu)} \left\{ -(3-4\nu) \ln |\underline{x} - \underline{x}'| \delta_{km} + \frac{(x_k - x'_k)(x_m - x'_m)}{|\underline{x} - \underline{x}'|^2} \right\}$$

Plane Strain
soln
 $\nu = 1/2$
 $m = 1, 2$

where $|\underline{x} - \underline{x}'| = \sqrt{(x - x')^2 + (y - y')^2}$ in the BIE

to attack problems use semi inverse technique: always state bc. clearly; draw free body diag

1. Pick state of stress to satisfy Equil Eq (may or may not solve BC).

2. Check to see if the solve compat.

3. Check to see what b.c. you've generated

a. If same as bc you want then stop

b. If not then $bc_{new} = bc_{old} - bc_{stress\ state\ picked}$
go back to 1.

Stress for ^{line of} force in infinite medium pg 19

Stress for line of force in half space pg 25 + handout

if $\phi = \frac{a_2}{2}x^2 + b_2xy + \frac{c_2}{2}y^2$: $\sigma_x = c_2$ $\sigma_y = a_2$ $\sigma_{xy} = -b_2$

state of uniform tension
compression
shear

if $\phi = \frac{a_3}{6}x^3 + \frac{b_3}{2}x^2y + \frac{c_3}{2}xy^2 + \frac{d_3}{6}y^3$: $\sigma_x = c_3x + d_3y$ $\sigma_y = b_3y + a_3x$ $\sigma_{xy} = -b_3x - c_3y$

pure bending $d_3 \neq 0$; $c_3, b_3, a_3 = 0$

if $\phi = \frac{a_4}{12}x^4 + \frac{b_4}{6}x^3y + \frac{c_4}{2}x^2y^2 + \frac{d_4}{6}xy^3 + \frac{e_4}{12}y^4$:

to solve compat $e_4 = -(2c_4 + a_4)$

$$\sigma_x = c_4x^2 + d_4xy = (2c_4 + a_4)y^2$$

$$\sigma_y = a_4x^2 + b_4xy + c_4y^2$$

$$\sigma_{xy} = -\frac{b_4}{2}x^2 - 2c_4xy - \frac{d_4}{2}y^2$$

cross-section $\oint_C (r \partial x + \alpha \partial y - \gamma_A (\frac{\partial x}{\partial x} - \frac{\partial y}{\partial y})) \partial x \partial y$
 counter-clockwise

Torsion

For arbitrary cross-section, ~~single~~ planes

do not remain plane but are warped

$$\Rightarrow w = \alpha \phi(x, y) \quad \text{index of } z \quad (1)$$

$\theta = \alpha z$
 rate of twist ϕ
 warping function

and as for circular bars

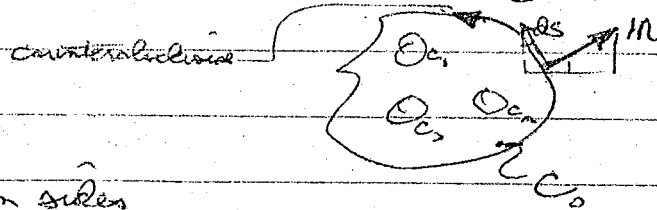
$$u = -\alpha z y \quad v = \alpha z x \quad (2)$$

Equil. equ. $\Rightarrow \nabla^2 \phi = 0$
 G.A.E. throughout region
 (3)

~~Only stress components are τ_{zx} and τ_{zy}~~
 Only stress components are $\tau_{\theta\theta}, \tau_{xz}, \tau_{xy}$

B.C.

No traction on sides; and on ends of bar
 no resultant forces, only resultant moment



$$n_x = \frac{\partial x}{\partial n} = \frac{\partial y}{\partial s}$$

$$n_y = \frac{\partial y}{\partial n} = -\frac{\partial x}{\partial s}$$

B.C. on sides

$$\frac{T_i}{T_i} = \tau_{ji} n_j$$

$$T_1 = \tau_{xx} n_x + \tau_{xy} n_y + \tau_{yx} n_x = 0 = T$$

$$T_3 = \tau_{yz} n_x + \tau_{zy} n_y + \tau_{zz} n_z = 0$$

$$\tau_{zx} = 2\mu \left[\frac{1}{2} \left(\frac{\partial w}{\partial x} + \frac{\partial u}{\partial z} \right) \right] = \mu \alpha \left(\frac{\partial \phi}{\partial x} - y \right)$$

$$\tau_{zy} = \mu \alpha \left(\frac{\partial \phi}{\partial y} + x \right)$$

$$T_3 = \mu \alpha \left[\left(\frac{\partial \phi}{\partial x} \frac{\partial x}{\partial n} + \frac{\partial \phi}{\partial y} \frac{\partial y}{\partial n} \right) - y n_x + x n_y \right] = 0$$

$$\Rightarrow \frac{\partial \phi}{\partial n} = y n_x - x n_y = y \frac{\partial y}{\partial s} - x \left(-\frac{\partial x}{\partial s} \right)$$

$$\left(\frac{\partial \phi}{\partial n} = y \frac{\partial y}{\partial s} + x \frac{\partial x}{\partial s} \right. \quad (7)$$

Introduce the conjugate harmonic function ψ such that

$$f(z) = \phi + i\psi \quad (8)$$

and from Cauchy-Riemann relations

$$\frac{\partial \phi}{\partial x} = \frac{\partial \psi}{\partial y} \quad (9)$$

$$\frac{\partial \phi}{\partial y} = -\frac{\partial \psi}{\partial x} \quad (10)$$

$$(9)(10) \Rightarrow (7)$$

$$\frac{\partial \psi}{\partial y} \frac{\partial x}{\partial n} - \frac{\partial \psi}{\partial x} \frac{\partial y}{\partial n} = y \frac{\partial y}{\partial s} + x \frac{\partial x}{\partial s} \quad (11)$$

$\frac{\partial x}{\partial s} \quad \frac{\partial y}{\partial s}$

$$\Rightarrow \frac{\partial \psi}{\partial s} = y \frac{\partial y}{\partial s} + x \frac{\partial x}{\partial s} = \frac{1}{2} \frac{\partial}{\partial s} (x^2 + y^2)$$

integrating along the boundary

$$\psi = \frac{1}{2} (x^2 + y^2) + R_0 \quad \text{on boundary } C_0$$

R_0 is arbitrary, usually taken as zero

if multiply connected (12) must hold on all boundaries $C_0, C_1, C_2, \dots, C_n$

$$\Rightarrow (12) \quad \psi = \frac{1}{2} (x^2 + y^2) + R_i \quad \text{on boundaries } C_i, i=0,1,2,\dots \quad (14)$$

B.C. ψ

and since ψ is analytic

$$\nabla^2 \psi = 0 \quad (15)$$

Can introduce the Prandtl stress function Ψ where

$$\Psi = \psi - \frac{1}{2}(x^2 + y^2) \quad (16)$$

$$\left. \begin{aligned} \Rightarrow \nabla^2 \Psi &= -2 \\ \text{B.C. } \Psi &= R_i \text{ on } C_i \end{aligned} \right\} \quad (17)$$

and satisfy (24) for compatibility in multiply

Problem was originally formulated in terms of displacement, thus for a simply connected region, compatibility is guaranteed, but for a multiply connected region in order for



$$\oint_S d\vec{u} = 0$$

where S is a reducible path (18)

$$\Rightarrow \oint_{C_i} d\vec{u} + \oint_{C_{i+1}} d\vec{u} = 0$$

(19)

must satisfy the additional relations

$$\oint_{C_i} d\vec{u} = 0$$

$i = 1, 2, 3, \dots$ (20)

R_i must be chosen so that (20) is satisfied

$$\vec{u} = u\hat{i} + v\hat{j} + w\hat{k} \quad (21)$$

u and v as given by (6) are single valued

thus (20) $\Rightarrow \oint_{C_i} dw = 0$

$$\Rightarrow \oint_{C_i} \left(\frac{\partial w}{\partial x} dx + \frac{\partial w}{\partial y} dy \right) = 0 \quad (22)$$

$$\frac{\partial w}{\partial x} = \alpha \frac{\partial \phi}{\partial x} = \alpha \frac{\partial \psi}{\partial y} = \alpha \left(\frac{\partial \Psi}{\partial y} + y \right) \quad (23)$$

likewise $\frac{\partial w}{\partial y} = \alpha \frac{\partial \phi}{\partial y} = -\alpha \frac{\partial \psi}{\partial x} = -\alpha \left(\frac{\partial \Psi}{\partial x} + x \right)$

(4)

(23) $\rightarrow$ (22)

$$\oint_{C_i} \left(\frac{\partial \psi}{\partial y} \frac{\partial x}{\partial s} - \frac{\partial \psi}{\partial x} \frac{\partial y}{\partial s} \right) ds = - \oint_{C_i} y \partial x - x \partial y$$

(by (4), but clockwise circuit) Green's theorem

$$\oint_{C_i} \left(\frac{\partial \psi}{\partial y} \frac{\partial y}{\partial n} - \frac{\partial \psi}{\partial x} \left(-\frac{\partial x}{\partial n} \right) \right) ds = + \iint_{A_i} (-1-1) dx dy = -2$$

sign change because of direction of integration

$$\oint_{C_i} \frac{\partial \psi}{\partial n} ds = -2A_i \quad i=1,2,\dots \quad (24)$$

See also Q 170 Sokolnikoff

End moment M_z may be found by integration of the stress times a suitable moment arm

$$M_z = G\alpha \left[2 \iint_A \psi x dy - 2k_0 A_0 + 2 \sum_{j=1}^m k_j A_j \right] = GJ\alpha \quad (25)$$

Membrane analogy

Membrane under uniform tension F with pressure θ on surface

$$\nabla^2 z = -\theta/F = -2\left(\frac{p}{2F}\right) \quad (26)$$

$$\nabla^2 \left(\frac{2F}{\theta} z \right) = -2 \quad (27)$$

$$\text{B.C. } z = c_i \quad \text{on boundaries} \quad (28)$$

$$\text{so } \frac{2F}{\theta} z = \psi$$

Membrane analogy is also useful as an aid in visualization of the stress distribution. Lines of $z = \text{const}$ are also lines of $\Psi = \text{const}$. The magnitude of the stress depends on $\frac{\partial \Psi}{\partial n}$, so $\max_{\text{stress}}$ must occur on the boundary.
(for Ψ or Ψ')

Also note solution is unaffected by cuts along a line $\Psi = \text{const}$ ($z = \text{const}$).
By this I mean if one has the ~~solution~~ solution for a solid elliptical bar, the lines of const. Ψ are similar ellipses with the same center. Thus, for a hollow elliptical bar with interior boundary along a line of const Ψ , solution for Ψ is same as that for a solid elliptical bar. It is of course not as stiff (lower J) due to the decreased volume.

ω related to rigid rotation; measures
avg. rotation of all line elements through a point

ϵ infinitesimal strain tensor

diagonal terms — related to change in length

off-diagonal terms — related to changes in angle

Compatibility

For single valued displacements, want du_i
to be an exact differential or $\oint du_i = 0$

For a simply connected region, this is insured
by

$$\nabla \times \epsilon \times \nabla = 0$$

6 equations
must be satisfied
at each point in the
medium

$$\epsilon_{ij,k\ell} + \epsilon_{k\ell,ij} - \epsilon_{ik,j\ell} - \epsilon_{j\ell,ik} = 0$$

For multiply-connected region

$$\oint_{C_i} du_i = 0 \quad \text{where } C_i \text{ are the interior contours}$$

Stress Compatibility

Beltrami Eqs. (const. body force)

$$\nabla^2 \sigma_{ij} + \frac{1}{1+\nu} \sigma_{kk,ij} = 0$$

and in this case $\nabla^2 \sigma_{kk} = 0$

Beltrami-Michell Eqs.

$$\nabla^2 \sigma_{ij} + \frac{1}{1+\nu} \sigma_{kk,ij} = -\frac{\nu}{1-\nu} \delta_{ij} \nabla \cdot F - (F_{i,j} + F_{j,i})$$

1000

1000

Theory of Elasticity Prof. Herrmann Rm 121

#521 Notes

Course supplemented by HW (3 or 4 in quarter)

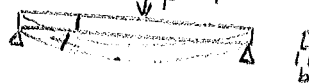
1 hr midterm + 1 final exam (Take home or in class)

Lab Assistant: Rich King Durand 264 12-2 W; 2-4 T

Course deals with deformable bodies. Elasticity deals with what action body will take under applied loads. When loads are removed, body returns to original shape without hysteresis.

— POSSIBLE CHOICE IN MODELS —

Strength of Material assumption: plane sections remain plane & are $\perp$ to Q of beam its a 1-D analysis



Elasticity - σ, ϵ relation exist, compatibility, equilib, boundary conditions must also be satisfied. Stress concentration

How to attack problem: What quantities do I need? Which model will I use to get this?

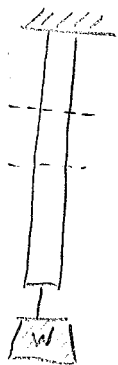
We will discuss

Stress, Strain, Rotation, Uniqueness of Solution, Problems, Torsion

Winter quarter plane problems, Problems dealing with cracks.

Spring quarter 3-D problems, contact stresses & elastic contact.

Notations found within the literature



Look at free body diagram since body in equil means any part in equil

Use Taylor's theorem to represent Q as a function of P in 1-D analysis

$$Q = P + \frac{dP}{dx} dx + \dots \quad (\text{to 1st order})$$

$$\sum F_y = 0: \text{EQUIL} \quad -P + \left[P + \frac{dP}{dx} dx \right] = 0 \quad \text{or} \quad \frac{dP}{dx} = 0 \quad \text{if no body force}$$

$$\text{if body force} \quad \frac{dP}{dx} = -F \quad \text{etc.}$$

Explicit cartesian form.

$$\frac{\partial \sigma_{xx}}{\partial x} + \frac{\partial \sigma_{yx}}{\partial y} + \frac{\partial \sigma_{zx}}{\partial z} + f_x = 0$$

$$\frac{\partial \sigma_{xy}}{\partial x} + \frac{\partial \sigma_{yy}}{\partial y} + \frac{\partial \sigma_{zy}}{\partial z} + f_y = 0$$

$$\frac{\partial \sigma_{xz}}{\partial x} + \frac{\partial \sigma_{yz}}{\partial y} + \frac{\partial \sigma_{zz}}{\partial z} + f_z = 0$$

stress equations of equil
(component form)

Cartesian Tensor form (indexial notation subscript)

$$\sigma_{jk,j} + f_k = 0$$

$$\frac{\partial \sigma_{jk}}{\partial x_j} + f_k = 0$$

indices { if appears once free subscript
if appears twice implies summation over $j=x,y,z$
comma represents differentiation

Limitation is only to cartesian system breaks down for other curvilinear coordinates

$$T^{\alpha\beta}_{;\alpha} + f^\beta = 0$$

$$(T^{\alpha\beta}_{;\alpha} + \Gamma^{\beta}_{\alpha\kappa} T^{\alpha\kappa} + \Gamma^{\kappa}_{\alpha\kappa} T^{\alpha\beta} + f^\beta = 0)$$

General Tensor form

T is stress tensor

$;$ is differentiation wrt curvilinear coordinates

Γ Cristoffel transformations

Rm 121 from now on

$$\nabla \cdot \sigma + f = 0$$

Symbolic, Dyadic, Gibbs form
greek letter w/ bar will be a 2nd rank tensor

Matrix Notation

$$5. (\text{div}_d T)^* + F = 0$$

✓ notations will be used

A.E.H. Love "A Treatise on the Mathematical Theory of Elasticity" (1892) (4th Ed 1926)
(Dover Publ.)

S. Timoshenko & J.N. Goodier "Theory of Elasticity" 3rd 1970 } use component form

I.S. Sokolnikoff "Math Theory of Elast" 2nd Ed 1956 uses CARTESIAN TENSORS

A.E. Green & W. Zerna "Theoretical Elasticity" Oxford University Press 1954 uses
general tensors emphasizes non-linear elast

F.T. Murman jhan "Finite deformations of an elastic solids" (Wiley 1951) use
matrix notation.

H.M. Westergaard "Theory of Elasticity & Plasticity" (1952 Harvard Univ Press,
uses DYADICS

For Math Background:

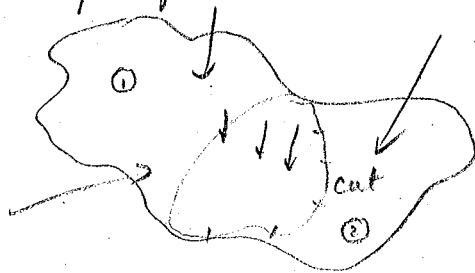
DYADIC notation: C.E. Weatherburn (Open Court 1948) "Elem Vector Anal & Advanced Vector Anal

Cartesian tensors: H Jeffreys "Cartesian Tensors" Cambridge Univ 1952

General tensors: Sokolnikoff "Tensor Analysis" Wiley 2nd 1948

matrices: A.D. Michel "Matrix & Tensor Calculus" Wiley 1947.

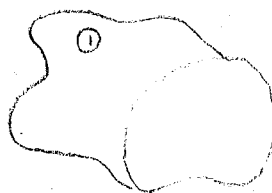
Analysis of Stress



if forces acting on it.

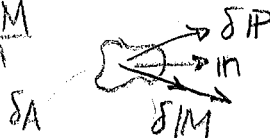
Contact forces - 2 contiguous bodies touching

Distance force - gravity



if body originally in equil
this part is also in equil

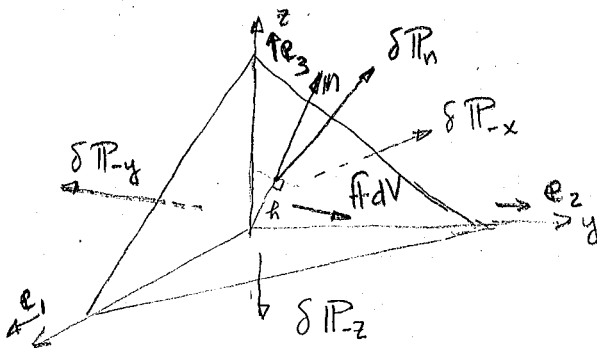
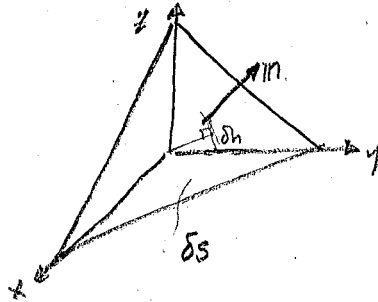
We assume that forces acting on ① by ② are uniform over entire surface
so we only need to look at one small area & resultant force & moment acting
over it. Look at $\frac{\delta F}{\delta A}$, $\frac{\delta M}{\delta A}$



take lim as $\delta A \rightarrow 0$ of each, $\lim_{\delta A \rightarrow 0} \frac{\delta F}{\delta A} = t_n$ traction (stress vector)

li $\frac{\delta M}{\delta A} = 0$ is assumed

Introduce cartesian



10/2/78

How to find the traction $t_n = \lim_{\delta S \rightarrow 0} \frac{\delta P_n}{\delta S}$ as a fn of the tractions in e_x, e_y, e_z
 Applying $\sum F = 0$
 $\delta P_{-j} = -\delta P_j$ (1)

$$\delta P_n + \delta P_{-x} + \delta P_{-y} + \delta P_{-z} + f dV = 0 \quad (2)$$

$$\delta P_n - \delta P_x - \delta P_y - \delta P_z + f dV = 0 \quad (1+2) \rightarrow (3)$$

$$\delta S_x = \delta S \cos(n, e_x)$$

$$\delta S_y = \delta S \cos(n, e_y)$$

$$\delta S_z = \delta S \cos(n, e_z)$$

$$\delta S = \frac{\delta S_j}{n \cdot e_j} \quad (4)$$

divide (3) by δS & sub (4) and $\delta V = \frac{\delta S \cdot \delta h}{3}$

$$\frac{\delta P_n}{\delta S} - \frac{\delta P_x}{\delta S} - \frac{\delta P_y}{\delta S} - \frac{\delta P_z}{\delta S} + \frac{f dV}{\delta S} = 0$$

$$\frac{\delta P_n}{\delta S} - n \cdot e_x \frac{\delta P_x}{\delta S_x} - n \cdot e_y \frac{\delta P_y}{\delta S_y} - n \cdot e_z \frac{\delta P_z}{\delta S_z} + f \frac{\delta S \cdot \delta h}{3 \delta S} = 0$$

now let $\delta S \rightarrow 0 \Rightarrow \delta S_x, \delta S_y, \delta S_z$ and $\delta h \rightarrow 0$ wipes out body forces.

$$\therefore t_n - n \cdot e_x t_x - n \cdot e_y t_y - n \cdot e_z t_z = 0$$

$$\text{or } t_n - n \cdot e_1 t_1 - n \cdot e_2 t_2 - n \cdot e_3 t_3 = 0$$

$$\text{or } t_n - n \cdot e_j t_j = 0 \quad \text{where } j \text{ appears twice implies summation}$$

$t_n = n \cdot e_j t_j$ Traction on a surface whose normal is n , as a linear combination of the tractions on the cartesian surfaces

t_x has a general direction and can be resolved.

$$\text{so } t_x = \sigma_{xx} e_x + \sigma_{xy} e_y + \sigma_{xz} e_z$$

$$t_y = \sigma_{yx} e_x + \sigma_{yy} e_y + \sigma_{yz} e_z$$

$$t_z = \sigma_{zx} e_x + \sigma_{zy} e_y + \sigma_{zz} e_z$$

State of stress at a point is said to be known if traction on every plane passed through that point is known

The 9 quantities called components of stress

$$t_j = \sigma_{jx} e_x + \sigma_{jy} e_y + \sigma_{jz} e_z = \sigma_{jk} e_k$$

$$\text{or } t_j = \sigma_{jk} e_k$$

$$t_n = n \cdot [e_1 t_1 + e_2 t_2 + e_3 t_3] = n \cdot \sigma$$

this is in terms of e_x, e_y, e_z in original coordinate system.

σ is called the stress dyadic and is a second rank tensor

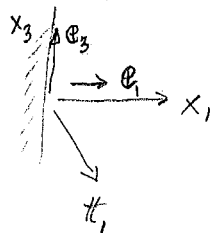
note: when vectors positioned next to each other, no operation.

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$$t_n = m \cdot e_1 t_1 + m \cdot e_2 t_2 + m \cdot e_3 t_3$$

these do not mean same

t_1 acts across the plane whose normal is e_1



$$t_1 = e_1 \cdot \sigma = \sigma_{xx} e_x + \sigma_{xy} e_y + \sigma_{xz} e_z$$

$$t_n = m \cdot [e_1 t_1 + e_2 t_2 + e_3 t_3] = m \cdot \sigma$$

dyadic (second rank tensor)

σ is now defined as a stress

$$t_x = \sigma_{xx} e_x + \sigma_{xy} e_y + \sigma_{xz} e_z$$

σ_{ij} ← orientation of traction
← direction it is acting in

$$t_j = \sigma_{jk} e_k \quad \text{subscript notation}$$

now since $t_n = m \cdot \sigma$

$$t_j = e_j \cdot \sigma$$

really means traction in the e_j direction

now subst $t_{x,y,z}$ for $t_{1,2,3}$

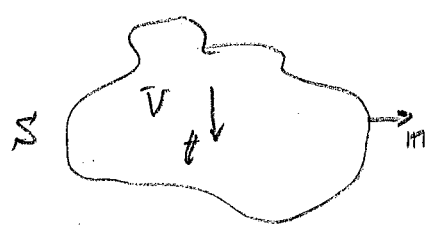
$$t_n = m \cdot [e_x (\sigma_{xx} e_x + \sigma_{xy} e_y + \sigma_{xz} e_z) + e_y (\sigma_{yx} e_x + \sigma_{yy} e_y + \sigma_{yz} e_z) + e_z (\sigma_{zx} e_x + \sigma_{zy} e_y + \sigma_{zz} e_z)]$$

$$= m \cdot \sigma \quad \text{or} \quad \boxed{\sigma = \sigma_{ij} e_i e_j}$$

we define normal stress σ_{ij} $i=j$
 shear stress σ_{ij} $i \neq j$

Principle of linear momentum [must know volume V and a given S (surface) with a given set of surface tractions and the unit body force]

$\Sigma \text{ force} = \frac{d}{dt}$ of momentum. Here we assume material $\rho = \text{const}$ & V is fixed

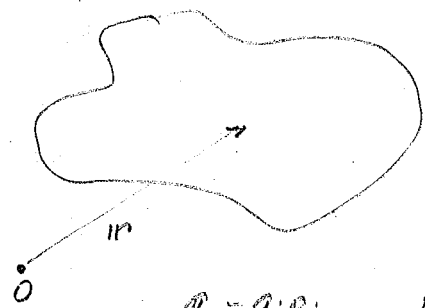


$$\int_S t_n ds + \int_V f dV = \int_V \rho \frac{\partial^2 u}{\partial t^2} dV$$

u = displacement vector

Principle of Angular Momentum

Σ moment about some given pt is the time rate of change of angular mom.



$$\int_S r \times t_n dS + \int_V r \times f dV = \int_V r \times \rho \frac{\partial^2 u}{\partial t^2} dV$$

$a = a_i e_i$ $b = b_j e_j$ $a \cdot b = a_i b_j \delta_{ij} = a_i b_i$
 $a \times b = (a_2 b_3 - a_3 b_2) e_1 + (a_3 b_1 - a_1 b_3) e_2 + (a_1 b_2 - a_2 b_1) e_3$

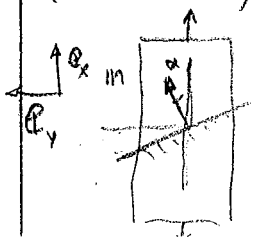
$a \times b = \begin{vmatrix} e_1 & e_2 & e_3 \\ a_1 & a_2 & a_3 \\ b_1 & b_2 & b_3 \end{vmatrix}$ $a \times b = a_i b_j e_{ijk} e_k$

Definition

Alternating symbol $e_{ijk} = \begin{cases} +1 & i, j, k \text{ are cyclic} \\ 0 & \text{if others.} \\ -1 & i, j, k \text{ are anti-cyclic} \end{cases}$

(HW due 2 wks) given in class

10/6/78



$\sigma = ?$
 $\sigma = \sigma_{xx} e_x e_x$
 $n = n_x e_x + n_y e_y$ in terms of the original e_x, e_y
 $n = n \cdot e_x e_x + n \cdot e_y e_y = \cos \alpha e_x + \sin \alpha e_y$

aside $A = a_x e_x + a_y e_y$
 $a \cdot e_x = a_x \quad \therefore$

$$a = a \cdot e_x e_x + a \cdot e_y e_y$$

$$= a \cdot [e_x e_x + e_y e_y]$$

$$= a \cdot \mathbb{I}$$

$\mathbb{I}$ identity dyadic
 idempactor

$$t_n = \mathbb{I} \cdot \nabla$$

$$= \nabla_{xx} \cos \alpha e_x$$

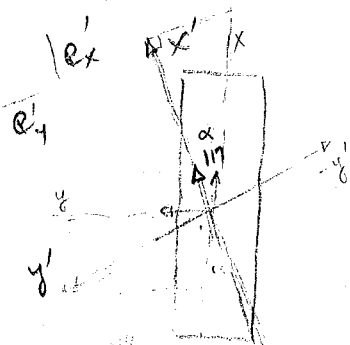
$$t_n = \frac{P}{A} \cos \alpha e_x$$

Now since we want to find t_n in terms of e'_x, e'_y & we know that $t_n = (\text{comp of } t_n \text{ in } e'_x) e'_x + (\text{comp of } t_n \text{ in } e'_y) e'_y$ then

$$t_n = t_n \cdot e'_x e'_x + t_n \cdot e'_y e'_y$$

$$= \frac{P \cos \alpha}{A} \underbrace{e_x \cdot e'_x}_{\cos \alpha} e'_x + \frac{P \cos \alpha (\sin \alpha)}{A} e'_y$$

$$= \underbrace{t_n}_{(n \cdot e_x)} \underbrace{e_x \cdot e'_x}_{(e_x \cdot e'_x)} + t_n e'_x (e_y)$$

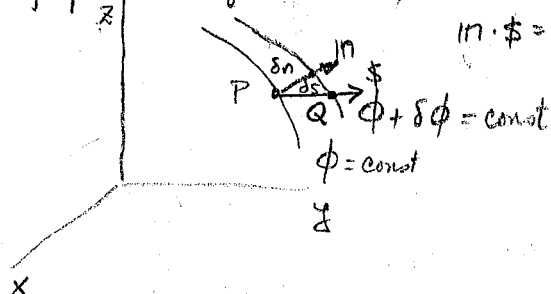


To work a problem

1. Introducing new coordinate system if convenient & work problem

Divergence Theorem $\int_V \nabla \cdot \underline{q} dV = \int_A \underline{q} \cdot \underline{n} dA$

working up to divergence theorem



$\mathbb{I} \cdot \underline{s} = \cos \alpha$ 0. let ϕ be a scalar fn.

1. @ P define tangent plane & the normal to that plane
2. draw $\underline{s}$ in arbitrary direction let it intersect $\phi + \delta \phi$ @ Q
3. look at $\frac{\delta \phi}{\delta s}$

$$\lim_{\delta s \rightarrow 0} \frac{\delta \phi}{\delta s} = \frac{\partial \phi}{\partial s} \quad (\text{directional derivative})$$

$$\lim_{\delta n \rightarrow 0} \frac{\delta \phi}{\delta n} = \frac{\partial \phi}{\partial n} \quad (\text{normal derivative})$$

note $\frac{\partial \phi}{\partial n} \geq \frac{\partial \phi}{\partial s}$ since $\partial \phi$ is the same & $\delta s > \delta n$

$$\frac{\partial \phi}{\partial s} = \lim_{\delta s \rightarrow 0} \frac{\delta \phi}{\delta n} \frac{\delta n}{\delta s} = \frac{\partial \phi}{\partial n} \cos(n, \hat{s}) = \frac{\partial \phi}{\partial n} \cdot \frac{(n \cdot \hat{s})}{|\hat{s}|} = n \frac{\partial \phi}{\partial n} \cdot \hat{s} = \nabla \phi \cdot \hat{s}$$

$n \frac{\partial \phi}{\partial n} = \nabla \phi$ This is ⁱⁿ the direction of greatest or fastest change

$$\frac{\partial \phi}{\partial s} = \nabla \phi \cdot \hat{s} \quad \text{the} \quad \frac{\partial \phi}{\partial x_k} = \nabla \phi \cdot \mathbf{e}_k$$

now write $\nabla \phi = \frac{\partial \phi}{\partial x_1} \mathbf{e}_1 + \frac{\partial \phi}{\partial x_2} \mathbf{e}_2 + \frac{\partial \phi}{\partial x_3} \mathbf{e}_3$

define $\nabla(\) = \frac{\partial(\)}{\partial x_1} \mathbf{e}_1 + \frac{\partial(\)}{\partial x_2} \mathbf{e}_2 + \frac{\partial(\)}{\partial x_3} \mathbf{e}_3 = \mathbf{e}_i \frac{\partial(\)}{\partial x_i}$

- now look @ $\mathbf{F} = \mathbf{F}(x, y, z)$

and $\nabla \cdot \mathbf{F}$ (Divergence of $\mathbf{F}$)

$\nabla \times \mathbf{F}$ (cross product or curl of $\mathbf{F}$)

let $\mathbf{F} = F_1 \mathbf{e}_1 + F_2 \mathbf{e}_2 + F_3 \mathbf{e}_3$

$$\nabla \cdot \mathbf{F} = \mathbf{e}_j \frac{\partial}{\partial x_j} \cdot (F_i \mathbf{e}_i) = \frac{\partial F_i}{\partial x_i} = F_{i,i} = \delta_{ij} \frac{\partial F_i}{\partial x_j} = \frac{\partial F_i}{\partial x_i}$$

$$\begin{aligned} \nabla \times \mathbf{F} &= \left(\mathbf{e}_j \frac{\partial}{\partial x_j} \right) \times F_i \mathbf{e}_i = \mathbf{e}_1 \left(\frac{\partial F_3}{\partial x_2} - \frac{\partial F_2}{\partial x_3} \right) + \mathbf{e}_2 \left(\frac{\partial F_1}{\partial x_3} - \frac{\partial F_3}{\partial x_1} \right) \\ &\quad + \mathbf{e}_3 \left(\frac{\partial F_2}{\partial x_1} - \frac{\partial F_1}{\partial x_2} \right) = \mathbf{e}_j \times \mathbf{e}_i \frac{\partial F_i}{\partial x_j} = \epsilon_{jil} \mathbf{e}_l \frac{\partial F_i}{\partial x_j} \\ &= \epsilon_{jil} \mathbf{e}_l \frac{\partial F_i}{\partial x_j} \\ &= \epsilon_{ijk} \mathbf{e}_k \frac{\partial F_i}{\partial x_j} \end{aligned}$$

$$\nabla \times \mathbf{F} = \epsilon_{ijk} F_{k,j} \mathbf{e}_i$$

$$\nabla \cdot \sigma = \nabla \cdot (t_k \mathbf{e}_k) = \frac{\partial t_1}{\partial x_1} + \frac{\partial t_2}{\partial x_2} + \frac{\partial t_3}{\partial x_3}$$

$$\nabla \cdot (t_k \mathbf{e}_k) = \mathbf{e}_i \frac{\partial}{\partial x_i} \cdot (\mathbf{e}_k \sigma_{kj} \mathbf{e}_j) = \sigma_{kji} \delta_{ik} \mathbf{e}_j = \sigma_{k,j,k} \mathbf{e}_j \quad \text{or since indices are dummy } \sigma_{j,k,j} \mathbf{e}_k$$

$$\nabla \cdot \sigma = \sigma_{j,k,j} \mathbf{e}_k$$

10/4/78

$\nabla \phi, \nabla \cdot \mathbf{F}, \nabla \times \mathbf{F}$

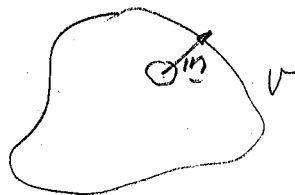
we can extend ∇ to dyadic $\therefore \nabla \cdot \sigma = \sigma_{j,k,j} \mathbf{e}_k$

Divergence Theorem (Gauss)

volume V enclosed by surface S Vector point function $G = G(x, y, z)$

$$\int_V \nabla \cdot G \, dV = \int_S G \cdot n \, ds$$

$$\int_V G_{i,j} \, dV = \int_S n_j G_{i,j} \, ds$$

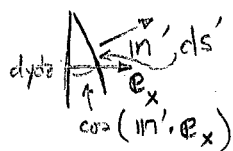


look at prism || to x axis

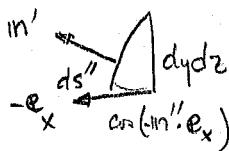


$$\iiint_V \frac{\partial G_x}{\partial x} \, dx \, dy \, dz = \iint (G'_x - G''_x) \, dy \, dz$$

$$dy \, dz = ds' n' \cdot e_x$$



$$\therefore dy \, dz = ds' \cos(n', e_x) = ds' \frac{n' \cdot e_x}{\|n'\| \|e_x\|} = ds' n' \cdot e_x$$



$$dy \, dz = + ds'' \cos(n'', -e_x) = -ds'' \frac{n'' \cdot e_x}{\|n''\| \|e_x\|} = -ds'' n'' \cdot e_x$$

$$\begin{aligned} \therefore \iint G'_x \, ds' n' \cdot e_x + G''_x \, ds'' n'' \cdot e_x &= \int_S G_x n \cdot e_x \, ds \\ &= \int_S n_x G_x \, ds \end{aligned}$$

to show $\int_V \nabla \cdot \sigma \, dV = \int_S n \cdot \sigma \, ds$

$$\int_V \nabla \cdot \sigma \, dV = \int_V \nabla \cdot (\underline{\underline{\sigma}}) \, dV = e_k \int_V \nabla \cdot \underline{\underline{\sigma}}_k \, dV$$

$$= e_k \int_V n_j \sigma_{kj} \, dV = e_k \int_S n_j \sigma_{kj} \, ds = \int_S e_k n_j \sigma_{kj} \, ds$$

since $\underline{\underline{\sigma}}_k = \sigma_{kj} e_j$ $\nabla \cdot \underline{\underline{\sigma}}_k = \sigma_{kj,k} e_j \text{ or } \sigma_{jk,j} e_k$

$$\int_S \sigma \cdot n \, ds = \leftarrow$$

$$\text{or } \int_V \sigma_{jk,j} \, dV = \int_S n_j \sigma_{jk} \, ds$$

to Principle of Linear Momentum (PLM) [to prove $(\sigma_{ij} = \sigma_{ji})$]

$$\int_S t_n ds = \int_S m \cdot \sigma ds = \int_V \nabla \cdot \sigma dV$$

$$\int_S t_n ds + \int_V f dV = + \int_V \rho \frac{\partial^2 u}{\partial t^2} dV$$

$$\text{or } \int_V \left[\nabla \cdot \sigma + f - \rho \frac{\partial^2 u}{\partial t^2} \right] dV = 0 \Rightarrow \nabla \cdot \sigma + f = \rho \frac{\partial^2 u}{\partial t^2}$$

we will only look @ statics w/ $\frac{\partial^2}{\partial t^2}$ terms out

$$\nabla \cdot \sigma + f = 0 \quad \sigma_{jk,j} + f_k = 0 \quad \text{will be our basic equation}$$

to Principle of Angular Mom.

take PLM $\times r$ with $r = x_i e_i$

$$\int_S t_n \times r ds + \int_V f \times r dV = \int_V \rho \frac{\partial^2 u}{\partial t^2} \times r dV$$

recall that $t_n = \sigma_{nk} e_k$ and write out eqn. w/ z -direction This arises from x cross y or y cross x term

$$\int_S (\sigma_{nx} y - \sigma_{ny} x) ds + \int_V (f_x y - f_y x) dV = \int_V \rho \left(\frac{\partial^2 u_x}{\partial t^2} y - \frac{\partial^2 u_y}{\partial t^2} x \right) dV$$

$$\sigma_{nx} = m \cdot \sigma \cdot e_x = m \cdot (\sigma_{ij} e_i \cdot e_j) \cdot e_x = m \cdot (\sigma_{ix} e_i) = \sigma_{xx} m \cdot e_x + \sigma_{yx} m \cdot e_y + \sigma_{zx} m \cdot e_z$$

and $m \cdot e_i = n_i$ (component of m in e_i direction)

$$\int_S ([n_x \sigma_{xx} + n_y \sigma_{yx} + n_z \sigma_{zx}] y - [n_x \sigma_{xy} + n_y \sigma_{yy} + n_z \sigma_{zy}] x) ds$$

$$\text{Apply } \int_V \sigma_{jk,j} dV = \int_S n_j \sigma_{jn} ds$$

$$\int_V \left\{ \left[\frac{\partial(\sigma_{xx} y)}{\partial x} + \frac{\partial(\sigma_{yx} y)}{\partial y} + \frac{\partial(\sigma_{zx} y)}{\partial z} \right] - \left[\frac{\partial(\sigma_{xy} x)}{\partial x} + \frac{\partial(\sigma_{yy} x)}{\partial y} + \frac{\partial(\sigma_{zy} x)}{\partial z} \right] \right\} dV$$

$$\text{or } (\sigma_{xx,x} + \sigma_{yx,y} + \sigma_{zx,z}) y + \sigma_{yx} - \sigma_{xy} - (\sigma_{xy,x} + \sigma_{yy,y} + \sigma_{zy,z}) x + (f_x y - \rho \frac{\partial^2 u_x}{\partial t^2} y - f_y x + \rho \frac{\partial^2 u_y}{\partial t^2} x) = 0$$

but $\sigma_{ij} = \sigma_{ji}$ which cancels w/ $\dots$

look at coeff of y that is PLM in x dir $\epsilon = 0$
 of x that is PLM in y dir $\epsilon = 0$

$$\Rightarrow \int_V (\sigma_{xy} + \sigma_{yx}) dV = 0 \Rightarrow \sigma_{xy} = \sigma_{yx} \text{ since any } dV \text{ is taken}$$

$$\sigma_{jk} = \sigma_{kj} \quad k \neq j$$

10/10/28

repeated derivations in indicial notation
 P.A.M.

$$\int_S e_{ijk} \sigma_{nj} x_k ds + \int_V e_{ijk} f_j x_k dV = \int_V \rho e_{ijk} \frac{\partial^2 u_j}{\partial t^2} x_k dV$$

$$\int_S e_{ijk} n_k \sigma_{ej} x_k ds = \int_S n_k [\sigma_{ej} x_k e_{ijk}] ds$$

$$\text{Applying divergence theorem } \int_V [\sigma_{ej} x_k e_{ijk}]_{,e} dV = \int_S n_k [\sigma_{ej} x_k e_{ijk}] ds$$

$$\text{or } \int e_{ijk} [\underbrace{\sigma_{ej,e} x_k}_{\sigma_{kj}} + \underbrace{\sigma_{ej} \delta_{ke}}_{\sigma_{kj}} + \sigma_{ej} x_k \underbrace{e_{ijk,e}}_{\text{zero}}] + f_j x_k - \rho \frac{\partial^2 u_j}{\partial t^2} x_k dV = 0$$

but $\sigma_{ej,e} + f_j - \rho \frac{\partial^2 u_j}{\partial t^2} = 0$ by principle of linear mom $\therefore$

$$\int_V e_{ijk} \sigma_{kj} dV = 0 \quad \text{and } \Rightarrow e_{ijk} \sigma_{kj} = 0 \Rightarrow \sigma_{ej} = \sigma_{jk}$$

now for non orthogonal directions m, n is $\sigma_{mn} = \sigma_{nm}$?

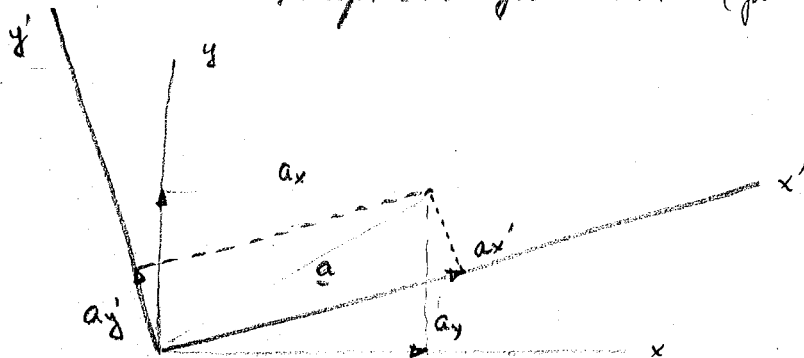
$$\sigma_{mn} = \underline{t}_m \cdot \underline{n} = \underline{n} \cdot \underline{\sigma} \cdot \underline{m} = \underline{m} \cdot \underline{\sigma}^T \cdot \underline{n} = \underline{m}_k \underline{t}_k \cdot \underline{n} = m_k \underline{t}_k \cdot \underline{n}$$

— this from angular momentum

$$\text{or } m_k \sigma_{kj} n_j = m_k \sigma_{jk} n_j = n_j \sigma_{jk} m_k = \underline{n} \cdot \underline{t}_k m_k = \underline{n} \cdot \underline{t}_m = \underline{n} \cdot \underline{\sigma} \cdot \underline{m}$$

$$= \sigma_{nm} \quad \therefore \sigma_{mn} = \sigma_{nm}$$

Coordinate transformation for a vector (for orthogonal systems)



Note that magnitude of $\vec{a}$ is same in all coordinate systems $\therefore a_x^2 + a_y^2 = a'^2_x + a'^2_y$

transformation of coordinate

$$\sigma'_{kl} = \mathbf{e}'_k \cdot \mathbf{T} \cdot \mathbf{e}'_l$$

where $\mathbf{T} = T_{ij} \mathbf{e}_i \mathbf{e}_j$

$$\text{then } \sigma'_{kl} = \mathbf{e}'_k \cdot T_{ij} \mathbf{e}_i \mathbf{e}_j \cdot \mathbf{e}'_l$$

$$\text{define for } \mathbf{e}'_j \cdot \mathbf{e}_i = l_{ji} = \cos(\mathbf{e}'_j, \mathbf{e}_i)$$

note that $l_{ij} \neq l_{ji}$

$$\therefore \mathbf{e}'_1 = \underbrace{(\mathbf{e}'_1 \cdot \mathbf{e}_1)}_{l_{11}} \mathbf{e}_1 + \underbrace{(\mathbf{e}'_1 \cdot \mathbf{e}_2)}_{l_{12}} \mathbf{e}_2 + \underbrace{(\mathbf{e}'_1 \cdot \mathbf{e}_3)}_{l_{13}} \mathbf{e}_3$$

$$\begin{array}{l} \mathbf{e}'_1 \\ \mathbf{e}'_2 \\ \mathbf{e}'_3 \end{array} \left| \begin{array}{ccc} \mathbf{e}_1 & \mathbf{e}_2 & \mathbf{e}_3 \\ l_{11} & l_{12} & l_{13} \\ l_{21} & l_{22} & l_{23} \\ l_{31} & l_{32} & l_{33} \end{array} \right|$$

$$\mathbf{e}'_1 \cdot \mathbf{e}'_1 = l_{11}^2 + l_{12}^2 + l_{13}^2 = 1$$

3 equations of this type exist

3 orthogonal relations $\mathbf{e}'_1 \cdot \mathbf{e}'_2 = l_{11}l_{21} + l_{12}l_{22} + l_{13}l_{23} = 0$

$$\therefore \mathbf{e}'_j \cdot \mathbf{e}'_k = \delta_{jk}$$

To prove by indicial notation

$$\text{or } \mathbf{e}'_j \cdot \mathbf{e}'_k = (l_{j\alpha} \mathbf{e}_\alpha) \cdot (l_{k\beta} \mathbf{e}_\beta) = l_{j\alpha} l_{k\beta} \delta_{\alpha\beta} = l_{j\alpha} l_{k\alpha} = \delta_{jk}$$

$$\text{now } \mathbf{a} = a_i \mathbf{e}_i = a'_i \mathbf{e}'_i$$

$$a'_i = \mathbf{a} \cdot \mathbf{e}'_i = a_k \mathbf{e}_k \cdot \mathbf{e}'_i = a_k l_{ik}$$

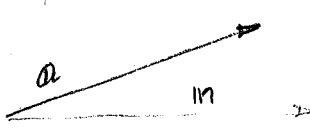
$$a_i = \mathbf{a} \cdot \mathbf{e}_i = a'_k \mathbf{e}'_k \cdot \mathbf{e}_i = a'_k l_{ki}$$

back to

$$\sigma'_{mn} = \mathbf{e}'_m \cdot \mathbf{T} \cdot \mathbf{e}'_n = T_{ij} (\mathbf{e}'_m \cdot \mathbf{e}_i) (\mathbf{e}_j \cdot \mathbf{e}'_n) \quad \text{or } \mathbf{T}' = \mathbf{L} \mathbf{T} \mathbf{L}^T$$

$$\sigma'_{mn} = T_{ij} l_{mi} l_{nj}$$

10/13/28



if we want component of a along n

take $a \cdot n = a_n$

$a \cdot n$ is max when $a \parallel n$
min when $a \perp n$

ask what about σ

we know that $t_n = \sigma \cdot n$; The stress tensor on the plane whose normal is n
lets look $\tau_{nn} = n \cdot t_n = n \cdot \sigma \cdot n$ how does it vary (this is the component of t_n in n dir)

- look at $\tau_{nn} = \tau_{nn}(n)$ then $d\tau_{nn} = dn \cdot (\sigma \cdot n) + n \cdot (\sigma \cdot dn)$

- note that since σ is invariant $\Rightarrow d\sigma = 0$

- since σ is symmetric then $d\tau_{nn} = 2dn \cdot \sigma \cdot n$

for a maximum set $d\tau_{nn} = 2dn \cdot (\sigma \cdot n) = 0$

since $\sigma \cdot n = t_n \Rightarrow dn \cdot t_n = 0 \therefore t_n$ must be $\perp dn$

hence t_n must be $\parallel n$

since n is unit vector can only change in direction $\nabla n \perp n$

now $\tau_{nn} = \sigma_{ij} n_i n_j$ & let $\lambda =$ a parameter; since $t_n \parallel n$ then $t_n = \lambda n$

$$d\tau_{nn} = \sigma_{ij} dn_i n_j + \sigma_{ij} n_i dn_j = 2\sigma_{ij} dn_j n_i$$

since $\sigma \cdot n = t_n = \lambda n \Rightarrow \sigma_{ij} n_i = \lambda n_j$

let $\lambda n = \lambda \hat{I} \cdot n$ or the Kronecker δ

$$\therefore \sigma_{ij} n_i = \lambda \delta_{ij} n_j$$

$$\therefore (\sigma_{ij} - \lambda \delta_{ij}) n_i = 0 \quad \text{or} \quad (\sigma - \lambda \hat{I}) \cdot n = 0$$

$$(\sigma_{xx} - \lambda) n_x + \sigma_{yx} n_y + \sigma_{zx} n_z = 0$$

$$\sigma_{xy} n_x + (\sigma_{yy} - \lambda) n_y + \sigma_{zy} n_z = 0$$

$$\sigma_{xz} n_x + \sigma_{yz} n_y + (\sigma_{zz} - \lambda) n_z = 0$$

} for a non trivial solution is that $\det = 0$

$$\begin{vmatrix} \sigma_{xx} - \lambda & \sigma_{yx} & \sigma_{zx} \\ \sigma_{xy} & \sigma_{yy} - \lambda & \sigma_{zy} \\ \sigma_{xz} & \sigma_{yz} & \sigma_{zz} - \lambda \end{vmatrix} = 0$$

Next hw assign expand this and get the cubic

$$-\lambda^3 + I\lambda^2 - II\lambda + III = 0$$

$$I = \sigma_{kk} = \sigma_{xx} + \sigma_{yy} + \sigma_{zz}$$

$$II = \frac{1}{2} \epsilon_{mik} \epsilon_{njl} \sigma_{ij} \sigma_{kl} \delta_{mn} = \sigma_{xx}\sigma_{yy} + \sigma_{yy}\sigma_{zz} + \sigma_{zz}\sigma_{xx} - \sigma_{xy}^2 - \sigma_{yz}^2 - \sigma_{zx}^2$$

$$III = \frac{1}{6} \epsilon_{mik} \epsilon_{njl} \sigma_{ij} \sigma_{kl} \sigma_{mn} = \sigma_{xx}\sigma_{yy}\sigma_{zz} + 2\sigma_{xy}\sigma_{yz}\sigma_{zx} - \sigma_{xx}\sigma_{yz}^2 - \sigma_{yy}\sigma_{xz}^2 - \sigma_{zz}\sigma_{yx}^2$$

these are stress invariants

Next time we will show that all λ are Re and $\lambda > 0$ and will be in 3 $\perp$ directions.
 and that I, II, III are stress invariants.

10/16/78

6 Nov - midterm 1 hr. open notes

we were working with $\sigma \cdot n = \lambda n$

$$\sigma_{ij} n_j = \lambda n_i = \lambda \delta_{ij} n_j \quad \text{or} \quad (\sigma_{ij} - \lambda \delta_{ij}) n_j = 0$$

$$\text{for non triv solution } \det |\sigma_{ij} - \lambda \delta_{ij}| = 0 \Rightarrow -\lambda^3 + I\lambda^2 - II\lambda + III = 0$$

We will now prove that $\lambda^{(k)}$ are real (obvious for a symmetric dyadic).

Solutions will be $\lambda^{(1)}, \lambda^{(2)}, \lambda^{(3)}$; for each $\lambda^{(k)}$ we can associate $n^{(k)}$

$$\text{Assume } \lambda^{(k)} = \alpha^{(k)} + i\beta^{(k)} \quad \text{ie } \lambda^{(k)} \text{ is complex}$$

$$n^{(k)} = a^{(k)} + i b^{(k)} \quad \text{ie } n^{(k)} \text{ is complex}$$

$$n^{(k)} \cdot \sigma = \lambda^{(k)} n^{(k)} \quad \text{or} \quad (a^{(k)} + i b^{(k)}) \cdot \sigma = (\alpha^{(k)} + i \beta^{(k)}) (a^{(k)} + i b^{(k)})$$

$$a^{(k)} \cdot \sigma + i b^{(k)} \cdot \sigma = \alpha^{(k)} a^{(k)} + i [\alpha^{(k)} b^{(k)} + \beta^{(k)} a^{(k)}] - \beta^{(k)} b^{(k)}$$

1. equate real & imaginary parts

2. mult $a^{(k)} \cdot \sigma$ by $b^{(k)}$ and then $b^{(k)} \cdot \sigma$ by $a^{(k)}$ and look at real part of $n^{(k)} \cdot \sigma$

$$a^{(k)} \cdot \sigma \cdot b^{(k)} = \alpha^{(k)} a^{(k)} \cdot b^{(k)} - \beta^{(k)} b^{(k)} \cdot b^{(k)}$$

$$\text{Same since } \sigma \text{ is symmetric } b^{(k)} \cdot \sigma \cdot a^{(k)} = a^{(k)} \beta^{(k)} \cdot a^{(k)} + b^{(k)} \alpha^{(k)} \cdot a^{(k)}$$

$$\text{Now subtract 1st from 2nd} \Rightarrow 0 = \beta^{(k)} [a^{(k)} \cdot a^{(k)} + b^{(k)} \cdot b^{(k)}] = \beta^{(k)} n^{(k)} \cdot n^{(k)}$$

$$= \beta^{(k)} \cdot 1 \quad \text{since magnitude } n = 1$$

$\therefore \beta^{(k)} = 0$ & $\lambda^{(k)}$ must therefore be real.

Look at eigenvector $n^{(1)}, \sigma = \lambda^{(1)} n^{(1)}, n^{(2)}, \sigma = \lambda^{(2)} n^{(2)}$

$$n^{(1)} \cdot \sigma \cdot n^{(2)} = \lambda^{(1)} n^{(1)} \cdot n^{(2)} \quad \text{also} \quad n^{(2)} \cdot \sigma \cdot n^{(1)} = \lambda^{(2)} n^{(2)} \cdot n^{(1)}$$

Since σ is symmetric $n^{(1)} \cdot \sigma \cdot n^{(2)} = n^{(2)} \cdot \sigma \cdot n^{(1)} = 0$

$$\nmid n^{(1)} \cdot n^{(2)} [\lambda^{(1)} - \lambda^{(2)}] = 0$$

if $\lambda^{(i)} \neq \lambda^{(k)} \Rightarrow n^{(i)} \perp n^{(k)}$

if $\lambda^{(i)} = \lambda^{(k)}$ we can no longer say that $n^{(i)} \perp n^{(k)}$

but in general $\lambda^{(i)} \neq \lambda^{(k)}$

$n^{(1)}, n^{(2)}, n^{(3)}$ are perpendicular: only the normal components exist and the shear components are zero on planes $\perp$ to principal directions at every point we can define the principal axes along $n^{(1)}, n^{(2)}$ and $n^{(3)}$. The stress tensor along $n^{(1,2,3)}$

is given by

$$\begin{pmatrix} \lambda^{(1)} & 0 & 0 \\ 0 & \lambda^{(2)} & 0 \\ 0 & 0 & \lambda^{(3)} \end{pmatrix}$$

if all 3 λ are equal we have hydrostatic pressure state.

TO OBTAIN THE EIGENVECTORS

Look for solutions of $n^{(k)}$ where $n_x^{(k)2} + n_y^{(k)2} + n_z^{(k)2} = 1$

$$(\sigma_{xx} - \lambda^{(k)}) n_x^{(k)} + \sigma_{xy} n_y^{(k)} = -\sigma_{xz} n_z^{(k)}$$

$$\sigma_{yx} n_x^{(k)} + (\sigma_{yy} - \lambda^{(k)}) n_y^{(k)} = -\sigma_{yz} n_z^{(k)}$$

$$n_x^{(k)} = \frac{n_z^{(k)} \begin{vmatrix} -\sigma_{xz} & \sigma_{xy} \\ -\sigma_{yz} & \sigma_{yy} - \lambda^{(k)} \end{vmatrix}}{\begin{vmatrix} \sigma_{xx} - \lambda^{(k)} & \sigma_{xy} \\ \sigma_{yx} & \sigma_{yy} - \lambda^{(k)} \end{vmatrix}}} = \frac{D_x n_z^{(k)}}{D}$$

$$n_y^{(k)} = \frac{n_z^{(k)} \begin{vmatrix} \sigma_{xx} - \lambda^{(k)} & -\sigma_{xz} \\ -\sigma_{yx} & -\sigma_{yz} \end{vmatrix}}{D} = \frac{D_y n_z^{(k)}}{D}$$

$$\text{put into } n_x^2 + n_y^2 + n_z^2 = 1 \Rightarrow n_z^{(k)2} \left[\frac{D_x^2}{D^2} + \frac{D_y^2}{D^2} + 1 \right] = 1$$

$$\therefore n_z^{(k)} = \pm \sqrt{\frac{D^2}{D_x^2 + D_y^2 + D^2}}$$

since D_x, D_y, D are real $\Rightarrow n_x^{(k)}, n_y^{(k)}, n_z^{(k)}$ are real

Geometric representation of Stress

$t_n = \sigma \cdot n$ this is the stress tensor on the surface whose normal is n

$\sigma_{nn} = n \cdot t_n = (n \cdot \sigma \cdot n)$ this is the normal component of t_n

let $N \equiv \sigma_{nn} = \sigma_{ij} n_i n_j$

introduce a local coordinate system w/ origin at the point P we are at and define $A \parallel n$ and let $X_i = A n_i$ (ie A starts at the pt we're looking at)

$NA^2 = \sigma_{ij} X_i X_j$ defines a quadratic surface and if we restrict A to have its tip lie on the surface then

$$\sigma_{ij} X_i X_j = \pm k^2 \Rightarrow N = \pm \frac{k^2}{A^2} \quad \text{this is Cauchy's stress quadric}$$

Since $A^2 > 0 \Rightarrow$ if $N > 0$ choose + sign.

The Reciprocal surface (Lamé's stress direction quadric)

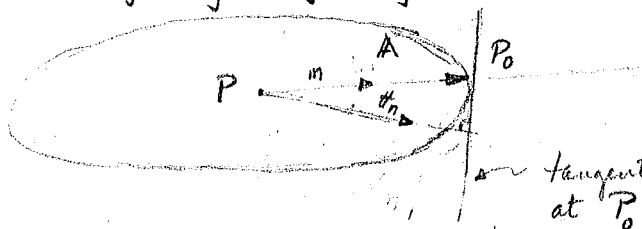
$$\sigma_{ij}^{-1} X_i X_j = \pm k^2 \quad \text{where } \sigma_{ij}^{-1} \sigma_{jk} = \delta_{ik} \quad \text{or } \sigma^{-1} \cdot \sigma = \hat{I}$$

now $|t_n|^2 = t_n \cdot t_n = n \cdot \sigma \cdot \sigma \cdot n = (n \cdot \sigma^2 \cdot n)$

if $n \cdot \sigma^2 \cdot n = k^2$ ellipsoid (Cauchy's stress ellipsoid)

$n \cdot (\sigma^{-1})^2 \cdot n = k^2$ Lamé's stress ellipsoid

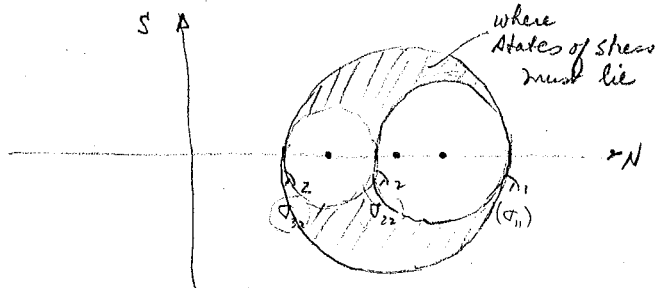
Note that $\sigma_{ij} n_i n_j = \sigma_{ij} n'_i n'_j = \pm k^2$ (invariant)



tangent plane to stress quadric at P_0 . t_n lies on line $\perp$ to tangent plane and P

10/8/78

TA T 3-5; W 1230-230 Rich King



Mohr's Circle

$$S_{max} = \frac{\sigma_1 - \sigma_3}{2} \quad \text{where } N = \frac{\sigma_1 + \sigma_3}{2}$$

$$N_{max} = \sigma_1, \quad N_{min} = \sigma_2, \quad S = 0$$

Examples of States of Stress

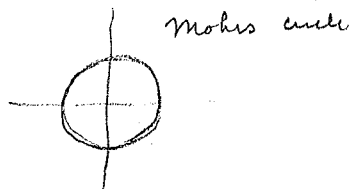
suppose $\sigma_{11} = \sigma_{22} = \sigma_{33}$

HYDROSTATIC or ISOTROPIC state
(properties are independent of direction)

stress ellipsoid $\Rightarrow$ a sphere.

Simple tension (or compression) $\sigma_{11} \neq 0$ $\sigma_{22} = \sigma_{33} = 0$

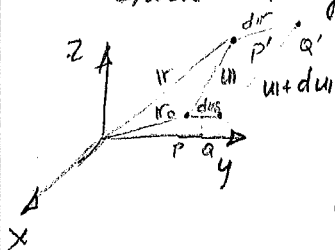
pure shear principle stresses $\sigma_{33} = 0$, $\sigma_{22} = -\sigma_{11}$



Plane stress $\sigma_{33} = 0$

Analysis of Deformation

Given a body w/a pt before deformation & then after



dr_0 is infinitesimal with respect to r_0

u is displ vector of P to P'

$u + du$ is change in displ of Q to Q'

given that $P(x_0, y_0, z_0)$ & $P'(x, y, z)$ then

$$r_0 = x_0 i + y_0 j + z_0 k \quad r = x i + y j + z k$$

note vectorially $u + dr = dr_0 + u + du$ and that $dr = dr_0 + du$

remember $\nabla \phi = e_j \frac{\partial \phi}{\partial x_j}$ and $dr = dx_k e_k$

$$\text{hence } dr \cdot \nabla \phi = \delta_{jk} \frac{\partial \phi}{\partial x_j} dx_k = \frac{\partial \phi}{\partial x_j} dx_j = d\phi$$

thus $d\phi = dr \cdot \nabla \phi$ differential of pt fn = inner product of ∇ and differential of position vector

$$\text{also } \nabla u = \left(e_j \frac{\partial}{\partial x_j} \right) (e_k u_k) = e_j e_k \frac{\partial u_k}{\partial x_j} \quad \text{second rank tensor}$$

$$\begin{aligned} dr \cdot \nabla u &= (e_j dx_j) \cdot \left(e_k e_l \frac{\partial u_l}{\partial x_k} \right) = \delta_{jk} e_l \frac{\partial u_l}{\partial x_k} dx_j = e_l \frac{\partial u_l}{\partial x_k} dx_k \\ &= e_l du_l = du \end{aligned}$$

$$du = (\text{dir} \cdot \nabla) u$$

∇u is the displacement gradient; second rank tensor which is not symmetric [ie $\frac{\partial u_k}{\partial x_j} e_j \cdot e_k \neq \frac{\partial u_j}{\partial x_k} e_k \cdot e_j$ in general]

now substitute into $\text{dir}_0 + du = \text{dir}$ or $\boxed{\text{dir}_0 = \text{dir} - du = \text{dir} \cdot [\hat{I} - \nabla u]}$

related undeformed state to deformed state

Eulerian Deformation tensor.

$$\hat{I} - \nabla u = \delta_{ij} - \frac{\partial u_j}{\partial x_i}$$

since ∇ operation is first $\Rightarrow$ 1st subscript of $\frac{\partial u_j}{\partial x_i}$ must be associated with differentiation

Conjugate dyadic of a to b is $b a$ or $(a b)_c = b a$

$$(\nabla u)_c = \text{Conjugate dyadic of } \nabla u = u \nabla = (e_i u_i) (e_j \frac{\partial}{\partial x_j}) = e_i e_j \frac{\partial u_i}{\partial x_j}$$

one can show that $du = (u \nabla) \cdot \text{dir}$ $du_c = du = (\text{dir})_c \cdot (\nabla u)_c = (\nabla u)_c \cdot (\text{dir})_c = (u \nabla) \cdot (\text{dir})_c = u \nabla \cdot \text{dir}$

$$\therefore \text{dir} = \text{dir}_0 + (u \nabla) \cdot \text{dir} ; \quad \boxed{\text{dir}_0 = \text{dir} - (u \nabla) \cdot \text{dir} = [\hat{I} - u \nabla] \cdot \text{dir}}$$

If we define differentiation wrt fixed reference state then

$$\nabla_0 = e_{x_0} \frac{\partial}{\partial x_0} + e_{y_0} \frac{\partial}{\partial y_0} + e_{z_0} \frac{\partial}{\partial z_0}$$

$$\therefore \text{dir}_0 \cdot \nabla_0(\cdot) = d_0(\cdot) \Rightarrow \text{dir}_0 \cdot \nabla_0 \phi = d_0 \phi$$

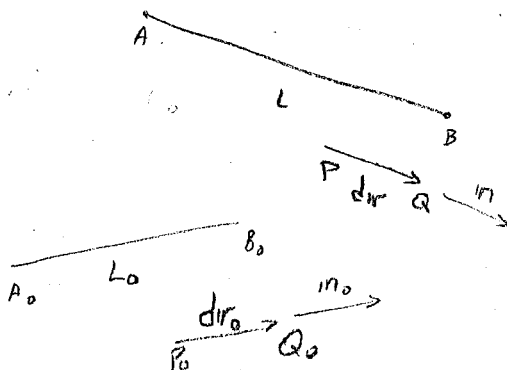
$$\therefore \text{dir} = \text{dir}_0 + du = \text{dir}_0 \cdot (\hat{I} + \nabla_0 u) = \text{dir}_0 + (\text{dir}_0 \cdot \nabla) u$$

$$\boxed{\text{dir} = (\hat{I} + u \nabla_0) \cdot \text{dir}_0 = \text{dir}_0 \cdot (\hat{I} + \nabla_0 u)} \quad \text{Lagrangian deformation tensor}$$

$$= \text{dir}_0 \cdot \left[\delta_{ij} + \frac{\partial u_j}{\partial x_{0i}} \right] e_i e_j$$

related deformed state to undeformed state

10/20/78



Eulerian stretch

$$\eta_L = \frac{L^2 - L_0^2}{2L^2}$$

Lagrangian stretch

$$\epsilon_{LL} = \frac{L^2 - L_0^2}{2L_0^2}$$

$$\therefore \eta_L = \left(\frac{L_0}{L} \right)^2 \epsilon_{LL}$$

Strength of materials definition of elongation

$$E_L = \frac{L - L_0}{L_0} = \frac{\Delta L}{L_0} \quad \text{if } \frac{\Delta L}{L_0} \ll 1 \quad \text{then } \eta_u = \epsilon_u = E_u$$

$$\eta_u = \frac{(L_0 + \Delta L)^2 - L_0^2}{2(L_0 + \Delta L)^2} = \frac{L_0^2 + 2\Delta L L_0 + \Delta L^2 - L_0^2}{2(L_0 + \Delta L)^2} = \frac{2(\frac{\Delta L}{L_0}) + (\frac{\Delta L}{L_0})^2}{2(1 + 2\frac{\Delta L}{L_0} + (\frac{\Delta L}{L_0})^2)} \approx \frac{2\frac{\Delta L}{L_0}}{2} = \frac{\Delta L}{L_0}$$

if we neglect all second order terms then

$$\eta_{rr} = \frac{\mathbf{dir} \cdot \mathbf{dir} - \mathbf{dir}_0 \cdot \mathbf{dir}_0}{2 \mathbf{dir} \cdot \mathbf{dir}} = \frac{\mathbf{dir} \cdot \mathbf{dir} - \mathbf{dir} \cdot (\hat{\mathbf{I}} - \nabla u_i) \cdot (\hat{\mathbf{I}} - u_i \nabla) \cdot \mathbf{dir}}{2 \mathbf{dir} \cdot \mathbf{dir}}$$

$$(\hat{\mathbf{I}} - \nabla u_i) \cdot (\hat{\mathbf{I}} - u_i \nabla) = \hat{\mathbf{I}} - \nabla u_i - u_i \nabla + \nabla u_i \cdot u_i \nabla$$

$$\mathbf{dir} \cdot \mathbf{dir} = \mathbf{dir} \cdot \hat{\mathbf{I}} \cdot \mathbf{dir}$$

$$\therefore \eta_{rr} = \frac{\mathbf{dir} \cdot (\nabla u_i + u_i \nabla - \nabla u_i \cdot u_i \nabla) \cdot \mathbf{dir}}{2 |\mathbf{dir}|^2}$$

note $\frac{\mathbf{dir}}{|\mathbf{dir}|} = \mathbf{n}$ unit vector in the dir direction

$$\eta_{rr} = \mathbf{n} \cdot \frac{1}{2} (\nabla u_i + u_i \nabla - \nabla u_i \cdot u_i \nabla) \cdot \mathbf{n} = \mathbf{n} \cdot \Phi \cdot \mathbf{n} \quad \text{defines change in length}$$

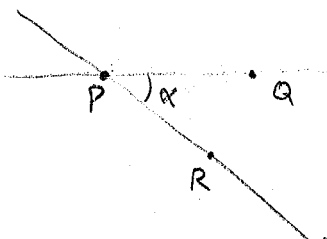
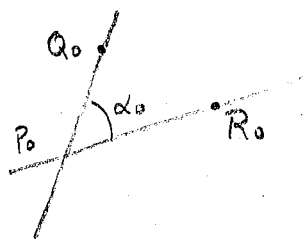
$$\frac{1}{2} (\nabla u_i + u_i \nabla - \nabla u_i \cdot u_i \nabla) = \Phi$$

$$\nabla u_i \cdot u_i \nabla = \frac{\partial u_i}{\partial x_k} u_j e_i e_j \cdot \frac{\partial u_k}{\partial x_l} e_k e_l = \frac{\partial u_j}{\partial x_i} \frac{\partial u_k}{\partial x_l} e_i e_l \delta_{jk} = \frac{\partial u_j}{\partial x_i} \frac{\partial u_j}{\partial x_l} e_i e_l$$

$$\Phi_{ij} = \frac{1}{2} \left(\frac{\partial u_j}{\partial x_i} + \frac{\partial u_i}{\partial x_j} - \frac{\partial u_k}{\partial x_i} \frac{\partial u_k}{\partial x_j} \right)$$

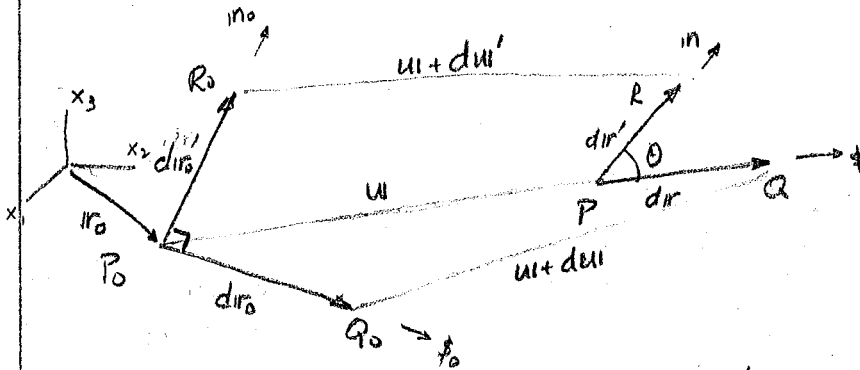
Eulerian Stretch Tensor

$$\Phi = e_i \Phi_{ij} e_j$$



look at angles that change w/ deformation

and let us restrict our attention to angles that were initially $\perp$



$$\frac{1}{2} \cos \theta = \frac{1}{2} \frac{dir' \cdot dir}{|dir'| |dir|}$$

$$dir' = dir_0' + du1'$$

$$du1' = dir' \cdot (\nabla u1)$$

$$dir = dir_0 + du1$$

$$du1 = (u1 \nabla) \cdot dir$$

$$dir' \cdot dir = [dir_0' + dir' \cdot (\nabla u1)] \cdot [dir_0 + u1 \nabla \cdot dir]$$

note $dir_0' \cdot dir_0 = 0$
since $dir_0' \perp dir_0$

$$= dir_0' \cdot u1 \nabla \cdot dir + dir' \cdot (\nabla u1) \cdot dir_0 + (dir' \cdot \nabla u1) \cdot (u1 \nabla \cdot dir)$$

now substitute for dir_0' and dir_0 in terms of dir' and dir i.e. $dir_0' = dir' - du1'$ and $dir_0 = dir - du1$

$$= dir' \cdot [\nabla u1 + u1 \nabla - \nabla u1 \cdot u1 \nabla] \cdot dir$$

now let $m = \frac{dir'}{|dir'|}$ and $s = \frac{dir}{|dir|}$

$$\therefore \frac{1}{2} \cos \theta = m \cdot \Phi \cdot s$$

defined change in angle

Φ contains not only changes in length but also changes in angle

to find what has happened to two $\perp$ lines in space, find the two unit vectors along the two $\perp$ lines m_1, m_2 then take $m_1 \cdot \Phi \cdot m_2$ is Φ symmetric? yes even if $\nabla u1$ is asymmetric then $\nabla u1 + u1 \nabla$ is symmetric and $u1 \nabla \cdot \nabla u1$ is symmetric; therefore Φ is symmetric

$$\text{Lagrangian strain tensor } \left[\Phi = \frac{1}{2} \left[\nabla_0 u1 + u1 \nabla_0 + \nabla_0 u1 \cdot u1 \nabla_0 \right] \right]$$

$$\epsilon_{ij} = \frac{1}{2} \left(\frac{\partial u_j}{\partial x_{0i}} + \frac{\partial u_i}{\partial x_{0j}} + \frac{\partial u_k}{\partial x_{0i}} \frac{\partial u_k}{\partial x_{0j}} \right) e_i \cdot e_j$$

$$\text{and } \Phi = e_i \cdot \epsilon_{ij} e_j$$

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What are the corresponding Lagrangian representation

$$d\mathbf{r}' = d\mathbf{r}_0 \cdot (\hat{\mathbf{I}} + \nabla_0 \mathbf{u})$$

$$d\mathbf{r} = (\hat{\mathbf{I}} + \mathbf{u} \nabla_0) \cdot d\mathbf{r}_0$$

$$\frac{1}{2} \cos \theta = \frac{\frac{1}{2} d\mathbf{r}'_0 \cdot (\hat{\mathbf{I}} + \nabla_0 \mathbf{u}) \cdot (\hat{\mathbf{I}} + \mathbf{u} \nabla_0) \cdot d\mathbf{r}_0}{|d\mathbf{r}'| |d\mathbf{r}|} = \frac{d\mathbf{r}'_0 \cdot d\mathbf{r}_0}{2 |d\mathbf{r}'| |d\mathbf{r}|}$$

recall that $d\mathbf{r}'_0 \cdot d\mathbf{r}_0 = 0$

$$\frac{1}{2} \cos \theta = \frac{|d\mathbf{r}'_0|}{|d\mathbf{r}'|} \cdot \Phi \cdot \frac{d\mathbf{r}_0}{|d\mathbf{r}_0|} \quad \text{where} \quad \Phi \equiv \frac{1}{2} [\nabla_0 \mathbf{u} + \mathbf{u} \nabla_0 + \nabla_0 \mathbf{u}_1 \cdot \mathbf{u} \nabla_0]$$

Lagrangian deformation tensor

Recall

$$\epsilon_{rr} = \frac{d\mathbf{r} \cdot d\mathbf{r} - d\mathbf{r}_0 \cdot d\mathbf{r}_0}{2 |d\mathbf{r}_0| |d\mathbf{r}_0|} \quad \text{thus this will lead} \quad \epsilon_{rr} = \frac{d\mathbf{r}_0}{|d\mathbf{r}_0|} \cdot \Phi \cdot \frac{d\mathbf{r}_0}{|d\mathbf{r}_0|}$$

now if $\mathbf{m}_0 = \frac{d\mathbf{r}_0}{|d\mathbf{r}_0|}$ then $\boxed{\epsilon_{rr} = \mathbf{m}_0 \cdot \Phi \cdot \mathbf{m}_0}$ this gives changes in length of line in direction of $d\mathbf{r}_0$

Common definition of elongation $\epsilon_{LL} = \sqrt{1 + 2\epsilon_{LL}} - 1 = \frac{L - L_0}{L_0}$ for Lagrangian

$$\frac{|d\mathbf{r}|}{|d\mathbf{r}_0|} = \epsilon_{rr} + 1 = \sqrt{1 + 2\epsilon_{rr}}$$

$$\frac{|d\mathbf{r}_0|}{|d\mathbf{r}|} = \frac{1}{\sqrt{1 + 2\epsilon_{rr}}}$$

$$\frac{|d\mathbf{r}'|}{|d\mathbf{r}'_0|} = \sqrt{1 + 2\epsilon_{rr'}}$$

$$\frac{|d\mathbf{r}'_0|}{|d\mathbf{r}'|} = \frac{1}{\sqrt{1 + 2\epsilon_{rr'}}}$$

now define $\frac{d\mathbf{r}_0}{|d\mathbf{r}_0|} = \Phi_0$ $\frac{d\mathbf{r}'_0}{|d\mathbf{r}'_0|} = \mathbf{m}_0$; Since $\frac{1}{2} \cos \theta = \frac{d\mathbf{r}'_0}{|d\mathbf{r}'_0|} \cdot \Phi \cdot \frac{d\mathbf{r}_0}{|d\mathbf{r}_0|} = \frac{d\mathbf{r}'_0}{|d\mathbf{r}'_0|} \cdot \Phi \cdot \frac{d\mathbf{r}_0}{|d\mathbf{r}_0|}$

Then $\frac{1}{2} \cos \theta = \frac{\mathbf{m}_0 \cdot \Phi \cdot \Phi_0}{\sqrt{1 + 2\epsilon_{s_0 s_0}} \sqrt{1 + 2\epsilon_{n_0 n_0}}} = \frac{\epsilon_{n_0 s_0}}{\sqrt{(1 + 2\epsilon_{s_0 s_0})(1 + 2\epsilon_{n_0 n_0})}}$ this gives the angular strain

$$\boxed{\epsilon_{ij} = \frac{1}{2} \left[\frac{\partial u_j}{\partial x_{i_0}} + \frac{\partial u_i}{\partial x_{j_0}} + \frac{\partial u_k}{\partial x_{i_0}} \frac{\partial u_k}{\partial x_{j_0}} \right]} \quad \text{Lagrangian stretch tensor}$$

Look at Volume changes using the Lagrangian method for principal direction

$$dx_i = dx_{0i} (1 + 2\epsilon_{ii}) \quad \text{for principal direction.}$$

$$dV = dV_0 \sqrt{(1+2\epsilon_{11})(1+2\epsilon_{22})(1+2\epsilon_{33})}$$

Define the dilatation $\Delta = \frac{\delta V - \delta V_0}{\delta V_0} = \sqrt{(1+2\epsilon_{11})(1+2\epsilon_{22})(1+2\epsilon_{33})} - 1$

$$\Delta = \sqrt{1 + 2I_E + 4II_E + 8III_E} - 1 \quad \text{where } I_E, II_E, III_E \text{ are the Lagrangian invariants}$$

for small strains $\sqrt{1+K} \approx 1 + \frac{1}{2}K \sim 1 + I_E = \frac{\delta u_i}{\delta x_i} + 1$

Define $\mathbb{E} = \frac{1}{2} (\nabla_0 u_i + u_i \nabla_0)$ symmetric part $\epsilon_{ij} = \frac{1}{2} \left(\frac{\partial u_j}{\partial x_i} + \frac{\partial u_i}{\partial x_j} \right)$ extension

$\omega = \frac{1}{2} (\nabla_0 u_i - u_i \nabla_0)$ anti symmetric part $\omega_{ij} = \frac{1}{2} \left(\frac{\partial u_j}{\partial x_i} - \frac{\partial u_i}{\partial x_j} \right)$ rotation of rigid body

$$\epsilon_{xx} = \frac{\partial u_x}{\partial x_0} \quad \epsilon_{yz} = \frac{1}{2} \left(\frac{\partial u_z}{\partial y_0} + \frac{\partial u_y}{\partial z_0} \right)$$

$$\omega_{xx} = 0 \quad \omega_{yz} = \frac{1}{2} \left(\frac{\partial u_z}{\partial y_0} - \frac{\partial u_y}{\partial z_0} \right) \quad \omega = \begin{pmatrix} 0 & \omega_{xy} & \omega_{xz} \\ -\omega_{xy} & 0 & \omega_{yz} \\ -\omega_{xz} & \omega_{yz} & 0 \end{pmatrix}$$

ROTATION MATRIX

note $\epsilon_{ij} = \epsilon_{ji} \quad \omega_{ij} = -\omega_{ji} \quad \text{and} \quad \nabla_0 u_i = \mathbb{E} + \omega$
 $u_i \nabla_0 = \mathbb{E} - \omega$

$$\mathbb{E} = \frac{1}{2} \left((\mathbb{E} + \omega) + (\mathbb{E} - \omega) + \mathbb{E} \cdot \mathbb{E} + \omega \cdot \mathbb{E} - \mathbb{E} \cdot \omega - \omega \cdot \omega \right)$$

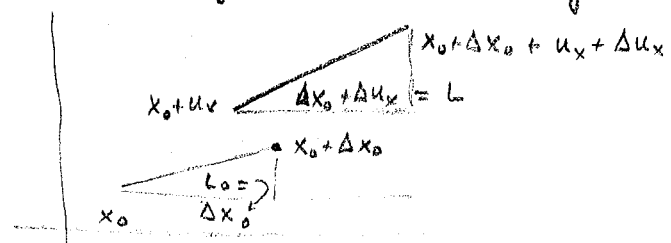
Lagrangian

$$\mathbb{E} = \mathbb{E} + \frac{1}{2} (\mathbb{E} \cdot \mathbb{E} + \omega \cdot \mathbb{E} - \mathbb{E} \cdot \omega - \omega \cdot \omega)$$

if we are in the realm of infinitesimal elasticity $\Rightarrow (\mathbb{E}, \omega \ll 1)$ then $\mathbb{E} \approx \mathbb{E}$
 and we won't need to make distinctions between Eulerian and Lagrangian extensional tensors

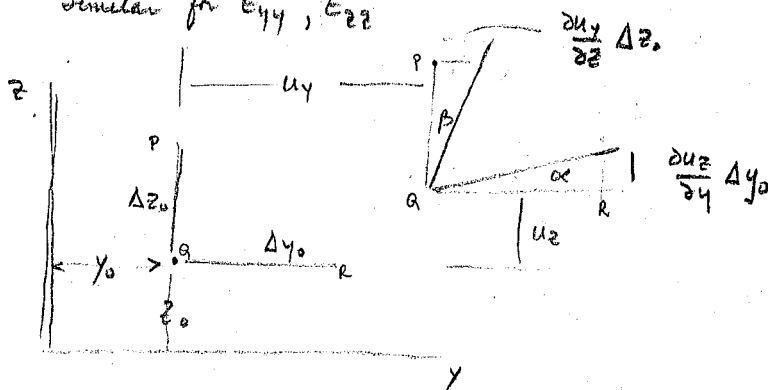
Geometric Significance of Quantities

Look at a single strand that undergoes extension & rotation



$$\epsilon_{xx} = \frac{\partial u_x}{\partial x_0} \approx \frac{\Delta u_x}{\Delta x_0} = \frac{(x_0 + \Delta x_0 + u_x + \Delta u_x) - (x_0 + \Delta u_x) - (\Delta x_0)}{\Delta x_0} = \frac{\Delta x_0 + \Delta u_x - \Delta x_0}{\Delta x_0} = \frac{L - L_0}{L_0}$$

Similar for $\epsilon_{yy}, \epsilon_{zz}$



If we assume small strains, rotations $\epsilon_{yy} \ll 1, \alpha \ll 1, \beta \ll 1, \epsilon_{zz} \ll 1$

$$\tan \alpha \approx \alpha = \frac{\partial u_z}{\partial y} \quad \tan \beta \approx \beta = \frac{\partial u_y}{\partial z}$$

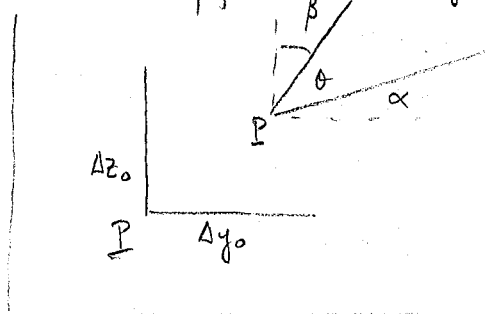
$$\cos \theta = \cos \left[\frac{\pi}{2} - (\alpha + \beta) \right] = \sin(\alpha + \beta) \approx \alpha + \beta = \frac{\partial u_z}{\partial y} + \frac{\partial u_y}{\partial z}$$

$$= 2 \epsilon_{yz} \quad \text{engineering strain } \delta y_z$$

10/25/78

So far we have made no constraints on the body (ie elastic) except to say that the body must be continuous.

We have noted that $\nabla u = (\epsilon + \omega)$ contained on its main diagonals the extensional changes for small ~~deformation~~ and on the off diag terms we have the angular changes in the body for small ~~deformation~~.



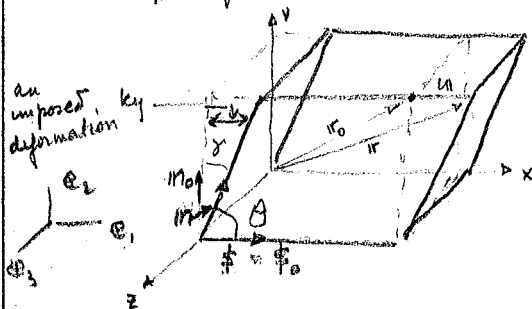
$$\frac{1}{2} \cos \theta = \frac{\alpha + \beta}{2} = \frac{1}{2} \left(\frac{\partial u_z}{\partial y} + \frac{\partial u_y}{\partial z} \right) = \epsilon_{yz}$$

$\omega_{yz} = \frac{1}{2} \left(\frac{\partial u_z}{\partial y} - \frac{\partial u_y}{\partial z} \right) = \frac{1}{2} (\alpha - \beta)$
represents an average rotation of all lines passing through point P (ω_{ij} represents rigid body rotation only).

We can then say that for small strains (infinitesimal deformation)
 $\Phi \Rightarrow \epsilon \Rightarrow \epsilon$

Dilatation $\Delta = \frac{\partial u_x}{\partial x} + \frac{\partial u_y}{\partial y} + \frac{\partial u_z}{\partial z} = u_{k,k} = \epsilon_{kk} = I_\epsilon$

Example of Finite Strains


 u_x

$$u = ky e_1 + 0 + 0$$

$$r = x e_1 + y e_2 + z e_3$$

$$r_0 = x_0 e_1 + y_0 e_2 + z_0 e_3$$

$$r = r_0 + u$$

$$dr = dr_0 + du$$

$$\begin{aligned} x &= x_0 + ky & y &= y_0, z = z_0 \\ dx &= dx_0 + \left[\frac{\partial u_x}{\partial x} dx + \frac{\partial u_x}{\partial y} dy + \frac{\partial u_x}{\partial z} dz \right] + 0 \text{ from other terms} \\ &= dx_0 + k dy \end{aligned}$$

$$\frac{\partial u_i}{\partial x_j} = \nabla u = k e_y e_x = \begin{Bmatrix} 0 & k & 0 \\ 0 & 0 & 0 \\ 0 & 0 & 0 \end{Bmatrix}$$

Eulerian Deformation Tensor

$$\hat{I} - \nabla u = \begin{Bmatrix} 1 & -k & 0 \\ 0 & 1 & 0 \\ 0 & 0 & 1 \end{Bmatrix}$$

$$= e_x e_x - k e_y e_x + e_y e_y + e_z e_z$$

$$u \nabla = k e_x e_y$$

Note that $\frac{\partial}{\partial y} = \frac{\partial}{\partial y_0}$, $\frac{\partial}{\partial z} = \frac{\partial}{\partial z_0}$

Lagrangian Deformation Tensor (in this example $\nabla u = \nabla_0 u$) since $e_x = e_{x_0}$, $e_y = e_{y_0}$

$$\hat{I} + \nabla_0 u = \begin{Bmatrix} 1 & k & 0 \\ 0 & 1 & 0 \\ 0 & 0 & 1 \end{Bmatrix}$$

Eulerian strain tensor,

$$\Phi = \frac{1}{2} (\nabla u + u \nabla - \nabla u \cdot u \nabla) = \begin{Bmatrix} 0 & \frac{k}{2} & 0 \\ \frac{k}{2} & -\frac{k^2}{2} & 0 \\ 0 & 0 & 0 \end{Bmatrix}$$

$$\Phi = \frac{k}{2} e_y e_x + \frac{k}{2} e_x e_y - \frac{k^2}{2} e_y e_y$$

$$\text{Lagrangian Strain tensor } \Phi = \frac{1}{2} (\nabla_0 u + u \nabla_0 + \nabla_0 u \cdot u \nabla_0) = \begin{Bmatrix} 0 & \frac{k}{2} & 0 \\ \frac{k}{2} & \frac{k^2}{2} & 0 \\ 0 & 0 & 0 \end{Bmatrix}$$

$$\Phi = \frac{k}{2} e_{y_0} e_{x_0} + \frac{k}{2} e_{x_0} e_{y_0} + \frac{k^2}{2} e_{y_0} e_{y_0}$$

$$\text{Now } \frac{1}{2} \cos \theta = m \cdot \Phi \cdot \Phi \quad \text{where } m = \sin \gamma e_1 + \cos \gamma e_2$$

$$\Phi = e_1$$

$$m \cdot \Phi \cdot \Phi$$

$$\frac{1}{2} \cos \theta = \cos \gamma \frac{k}{2}; \text{ but } \tan \gamma = k \text{ (from sketch)} \quad \text{Thus}$$

$$= \cos \gamma \cdot \frac{1}{2} \tan \gamma = \frac{1}{2} \sin \gamma$$

$$\frac{1}{2} \cos \theta = \frac{n_0 \cdot \phi \cdot \phi_0}{\sqrt{(1+2\epsilon_{s_0 s_0})(1+2\epsilon_{n_0 n_0})}}$$

Infinitesimal $\phi = \frac{1}{2} (\nabla u + u \nabla) = \begin{Bmatrix} 0 & \frac{k}{2} & 0 \\ \frac{k}{2} & 0 & 0 \\ 0 & 0 & 0 \end{Bmatrix}$ note this doesn't have any diagonal elements (unlike finite strain theory)

Problem : Find displacements from given strains (if you are given strain you know stresses since $\sigma_{ij} = C_{ijkl} \epsilon_{kl}$)

Example $\epsilon_{xy} = xy$, all others $\epsilon_{ij} = 0$

$$\epsilon_{xx} = \frac{\partial u_x}{\partial x} = 0$$

$$\epsilon_{yy} = \frac{\partial u_y}{\partial y} = 0$$

$$\epsilon_{xy} = \frac{1}{2} \left(\frac{\partial u_y}{\partial x} + \frac{\partial u_x}{\partial y} \right)$$

$$u_x = f(y, z)$$

$$u_y = g(x, z)$$

$$\epsilon_{xy} = \frac{1}{2} (g' + f') = xy$$

this is an incompatible displacement field since product $xy \neq$ sum of two functions of different variables. Thus we must find compatibility conditions.

10/27/78

In infinitesimal strains we can forget about the differences between Lagrange/Euler. Thus we can use

$$\phi = \frac{1}{2} (\nabla u + u \nabla)$$

$$\epsilon_{ij} = \frac{1}{2} (u_{j,i} + u_{i,j})$$

$$\omega = \frac{1}{2} (\nabla u - u \nabla)$$

$$\omega_{ij} = \frac{1}{2} (u_{j,i} - u_{i,j})$$

now we are defining 6 dependent quantities (ϵ_{ij}) in terms of 3 indep quantities u_x, u_y, u_z .
 $\therefore$ there must be compatibility conditions by which the functions become determined uniquely.

$$\epsilon_{xx} = u_{x,x}$$

$$\epsilon_{xy} = \frac{1}{2} (u_{x,y} + u_{y,x})$$

$$\epsilon_{xx,yy} = u_{x,xyy}$$

$$\epsilon_{yy} = u_{y,y}$$

$$\epsilon_{yx} = \epsilon_{xy}$$

$$\epsilon_{yy,xx} = u_{y,xyx}$$

$$\epsilon_{xy} = \frac{1}{2} (u_{x,y} + u_{y,x})$$

$$2\epsilon_{xy,xy} = u_{x,xyy} + u_{y,xyx}$$

$$\therefore \frac{\partial^2 \epsilon_{xx}}{\partial y^2} + \frac{\partial^2 \epsilon_{yy}}{\partial x^2} - 2 \frac{\partial^2 \epsilon_{xy}}{\partial x \partial y} = 0$$

Compatibility

in the same manner we get

$$\psi_{xx} = 2 \frac{\partial^2 \epsilon_{yz}}{\partial y \partial z} - \frac{\partial^2 \epsilon_{yy}}{\partial z^2} - \frac{\partial^2 \epsilon_{zz}}{\partial y^2} = 0$$

$$\psi_{yy} = 2 \frac{\partial^2 \epsilon_{xz}}{\partial x \partial z} - \frac{\partial^2 \epsilon_{xx}}{\partial z^2} - \frac{\partial^2 \epsilon_{zz}}{\partial x^2} = 0$$

$$\psi_{zz} = 2 \frac{\partial^2 \epsilon_{xy}}{\partial x \partial y} - \frac{\partial^2 \epsilon_{xx}}{\partial y^2} - \frac{\partial^2 \epsilon_{yy}}{\partial x^2} = 0$$

and

$$\psi_{yz} = \frac{\partial^2 \epsilon_{xx}}{\partial y \partial z} - \frac{\partial}{\partial x} \left(-\frac{\partial \epsilon_{yz}}{\partial x} + \frac{\partial \epsilon_{zx}}{\partial y} + \frac{\partial \epsilon_{xy}}{\partial z} \right) = 0$$

$$\psi_{zx} = \frac{\partial^2 \epsilon_{yy}}{\partial z \partial x} - \frac{\partial}{\partial y} \left(-\frac{\partial \epsilon_{zx}}{\partial y} + \frac{\partial \epsilon_{xy}}{\partial z} + \frac{\partial \epsilon_{yz}}{\partial x} \right) = 0$$

$$\psi_{xy} = \frac{\partial^2 \epsilon_{zz}}{\partial x \partial y} - \frac{\partial}{\partial z} \left(-\frac{\partial \epsilon_{xy}}{\partial z} + \frac{\partial \epsilon_{yz}}{\partial x} + \frac{\partial \epsilon_{zx}}{\partial y} \right) = 0$$

EQUATIONS
OF
COMPATIBILITY

Cesaro proved that these are the equations which are unique by saying

$\oint_C du = 0$ also any space curve. (basically, this is morass than in a complex plane).

Look at handout $\Psi = \nabla \times \Phi \times \nabla$ is the incompatibility tensor
it is a symmetric tensor.

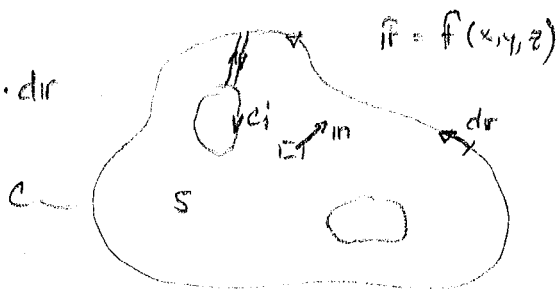
Φ is symmetric and curl on both sides keeps the symmetry.

Stokes theorem

$$\int_S \nabla \times \mathbf{f} \cdot d\mathbf{s} = \oint_C \mathbf{f} \cdot d\mathbf{r}$$

or

$$\int_S n_r \epsilon_{rmn} f_{n,m} ds = \oint_C f_k dx_k$$



10/31/78

Continue looking at handout

- ① - Compatibility $\nabla \times \xi \times \nabla = 0$ must be satisfied everywhere & must be satisfied pointwise.
- ② - For multiply connected region in addition

$$\oint_{C_i} dr \cdot (\xi \times \nabla) = 0$$
- ③ - and $\oint_{C_i} [\xi + (r - r_0) \times (\nabla \times \xi)] \cdot dr = 0$ these are integral relations that must be satisfied, not on a pointwise basis though.

For simply connected region: must satisfy ①

For multiply connected region: must satisfy ①, ②, ③

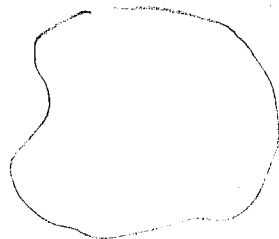
strain field requires compatibility based only on geometric considerations not on what loads produces this

Stress - Strain Relationships

Right now we only have 3 equations w/ 6 unknown. To get the other 3 equations must look at the material itself (ie 3 eqns of motion & 6 stresses)

Start with principles of conservation of energy

Given



the rate at which tractions do work ^{on the} ~~area~~ boundary
 + the rate of which body forces do work = change
 of internal energy + time change of energy flux across
 boundary or

$$\int_S t_n \cdot v ds + \int_V f \cdot v dV = \frac{D}{Dt} \left(\frac{1}{2} \rho V^2 dV + \frac{D}{Dt} \int_V \rho U_0 dV \right)$$

where U_0 is the internal energy density/unit mass and $v = \frac{\partial u_i}{\partial t}$

using linear theory (Eulerian = Lagrangian) we obtain

$$\int_V \rho \frac{\partial}{\partial t} \left(\underbrace{\frac{1}{2} V^2}_{KE} + \underbrace{U_0}_{\text{internal}} \right) dV = \int_V \rho \left(\frac{\partial u_i}{\partial t} \cdot \frac{\partial^2 u_i}{\partial t^2} + \frac{\partial U_0}{\partial t} \right) dV$$

Equations of Compatibility

I. Identities needed:

$$1) \quad a \times (b \times c) = a \cdot c b - a \cdot b c = a \cdot (cb - bc)$$

$$2) \quad I \times (b \times c) = I \cdot cb - I \cdot bc \\ = I \cdot (cb - bc) = cb - bc$$

$$3) \quad \nabla \times \nabla a = 0 \quad ; \quad a \nabla \times \nabla = 0$$

$$4) \quad \nabla \times (I \times a) = a \nabla - I \nabla \cdot a$$

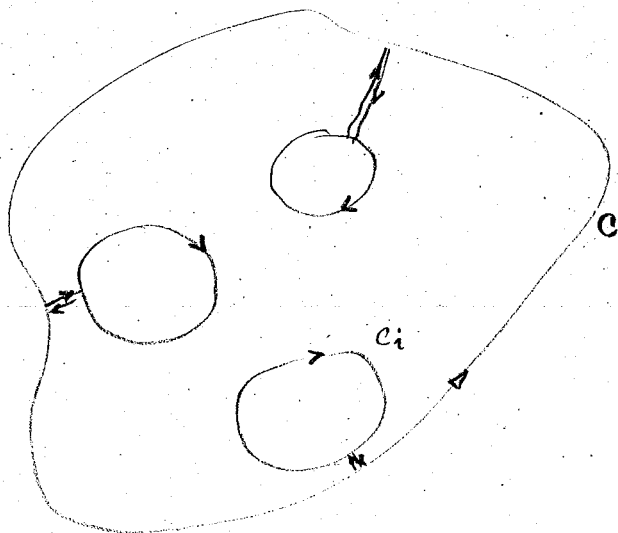
$$5) \quad \nabla \cdot (\nabla \times a) = 0$$

$\nabla \times a$ is a vector $\perp$ to ∇ & a $\therefore$ dotting it with any vector in the plane of ∇ & $a = 0$. Since they are $\perp$

$$6) \quad a \cdot (I \times b) = a \times b$$

$$a \cdot (I \times b) = I \cdot (a \times b) = a \times b$$

$$7) \quad da = dr \cdot \nabla a$$



$$\oint_C da = 0$$

(2)

II. To show that $\nabla \times \mathcal{F} \times \nabla = 0$

$$\text{Let } \nabla \mathcal{U} = \frac{1}{2} (\nabla \mathcal{U} + \mathcal{U} \nabla) + \frac{1}{2} (\nabla \mathcal{U} - \mathcal{U} \nabla) \\ = \mathcal{F} - \frac{1}{2} \mathbb{I} \times (\nabla \times \mathcal{U}) \quad \text{relationships 2}$$

Define $\mathcal{W} = \frac{1}{2} \nabla \times \mathcal{U}$ ^{vortexicity if fluid meets} Note: $\nabla \cdot \mathcal{W} = 0$ by relation 5

Thus $\nabla \mathcal{U} = \mathcal{F} - \mathbb{I} \times \mathcal{W}$

Curl: $\nabla \times \nabla \mathcal{U} = \nabla \times \mathcal{F} - \nabla \times (\mathbb{I} \times \mathcal{W}) = \nabla \times \mathcal{F} - \mathcal{W} \nabla + \mathbb{I} (\nabla \cdot \mathcal{W})$ ^{relation 4}
 $0 = \nabla \times \mathcal{F} - \mathcal{W} \nabla + 0$ ^{above}

Thus: $\nabla \times \mathcal{F} = \mathcal{W} \nabla$

Curl $\nabla \times \mathcal{F} \times \nabla = \mathcal{W} \nabla \times \nabla = 0$ ^{2nd formula relation 3}, p.e.d.

III. Multiply - Connected Regions

value is assigned

using relation 7

using $\nabla \mathcal{U} = \mathcal{F} - \mathbb{I} \times \mathcal{W}$

$$\Delta \mathcal{U} = \oint d\mathcal{U} = \oint \text{dir} \cdot \nabla \mathcal{U} = \oint \text{dir} \cdot \mathcal{F} - \oint \text{dir} \cdot (\mathbb{I} \times \mathcal{W})$$

look at 2nd term.

choosing a ref point $\mathcal{U}_0$

relation 6

$$\oint \text{dir} \cdot (\mathbb{I} \times \mathcal{W}) = \oint d(\mathcal{U} - \mathcal{U}_0) \cdot (\mathbb{I} \times \mathcal{W}) = \oint d(\mathcal{U} - \mathcal{U}_0) \times \mathcal{W}$$

$$= - \oint \mathcal{W} \times d(\mathcal{U} - \mathcal{U}_0) = - [\mathcal{W} \times (\mathcal{U} - \mathcal{U}_0)]_{\mathcal{U}_0}^{\mathcal{U}} + \oint d\mathcal{U} \times (\mathcal{U} - \mathcal{U}_0)$$

$$= \oint \text{dir} \cdot \nabla \mathcal{W} \times (\mathcal{U} - \mathcal{U}_0)$$

$$\int f dg = fg - \int g df$$

$$\mathcal{W} d(\mathcal{U} - \mathcal{U}_0) = \mathcal{W} (\mathcal{U} - \mathcal{U}_0) \Big|_{\mathcal{U}_0}^{\mathcal{U}} - \int d\mathcal{W} \times (\mathcal{U} - \mathcal{U}_0)$$

Note: $\mathcal{W} \nabla = \nabla \times \mathcal{F}$ ^{from above}

now remember $\Delta \mathcal{U} = \oint d\mathcal{U} = \oint \text{dir} \cdot \mathcal{F} - \oint \text{dir} \cdot \nabla \mathcal{W} \times (\mathcal{U} - \mathcal{U}_0)$

$$\Delta \mathcal{U} = \oint \text{dir} \cdot \mathcal{F} + \oint \text{dir} \cdot (\mathcal{F} \times \nabla) \times (\mathcal{U} - \mathcal{U}_0) \quad (\cdot) \cdot \text{dir} = \text{in} \cdot \nabla \times (\cdot)$$

$$= \iint \text{in} \cdot \nabla \times [\mathcal{F} + (\mathcal{F} \times \nabla) \times (\mathcal{U} - \mathcal{U}_0)] ds + \oint d\mathcal{U}$$

$$= \int \text{in} \cdot [\nabla \times \mathcal{F} \times \nabla] \times (\mathcal{U} - \mathcal{U}_0) ds + \oint d\mathcal{U} = 0 \text{ if } \nabla \times \mathcal{F} \times \nabla = 0 \text{ and } \oint d\mathcal{U} = 0$$

show that $\int \text{in} \cdot \nabla \times \mathcal{F} ds = \int (\nabla \cdot (\nabla \times \mathcal{F})) dV = 0$ $\therefore \nabla \cdot \nabla \times \mathcal{F} = 0$

Another starting point: but $\oint_{C_i} du = \oint_{C_i} dr \cdot \nabla + \oint_{C_i} dr \cdot (\nabla \times \mathbf{r}) \times (\mathbf{r} - \mathbf{r}_0)$

whether $\oint_{C_i} du = \oint_{C_i} dr \cdot [\nabla + (\nabla \times \mathbf{r}) \times (\mathbf{r} - \mathbf{r}_0)] = 0$
or ∇ does it matter

Subtract: $\oint_{C_i} [dr \cdot (\nabla \times \mathbf{r}) \times (\mathbf{r} - \mathbf{r}_0)] = 0$

or $\oint_{C_i} dr \cdot (\nabla \times \mathbf{r}) = 0$ 3 indep. eqns.

3 further indep. eqns. obtained $\Rightarrow$ follows:

We had

$$\oint_{C_i} [\nabla \cdot dr - d(\mathbf{r} - \mathbf{r}_0) \cdot (\nabla \times \mathbf{r})] = 0$$

relation #6

as $\oint_{C_i} [\nabla \cdot dr - d(\mathbf{r} - \mathbf{r}_0) \times \mathbf{r}] = 0$

After integration by parts:

$$\int d(\mathbf{r} - \mathbf{r}_0) \times \mathbf{r} = (\mathbf{r} - \mathbf{r}_0) \times \mathbf{r} - \int \mathbf{r} \times \mathbf{r}_0 \times d\mathbf{r}$$

$$\oint (\mathbf{r} - \mathbf{r}_0) \times \mathbf{r} = 0$$

$$\oint_{C_i} [\nabla \cdot dr + (\mathbf{r} - \mathbf{r}_0) \times d\mathbf{r}] = 0$$

or $\oint_{C_i} [\nabla + (\mathbf{r} - \mathbf{r}_0) \times \nabla] \cdot dr = 0$

or $\oint_{C_i} [\nabla + (\mathbf{r} - \mathbf{r}_0) \times (\nabla \times \mathbf{r})] \cdot dr = 0$ 3 indep. eqns.

the left hand side of the conserved eqn.

$$\int_S t_n \cdot \frac{\partial u_i}{\partial t} ds = \int_S m \cdot \sigma \cdot \frac{\partial u_i}{\partial t} ds = \int_V \nabla \cdot (\sigma \cdot \frac{\partial u_i}{\partial t}) dV = \int_V (\sigma_{kj} \frac{\partial u_j}{\partial t})_{,k} dV$$

$$\int_V (\sigma_{kj} \frac{\partial u_j}{\partial t})_{,k} dV = \int_V (\sigma_{kj,k} \frac{\partial u_j}{\partial t} + \sigma_{kj} \frac{\partial u_{j,k}}{\partial t}) dV = \int_V [\sigma_{kj,k} \frac{\partial u_j}{\partial t} + \sigma_{kj} \frac{\partial}{\partial t} (\epsilon_{kj} + \omega_{kj})] dV$$

since $\nabla u_i = \dot{\epsilon} + \omega$

$$\sigma_{kj} \frac{\partial}{\partial t} \epsilon_{kj}$$

now $\sigma_{kj} \frac{\partial}{\partial t} \omega_{kj} = 0$ since σ is sym & ω is antisym $\Rightarrow$ there will be like terms w/ opposite signs causing the terms to cancel

11/1/78

Compatibility Ψ is only good for small deformations not finite strains.

Normally we have 6 equations relating 9 unknowns $\therefore$ we must define 3 more equations from the material properties

$$\int_S t_n \cdot \frac{\partial u_i}{\partial t} ds = \int_S m \cdot (\sigma \cdot \frac{\partial u_i}{\partial t}) ds = \int_V \nabla \cdot (\sigma \cdot \frac{\partial u_i}{\partial t}) dV = \int_V (\sigma_{kj} \frac{\partial u_j}{\partial t})_{,k} dV$$

$$\int_V (\sigma_{kj} \frac{\partial u_j}{\partial t})_{,k} dV = \int_V (\sigma_{kj,k} \frac{\partial u_j}{\partial t} + \sigma_{kj} \frac{\partial u_{j,k}}{\partial t}) dV = \int_V \left\{ (\nabla \cdot \sigma) \cdot \frac{\partial u_i}{\partial t} + \sigma : \frac{\partial \nabla u_i}{\partial t} \right\} dV$$

but $\nabla u = \dot{\epsilon} + \omega = \frac{1}{2} (\underbrace{\nabla u_i + u_i \nabla}_{\text{sym}}) + \frac{1}{2} (\underbrace{\nabla u_i - u_i \nabla}_{\text{anti sym}})$

but $\sigma : \nabla u_i = \sigma : (\dot{\epsilon} + \omega) = \sigma : \dot{\epsilon} + \sigma : \omega = \sigma : \dot{\epsilon}$ where $\sigma : \omega = 0$ since σ is sym & ω is anti sym

$\therefore$ we can write the conservation of energy

$$\int_V \left(\frac{\partial u_j}{\partial t} \sigma_{kj,k} + \frac{\partial u_j}{\partial t} f_j - \rho \frac{\partial u_j}{\partial t} \frac{\partial^2 u_j}{\partial t^2} + \sigma_{kj} \frac{\partial \epsilon_{kj}}{\partial t} - \rho \frac{\partial U_0}{\partial t} \right) dV = 0$$

$\frac{\partial u_j}{\partial t} [P.L.M. = 0]$

$$\therefore \int_V (\sigma_{kj} \frac{\partial \epsilon_{kj}}{\partial t} - \rho \frac{\partial U_0}{\partial t}) dV = 0 \Rightarrow \sigma_{kj} \frac{\partial \epsilon_{kj}}{\partial t} - \rho \frac{\partial U_0}{\partial t} = 0 \text{ for any } dV$$

if we set $\dot{U}$ = internal energy per volume then $\sigma_{kj} \frac{\partial \epsilon_{kj}}{\partial t} = \frac{\partial U}{\partial t}$

or $\sigma : \dot{\epsilon} = \dot{U}$

we now postulate (assume) that $U = U(\text{strain only}) = U(\epsilon) = U(\epsilon_{ij})$

$$\dot{U} = \frac{\partial U}{\partial \epsilon_{11}} \dot{\epsilon}_{11} + \frac{\partial U}{\partial \epsilon_{22}} \dot{\epsilon}_{22} + \frac{\partial U}{\partial \epsilon_{33}} \dot{\epsilon}_{33} + \frac{\partial U}{\partial \epsilon_{23}} \dot{\epsilon}_{23} + \frac{\partial U}{\partial \epsilon_{31}} \dot{\epsilon}_{31} + \frac{\partial U}{\partial \epsilon_{12}} \dot{\epsilon}_{12}$$

since $\dot{U} = \sigma_{kj} \frac{\partial \epsilon_{kj}}{\partial t} = \sigma_{11} \dot{\epsilon}_{11} + \sigma_{22} \dot{\epsilon}_{22} + \sigma_{33} \dot{\epsilon}_{33} + 2\sigma_{23} \dot{\epsilon}_{23} + 2\sigma_{13} \dot{\epsilon}_{13} + 2\sigma_{12} \dot{\epsilon}_{12}$

$$\Rightarrow \sigma_{ij} = \frac{\partial U}{\partial \epsilon_{ij}} \quad \text{if } i=j$$

$$= \frac{1}{2} \frac{\partial U}{\partial \epsilon_{ij}} \quad \text{if } i \neq j$$

in equil state we want no residual stress
 $\therefore$ we want no strains or stress

if we expand U as a fn of ϵ then $U(\epsilon) = U(0) + U'(0)\epsilon + \frac{1}{2}U''(0)\epsilon^2 + \text{h.o.t.}$
 will be dropped
 since its only a reference state

$\therefore U = \frac{1}{2}U''(0)\epsilon^2$ in most general form $\epsilon^2 = \epsilon_{ij}\epsilon_{kl}$ then $U''(0) = C_{ijkl}$

$\therefore \boxed{U = \frac{1}{2} C_{ijkl} \epsilon_{ij} \epsilon_{kl}}$ this C_{ijkl} is something we pick to represent $U''(0)$

We drop the h.o.t. because they produce non linear terms
 From this we see that there should be 81 C_{ijkl} ; however this can be reduced to 36 since ϵ_{ij} has only 6 independent terms (& so does ϵ_{kl} $\therefore 6 \times 6 = 36$).
 of these 36, 21 will be unique (6 on main diag + 15 off diag terms).

11/3/78

Suppose we define $\sigma_p = \sigma_{ij}$ where $\sigma_1 = \sigma_{xx}, \sigma_2 = \sigma_{yy}, \sigma_3 = \sigma_{zz}, \sigma_4 = \sigma_{yz}, \sigma_5 = \sigma_{zx}, \sigma_6 = \sigma_{xy}$
 and $\epsilon_p = \epsilon_{ij}$ $\epsilon_1 = \epsilon_{xx}, \epsilon_2 = \epsilon_{yy}, \epsilon_3 = \epsilon_{zz}, \epsilon_4 = \gamma_{yz} = 2\epsilon_{yz}, \epsilon_5 = \gamma_{zx} = 2\epsilon_{zx}, \epsilon_6 = \gamma_{xy} = 2\epsilon_{xy}$

$$\sigma_{ij} = \begin{cases} \frac{\partial U}{\partial \epsilon_{ij}} & i=j \\ \frac{1}{2} \frac{\partial U}{\partial \epsilon_{ij}} & i \neq j \end{cases} \quad \text{now } \sigma_p = \frac{\partial U}{\partial \epsilon_p} \quad p=1, \dots, 6$$

Thus the strain energy

$$U = \frac{1}{2} C_{pq} \epsilon_p \epsilon_q$$

$$C_{12} = (C_{12} + C_{21}) \epsilon_1 \epsilon_2$$

$$U = \frac{1}{2} C_{11} \epsilon_1^2 + C_{12} \epsilon_1 \epsilon_2 + C_{13} \epsilon_1 \epsilon_3 + \dots + C_{16} \epsilon_1 \epsilon_6$$

$$+ \frac{1}{2} C_{22} \epsilon_2^2 + C_{23} \epsilon_2 \epsilon_3 + \dots + C_{26} \epsilon_2 \epsilon_6$$

$$+ \frac{1}{2} C_{66} \epsilon_6^2$$

Now we note that

$$\sigma_r = \frac{\partial U}{\partial \epsilon_r} = C_{11} \epsilon_1 + C_{12} \epsilon_2 + \dots + C_{16} \epsilon_6 \quad \text{and this is true for any } \sigma_r \quad (r=1, \dots, 6)$$

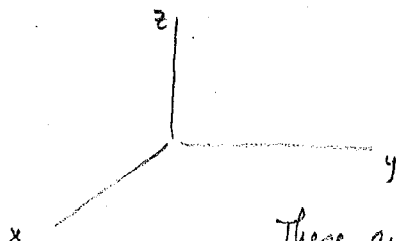
$$\text{Now } \sigma_r = C_{rp} \epsilon_p \quad (r, p=1 \dots 6) = \frac{\partial U}{\partial \epsilon_p}$$

Most general stress-strain relation of a linearly elastic solid involves 21 independent constants C_{rp}

Material scientists & physicists provided theory of crystals in order to determine C_{rp}
Crystal theory Triclinic involves 21 constants

Monoclinic	13	1 symmetry
Orthotropic / Orthorhombic	9	2 symmetries
Tetragonal / Trigonal	7, 6	
Hexagonal	5	
Cubic	3	
Isotropic	2	material properties in all directions are exactly the same.

To reduce from 21 to 2 we introduce cartesian plane & use symmetries of the crystal



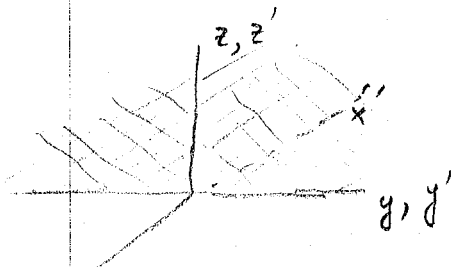
These are original relations involving $\sigma_i, C_{ij} \& \epsilon_j$

$$\sigma_1 = C_{11} \epsilon_1 + C_{12} \epsilon_2 + C_{13} \epsilon_3 + \dots + C_{16} \epsilon_6$$

$$\sigma_2 = C_{12} \epsilon_1 + \dots + C_{26} \epsilon_6$$

$$\sigma_6 = C_{16} \epsilon_1 + \dots + C_{66} \epsilon_6$$

Now let the yz plane be the plane of symmetry i.e. (x, x')



	x	y	z
x'	-1	0	0
y'	0	1	0
z'	0	0	1

remember $\sigma'_{mn} = \sigma_{ij} l_{mi} l_{nj}$ $\epsilon'_{mn} = \epsilon_{ij} l_{mi} l_{nj}$

$$\begin{array}{ll} \therefore \sigma'_1 = \sigma_1 & \sigma'_4 = \sigma_4 \\ \sigma'_2 = \sigma_2 & \sigma'_5 = -\sigma_5 \\ \sigma'_3 = \sigma_3 & \sigma'_6 = -\sigma_6 \end{array} \quad \left| \quad \begin{array}{ll} \epsilon'_1 = \epsilon_1 & \epsilon'_4 = \epsilon_4 \\ \epsilon'_2 = \epsilon_2 & \epsilon'_5 = -\epsilon_5 \\ \epsilon'_3 = \epsilon_3 & \epsilon'_6 = -\epsilon_6 \end{array} \right.$$

Now $\sigma'_1 = C_{11} \epsilon'_1 + \dots + C_{16} \epsilon'_6$ ok C_{ij} is invariant under transformation.
 $\sigma_1 = C_{11} \epsilon_1 + \dots - C_{15} \epsilon_5 - C_{16} \epsilon_6$ Substituting for $\sigma'_1, \epsilon'_1, \dots, \epsilon'_6$
 but originally $\sigma_1 = C_{11} \epsilon_1 + \dots + C_{15} \epsilon_5 + C_{16} \epsilon_6$ must also be true $\therefore C_{15} = C_{16} = 0$

$\sigma_2 = \sigma'_2$ from 2nd relation $\Rightarrow C_{25} = C_{26} = 0$

$\sigma_3 = \sigma'_3$ from 3rd relation $\Rightarrow C_{35} = C_{36} = 0$

$\sigma_4 = \sigma'_4$ from 4th relation $\Rightarrow C_{45} = C_{46} = 0$

$\sigma'_5 = -\sigma_5$ no change $\Rightarrow -\sigma_5 = -C_{55} \epsilon_5 - C_{56} \epsilon_6$

$\sigma'_6 = -\sigma_6$ no change $\Rightarrow -\sigma_6 = -C_{65} \epsilon_5 - C_{66} \epsilon_6$

These are the coeff ($\neq 0$) for monoclinic $C_{56}, C_{66}, C_{11}, C_{12}, C_{13}, C_{14}, C_{55}, C_{22}, C_{23}, C_{24}, C_{33}, C_{34}, C_{44}$

Now we look at symmetry in the (y, y')

then for orthorhombic $C_{14} = C_{24} = C_{34} = C_{56} = 0$

Non zero coeff $C_{11}, C_{12}, C_{13}, C_{22}, C_{23}, C_{33}, C_{66}, C_{55}, C_{44}$

	x	y	z
x'	1	0	0
y'	0	-1	0
z'	0	0	1

$$\sigma_1 = C_{11} \epsilon_1 + C_{12} \epsilon_2 + C_{13} \epsilon_3$$

$$\sigma_2 = C_{12} \epsilon_1 + C_{22} \epsilon_2 + C_{23} \epsilon_3$$

$$\sigma_3 = C_{13} \epsilon_1 + C_{23} \epsilon_2 + C_{33} \epsilon_3$$

$$\sigma_4 =$$

$$\sigma_5 =$$

$$\sigma_6 =$$

$$\begin{array}{ccc} C_{44} \epsilon_4 & 0 & 0 \\ 0 & C_{55} \epsilon_5 & 0 \\ 0 & 0 & C_{66} \epsilon_6 \end{array}$$

orthorhombic

$$\sigma'_1 = \sigma_1 \quad \sigma'_4 = -\sigma_4$$

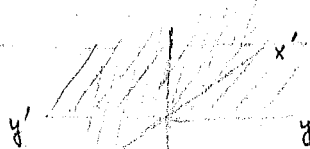
$$\sigma'_2 = \sigma_2 \quad \sigma'_5 = \sigma_5$$

$$\sigma'_3 = \sigma_3 \quad \sigma'_6 = -\sigma_6$$

$$\left\{ \begin{array}{ll} \epsilon'_1 = \epsilon_1 & \epsilon'_4 = -\epsilon_4 \\ \epsilon'_2 = \epsilon_2 & \epsilon'_5 = \epsilon_5 \\ \epsilon'_3 = \epsilon_3 & \epsilon'_6 = -\epsilon_6 \end{array} \right.$$

$$\begin{pmatrix} C_{11} & C_{12} & C_{13} & C_{14} \\ C_{12} & C_{22} & C_{23} & C_{24} \\ C_{13} & C_{23} & C_{33} & C_{34} \\ C_{14} & C_{24} & C_{34} & C_{44} \end{pmatrix} \begin{pmatrix} \epsilon_1 \\ \epsilon_2 \\ \epsilon_3 \\ \epsilon_4 \end{pmatrix} = \begin{pmatrix} \sigma_1 \\ \sigma_2 \\ \sigma_3 \\ \sigma_4 \end{pmatrix}$$

monoclinic



Stress Strain Relations and Elastic Symmetry

References: Sokolnikoff, Mathematical Theory of Elasticity pp. 56-71

Moivre, Introduction to The Mechanics of A Deformable Medium, pp 273-29

Triclinic Crystal (Most General Anisotropic material)

21 constants.

$$\begin{bmatrix} \epsilon_1 \\ \epsilon_2 \\ \epsilon_3 \\ \epsilon_4 \\ \epsilon_5 \\ \epsilon_6 \end{bmatrix} = \begin{bmatrix} C_{11} & C_{12} & C_{13} & C_{14} & C_{15} & C_{16} \\ C_{12} & C_{22} & C_{23} & C_{24} & C_{25} & C_{26} \\ C_{13} & C_{23} & C_{33} & C_{34} & C_{35} & C_{36} \\ C_{14} & C_{24} & C_{34} & C_{44} & C_{45} & C_{46} \\ C_{15} & C_{25} & C_{35} & C_{45} & C_{55} & C_{56} \\ C_{16} & C_{26} & C_{36} & C_{46} & C_{56} & C_{66} \end{bmatrix} \begin{bmatrix} \sigma_1 \\ \sigma_2 \\ \sigma_3 \\ \sigma_4 \\ \sigma_5 \\ \sigma_6 \end{bmatrix}$$

C (Stiffness) Matrix

Monoclinic Crystal one plane of elastic symmetry. e.g. yz plane. Reflection of x axis leaves constants unchanged

Direction Cosines of Transformation:

$$\begin{matrix} x' \\ y' \\ z' \end{matrix} \begin{bmatrix} x & y & z \\ -1 & 0 & 0 \\ 0 & 1 & 0 \\ 0 & 0 & 1 \end{bmatrix}$$

"C" Matrix:

$$\begin{bmatrix} C_{11} & C_{12} & C_{13} & C_{14} & 0 & 0 \\ C_{12} & C_{22} & C_{23} & C_{24} & 0 & 0 \\ C_{13} & C_{23} & C_{33} & C_{34} & 0 & 0 \\ C_{14} & C_{24} & C_{34} & C_{44} & 0 & 0 \\ 0 & 0 & 0 & 0 & C_{55} & C_{56} \\ 0 & 0 & 0 & 0 & C_{56} & C_{66} \end{bmatrix}$$

Orthorhombic (Orthotropic) Material 3 mutually orthogonal planes of symmetry. Reflection of x, y, and z leaves constants unchanged

Direction Cosines (reflect y axis)

$$\begin{matrix} x' \\ y' \\ z' \end{matrix} \begin{bmatrix} x & y & z \\ 1 & 0 & 0 \\ 0 & -1 & 0 \\ 0 & 0 & 1 \end{bmatrix}$$

"C" Matrix:

$$\begin{bmatrix} C_{11} & C_{12} & C_{13} \\ C_{12} & C_{22} & C_{23} \\ C_{13} & C_{23} & C_{33} \end{bmatrix} \begin{matrix} C_{44} \\ C_{55} \\ C_{66} \end{matrix}$$

Cubic Material Interchange of axes (i.e. rotate 90° then reflect) leaves constants unchanged.

"C" matrix:

Direction Cosines (interchange y & z)

$$\begin{matrix} x' \\ y' \\ z' \end{matrix} \begin{bmatrix} x & y & z \\ 1 & 0 & 0 \\ 0 & 0 & 1 \\ 0 & 1 & 0 \end{bmatrix}$$

$$\begin{bmatrix} C_{11} & C_{12} & C_{12} \\ C_{12} & C_{11} & C_{12} \\ C_{12} & C_{12} & C_{11} \end{bmatrix} \begin{matrix} C_{44} \\ C_{44} \\ C_{44} \end{matrix}$$

Isotropic Material: Any coordinate transformation leaves Elastic Constants unchanged

"C" matrix:

$$\begin{bmatrix} C_{11} & C_{12} & C_{12} \\ C_{12} & C_{11} & C_{12} \\ C_{12} & C_{12} & C_{11} \end{bmatrix} \begin{matrix} C_{44}^* \\ C_{44}^* \\ C_{44}^* \end{matrix} \quad \text{or} \quad \begin{bmatrix} \lambda + 2\mu & \lambda & \lambda \\ \lambda & \lambda + 2\mu & \lambda \\ \lambda & \lambda & \lambda + 2\mu \end{bmatrix} \begin{matrix} \mu \\ \mu \\ \mu \end{matrix}$$

$C_{44}^* = \frac{C_{11} - C_{12}}{2}$

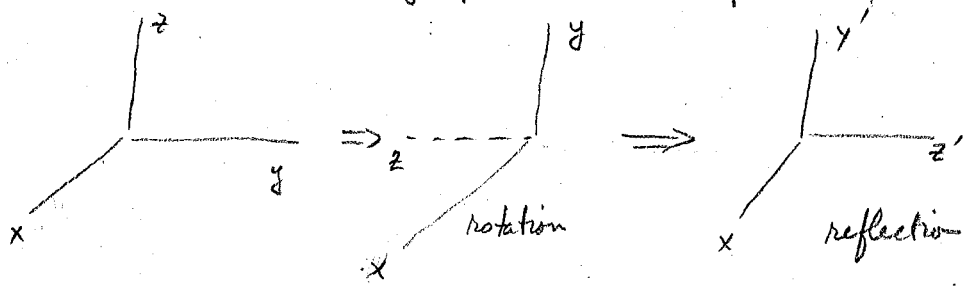
λ, μ , are the Lamé constants

$\sigma = C \epsilon$ may be written $\sigma_{ij} = \lambda E_{kk} \delta_{ij} + 2\mu \epsilon_{ij}$

where E_{kk} is the dilatation

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cubic material - interchange of 2 axes rotate by 90° & reflect.



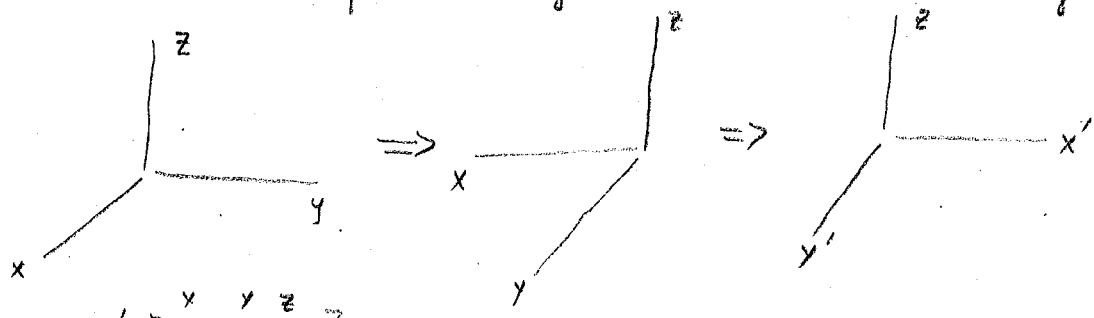
$$\begin{matrix} x' \\ y' \\ z' \end{matrix} \begin{bmatrix} x & y & z \\ 1 & 0 & 0 \\ 0 & 0 & 1 \\ 0 & 1 & 0 \end{bmatrix}$$

$$\begin{matrix} \sigma_1' = \sigma_1 & \sigma_4' = \sigma_4 \\ \sigma_2' = \sigma_3 & \sigma_5' = \sigma_6 \\ \sigma_3' = \sigma_2 & \sigma_6' = \sigma_5 \end{matrix} \quad \left\{ \begin{matrix} \epsilon_1' = \epsilon_1 & \epsilon_4' = \epsilon_4 \\ \epsilon_2' = \epsilon_3 & \epsilon_5' = \epsilon_6 \\ \epsilon_3' = \epsilon_2 & \epsilon_6' = \epsilon_5 \end{matrix} \right.$$

$$\begin{pmatrix} \epsilon_1' \\ \epsilon_2' \\ \epsilon_3' \\ \epsilon_4' \\ \epsilon_5' \\ \epsilon_6' \end{pmatrix} = \begin{pmatrix} \sigma_1 \\ \sigma_3 \\ \sigma_2 \\ \sigma_4 \\ \sigma_6 \\ \sigma_5 \end{pmatrix} = \begin{pmatrix} c_{11} & c_{12} & c_{13} \\ c_{12} & c_{23} & c_{22} \\ c_{13} & c_{23} & c_{23} \\ & c_{44} & \\ & & c_{55} \\ & & & c_{66} \end{pmatrix} \begin{pmatrix} \epsilon_1 \\ \epsilon_2 \\ \epsilon_3 \\ \epsilon_4 \\ \epsilon_5 \\ \epsilon_6 \end{pmatrix} = \begin{pmatrix} c_{11} & c_{12} & c_{13} \\ c_{12} & c_{22} & c_{23} \\ c_{13} & c_{23} & c_{33} & c_{44} & c_{55} & c_{66} \end{pmatrix}$$

$$\Rightarrow c_{13} = c_{12}, c_{22} = c_{33}, c_{55} = c_{66}$$

now since this is only 1 interchange we can do another interchange



$$\begin{matrix} x' \\ y' \\ z' \end{matrix} \begin{bmatrix} x & y & z \\ 0 & 1 & 0 \\ 1 & 0 & 0 \\ 0 & 0 & 1 \end{bmatrix}$$

using this we will get $c_{11} = c_{22}, c_{12} = c_{23}, c_{44} = c_{55}$ such that

$$\begin{pmatrix} c_{11} & c_{12} & c_{12} \\ c_{12} & c_{11} & c_{12} \\ c_{12} & c_{12} & c_{11} & c_{44} & c_{44} & c_{44} \end{pmatrix}$$

to go to isotropic just rotate about the x axis

$$\begin{matrix} x' \\ y' \\ z' \end{matrix} \begin{bmatrix} x & y & z \\ 1 & 0 & 0 \\ 0 & \cos\theta & \sin\theta \\ 0 & -\sin\theta & \cos\theta \end{bmatrix} \quad \sigma_4' = \frac{(\sigma_3 - \sigma_2)}{2} \sin 2\theta + \sigma_4 (\cos 2\theta) \quad \text{but } \sigma_4' = C_{44} \epsilon_4' \\ \text{and } \epsilon_4' = (\epsilon_3 - \epsilon_2) \sin 2\theta + \epsilon_4 \cos 2\theta$$

from the cubic $\sigma_3 - \sigma_2 = (C_{11} - C_{12})(\epsilon_3 - \epsilon_2)$ and $\sigma_4 = C_{44} \epsilon_4$ then we obtain

$$\frac{C_{11} - C_{12}}{2} (\epsilon_3' - \epsilon_2') \sin 2\theta + C_{44} \epsilon_4 \cos 2\theta = C_{44} (\epsilon_3 - \epsilon_2) \sin 2\theta + C_{44} \epsilon_4 \cos 2\theta$$

since θ is arbitrary $\Rightarrow C_{44} = \frac{C_{11} - C_{12}}{2}$

if we define $C_{44} = \mu$, $C_{12} = \lambda \Rightarrow C_{11} = \lambda + 2\mu$ λ, μ are Lamé constants.

$$\text{and } \left\{ \sigma_{ij} = \lambda E_{kk} \delta_{ij} + 2\mu \epsilon_{ij} \right\} \quad \text{Generalized Hooke's Law}$$

E_{kk} is the dilatation change in volume.

We can go directly to isotropic from the general strain energy density U being a scalar it is invariant under any transformation for an isotropic material

Normally $U = f(\epsilon_{ij}) = \frac{1}{2} C_{ijkl} \epsilon_{ij} \epsilon_{kl}$ for anisotropic
but $U = f(\epsilon_{ij})$ for isotropic and since it is invariant under any transformation we can pick U as a fn of the strain invariants or

$$U = f(I, II, III); \quad \text{since III involves cubic terms of } \epsilon_{ij} \text{ we can drop it to the first approx } \therefore$$

$$U = f(I, II) \cong \bar{A} I^2 + B II$$

where $I = \epsilon_{qq}$

$$II = \epsilon_{rs} \epsilon_{rs} - \epsilon_{qq} \epsilon_{rr}$$

$$\begin{aligned} U &\cong \bar{A} \epsilon_{qq} \epsilon_{rr} + B \epsilon_{rs} \epsilon_{rs} - B \epsilon_{qq} \epsilon_{rr} \\ &= A \epsilon_{qq} \epsilon_{rr} + B \epsilon_{rs} \epsilon_{rs} \end{aligned}$$

Stress-Energy Tensor of Isotropic Elastic Medium

Stress-Energy Density (Or Π)

Π is a scalar under invariant under transformation
 even for an anisotropic material. However for an isotropic
 medium, it must have the same "f" regardless of coordinate
 system, thus

$$\Pi = f(\epsilon_{ij}) = f(I, II)$$

To ensure this, Π must depend only on the invariants
 of strain, $\Pi = f(I, II, III)$

for quadratic form

$$\Pi = A(I)^2 + B(II)$$

$$I = \epsilon_{ij} \epsilon_{ij} = I^2 \text{ for } i=j$$

$$II = \epsilon_{ij} \epsilon_{jk} \epsilon_{ki} = \epsilon_{ij} \epsilon_{ji}$$

$$\Pi = A \epsilon_{ij} \epsilon_{ij} + B \epsilon_{ij} \epsilon_{ji} = B \epsilon_{ij} \epsilon_{ji}$$

$$= A \epsilon_{ij} \epsilon_{ij} + B \epsilon_{ij} \epsilon_{ji} \quad \text{where } A = B$$

$$\epsilon_{ij} = \frac{\partial u_i}{\partial x_j} \quad \text{and strain components are independent}$$

$$\text{e.g. } \frac{\partial \epsilon_{11}}{\partial x_2} = 0, \quad \frac{\partial \epsilon_{12}}{\partial x_1} = \frac{\partial \epsilon_{21}}{\partial x_1} = \frac{\partial \epsilon_{22}}{\partial x_1} = \dots$$

$$\frac{\partial \Pi}{\partial \epsilon_{ij}} = A(\epsilon_{ij} \delta_{ij} + \delta_{ij} \epsilon_{ij}) + B(\epsilon_{ij} \delta_{ij} + \delta_{ij} \epsilon_{ij})$$

$$= A(\epsilon_{ij} \delta_{ij} + \delta_{ij} \epsilon_{ij}) + 2B\epsilon_{ij}$$

$$= 2A\epsilon_{ij} \delta_{ij} + 2B\epsilon_{ij}$$

$$\text{where } A = B = \text{const. (constants)}$$

$$\boxed{\sigma_{ij} = 2A\epsilon_{ij} \delta_{ij} + 2B\epsilon_{ij}}$$

Stress-Strain Relations for Isotropic Material

Indicial form $\sigma_{ij} = \lambda \epsilon_{kk} + 2\mu \epsilon_{ij}$ (Hooke's Law for Isotropy)

Expanded Form $\sigma_{11} = \lambda(\epsilon_{11} + \epsilon_{22} + \epsilon_{33}) + 2\mu \epsilon_{11}$

$\sigma_{22} = \lambda(\quad) + 2\mu \epsilon_{22}$

$\sigma_{33} = \lambda(\quad) + 2\mu \epsilon_{33}$

$\sigma_{12} = 2\mu \epsilon_{12}$; $\sigma_{23} = 2\mu \epsilon_{23}$; $\sigma_{13} = 2\mu \epsilon_{13}$

λ, μ , are the Lamé constants

$\mu = G$ = shear modulus or modulus of rigidity

Bulk modulus: sum first (3) of eqns (1) to give

$$\sigma_{kk} = 3\lambda = (3\lambda + 2\mu)\Delta \quad (\Delta = \epsilon_{kk} = dilatation)$$

$$\text{or } \Delta = \frac{1}{3} \left(\frac{\sigma_{kk}}{\lambda + \frac{2}{3}\mu} \right)$$

$$K = \frac{\Delta}{3\epsilon_{ii}} = \frac{\sigma_{ii}}{3\epsilon_{ii}}$$

Bulk modulus $K = \lambda + \frac{2}{3}\mu$ (pressure required to cause unit change in volume)

Inverted σ - ϵ relations:

Last (3) of (1) invert directly $\Rightarrow \epsilon_{12} = \frac{1}{2\mu} \sigma_{12}$, $\epsilon_{13} = \frac{1}{2\mu} \sigma_{13}$, $\epsilon_{23} = \frac{1}{2\mu} \sigma_{23}$

1st (1): Eliminate Δ using K

$$\Rightarrow \sigma_{ij} = \frac{\lambda \sigma_{kk}}{3K} + 2\mu \epsilon_{ij}$$

$$\text{solve for } \epsilon_{11} = \frac{1}{2\mu} \left[\sigma_{11} - \frac{\lambda \sigma_{kk}}{3\lambda + 2\mu} \right]; \epsilon_{22} = \dots \epsilon_{33} = \dots$$

indicial inverted form: $\epsilon_{ij} = \frac{1}{2\mu} \left[\sigma_{ij} - \frac{\lambda \sigma_{kk}}{3\lambda + 2\mu} \delta_{ij} \right]$

Relation between λ, μ and E, ν :

$$\text{change modulus} = \frac{\mu(3\lambda + 2\mu)}{\lambda + \mu}; \quad \nu = \frac{\lambda}{3\lambda + 2\mu} \quad (\text{Poisson's Ratio})$$

$$\text{or } \lambda = \frac{E\nu}{(1+\nu)(1-2\nu)}, \quad \mu = \frac{E}{2(1+\nu)}$$

in terms of E, ν : $\epsilon_{11} = \frac{1}{E} (\sigma_{11} - \nu(\sigma_{22} + \sigma_{33}))$ $\epsilon_{22} = \dots$ $\epsilon_{33} = \dots$ (cyclic)

$$\epsilon_{12} = \frac{1+\nu}{E} \sigma_{12} \quad \epsilon_{23} = \dots \quad \text{and } \gamma_{ij} = 2\epsilon_{ij} = \frac{2(1+\nu)}{E} \sigma_{ij}$$

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TABLE 2.1

Relations between the elastic constants of an isotropic body.

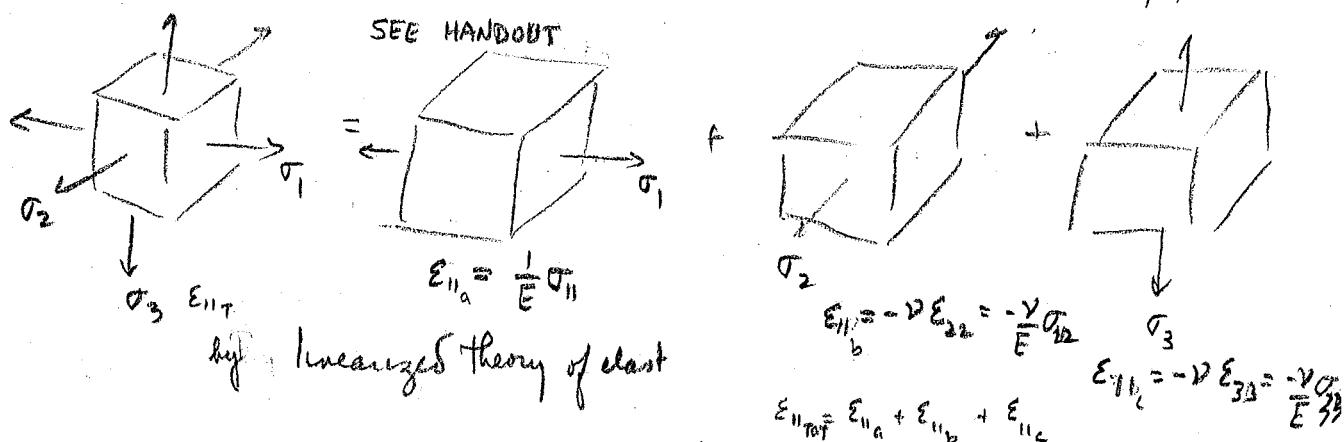
Constant ↓ in terms of E, ν of λ, μ, κ	E, ν	E, μ	E, κ	ν, λ	ν, μ	ν, κ	λ, μ	λ, κ	μ, κ
$E =$	E	E	E	$\frac{\lambda(1+\nu)(1-2\nu)}{\nu}$	$2\mu(1+\nu)$	$3\kappa(1-2\nu)$	$\frac{\mu(3\lambda+2\mu)}{\lambda+\mu}$	$\frac{9\kappa(\kappa-\lambda)}{3\kappa-\lambda}$	$\frac{9\kappa\mu}{3\kappa+\mu}$
$\nu =$	ν	$\frac{E}{2\mu} - 1$	$\frac{1}{2} - \frac{E}{6\kappa}$	ν	ν	ν	$\frac{\lambda}{2(\lambda+\mu)}$	$\frac{\lambda}{3\kappa-\lambda}$	$\frac{3\kappa-2\mu}{2(3\kappa+\mu)}$
$\lambda =$	$\frac{E\nu}{(1+\nu)(1-2\nu)}$	$\frac{\mu(E-2\mu)}{3\mu-E}$	$\frac{3\kappa(3\kappa-E)}{9\kappa-E}$	λ	$\frac{2\mu\nu}{1-2\nu}$	$\frac{3\kappa\nu}{1+\nu}$	λ	λ	$\frac{3\kappa-2\mu}{3}$
$\mu =$	$\frac{E}{2(1+\nu)}$	μ	$\frac{3\kappa E}{9\kappa-E}$	$\frac{\lambda(1-2\nu)}{2\nu}$	μ	$\frac{3\kappa(1-2\nu)}{2(1+\nu)}$	μ	$\frac{3(\kappa-\lambda)}{2}$	μ
$\kappa =$	$\frac{E}{3(1-2\nu)}$	$\frac{\mu E}{3(3\mu-E)}$	κ	$\frac{\lambda(1+\nu)}{3\nu}$	$\frac{2\mu(1+\nu)}{3(1-2\nu)}$	κ	$\frac{3\lambda+2\mu}{3}$	κ	κ

now $\sigma_{ij} = \frac{\partial U}{\partial \epsilon_{ij}} = A \frac{\partial \epsilon_{qq}}{\partial \epsilon_{ij}} \epsilon_{rr} + A \epsilon_{qq} \frac{\partial \epsilon_{rr}}{\partial \epsilon_{ij}} + 2B \frac{\partial \epsilon_{rs}}{\partial \epsilon_{ij}} \epsilon_{rs}$

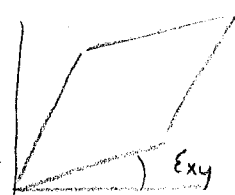
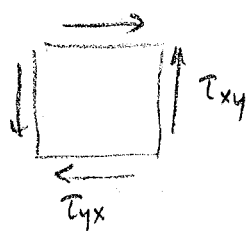
$A (\delta_{iq} \delta_{jq} \epsilon_{rr} + \delta_{ir} \delta_{jr} \epsilon_{qq}) + 2B \delta_{ir} \delta_{js} \epsilon_{rs}$

$= 2A \epsilon_{kk} \delta_{ij} + 2B \epsilon_{ij}$ LAMÉ $\lambda = 2A$ $B = \mu$

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$\therefore \epsilon_{11} = \frac{1}{E} (\sigma_{11} - \nu (\sigma_{22} + \sigma_{33}))$



$2\epsilon_{xy} = \gamma_{xy}$

$\gamma_{xy} = \frac{1}{\mu} \tau_{xy}$

$\epsilon_{xy} = \frac{1}{2\mu} \tau_{xy}$

$\mu = G$ shear modulus

$U = \frac{1}{2} \sigma_{ij} \epsilon_{ij} = \frac{1}{2} (\lambda \Delta \delta_{ij} + 2\mu \epsilon_{ij}) \epsilon_{ij}$ where $\Delta = \epsilon_{kk} = \nabla \cdot u$

$= \frac{1}{2} (\lambda \Delta \delta_{ij} \epsilon_{ij} + 2\mu \epsilon_{ij} \epsilon_{ij}) = \frac{1}{2} (\lambda \Delta^2 + 2\mu \epsilon_{ij} \epsilon_{ij}) ; \epsilon_{ij} = \delta_{ij} \epsilon_{ii}$

$U = \frac{1}{2} \lambda \Delta^2 + \mu [\epsilon_{11}^2 + \epsilon_{22}^2 + \epsilon_{33}^2 + 2(\epsilon_{12}^2 + \epsilon_{13}^2 + \epsilon_{23}^2)]$

- U will be shown to be positive definite ; we can rewrite U as

$U = \frac{1}{2} \left\{ \left(\lambda + \frac{2\mu}{3} \right) \Delta^2 + \mu \left[\frac{1}{3} (2\epsilon_{22} - \epsilon_{33} - \epsilon_{11})^2 + (\epsilon_{33} - \epsilon_{11})^2 + 4(\epsilon_{21}^2 + \epsilon_{31}^2 + \epsilon_{12}^2) \right] \right\}$

note that $\epsilon_{12}, \epsilon_{13}, \epsilon_{23}$ are independent; since $K = \lambda + \frac{2\mu}{3} > 0$ & $\mu > 0$
 we still need to prove that $\Delta, 2\epsilon_{11} - \epsilon_{22} - \epsilon_{33}, -\epsilon_{11} + \epsilon_{33}$ are indep.
 - look at det of coeff; it is $\neq 0$ $\therefore$ they are indep. $\Rightarrow U$ is positive definite.

Now to prove uniqueness of solution

$$\begin{cases} \sigma_{ij,i} + f_j = \rho \ddot{u}_j & (3) \\ \epsilon_{ij} = \frac{1}{2} (u_{i,j} + u_{j,i}) & (6) \\ \sigma_{ij} = \lambda \epsilon_{kk} \delta_{ij} + 2\mu \epsilon_{ij} & (6) \end{cases} \quad \begin{matrix} 3u_j, 6\epsilon_{ij}, 6\sigma_{ij} = 15 \text{ unknowns} \\ 15 \text{ eqs.} \end{matrix}$$

(15)

Assume 2 solutions let $\sigma'_{ij}, \epsilon'_{ij}, u'_j$ be solution #1
 $\sigma''_{ij}, \epsilon''_{ij}, u''_j$ be solution #2
 $\sigma^*_j, \epsilon^*_j, u^*_j$ be sol #1 - sol #2

by linear theory if sol #1, sol #2 solve eqs so do any linear comb
 of sol #1 & sol #2 $\Rightarrow$ sol* is also a solution

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Kirchoff: proof of uniqueness - assume 2 solns to same problem; since difference must also be a solution, one can use the energy equation and try to find conditions on the solution.

Uniqueness of solution of a typical problem

1. Assume 2 solutions $(\sigma'_{ij}, \epsilon'_{ij}, u'_j)$ $(\sigma''_{ij}, \epsilon''_{ij}, u''_j)$

$$\left. \begin{aligned} \text{let } \sigma^*_{ij} &= \sigma'_{ij} - \sigma''_{ij} \\ \epsilon^*_{ij} &= \epsilon'_{ij} - \epsilon''_{ij} \\ u^*_i &= u'_i - u''_i \end{aligned} \right\} \begin{matrix} \text{must also satisfy stress strain} \\ \text{strain energy} \end{matrix}$$

The "difference" solution must satisfy (because of linearity) the stress eqs of equil $(\sigma^*_{ij,i} + f^*_j = \rho \ddot{u}^*_j)$, the strain relation $\epsilon^*_{ij} = \frac{1}{2} (u^*_{j,i} + u^*_{i,j})$ and the stress strain $\sigma^*_{ij} = \lambda \Delta^* \delta_{ij} + 2\mu \epsilon^*_{ij}$ where $f^*_j = f'_j - f''_j$

We postulate that the total energy of the "difference" solution should vanish at all times. Let us derive the energy for the system

$$E = E(t_0) + \int_{t_0}^t \int_V (K + U) dV dt$$

$$U = (\lambda + \frac{2}{3}\mu) \Delta^{*2} + \mu \left[\frac{1}{3} (2\epsilon_{22}^* - \epsilon_{33}^* - \epsilon_{11}^*)^2 + (\epsilon_{33}^* - \epsilon_{11}^*)^2 + 4(\epsilon_{23}^* + \epsilon_{31}^* + \epsilon_{12}^*)^2 \right]$$

$$K = \frac{1}{2} \rho \dot{u}_K^* \dot{u}_K^* = \frac{1}{2} \rho (\dot{u}_1^{*2} + \dot{u}_2^{*2} + \dot{u}_3^{*2})$$

at $t=t_0$ $E = E(t_0) = 0$ $\sigma_0^*, \epsilon_0^*, \dot{u}_0^* = 0$ I.C. must also be specified

Since $K+U$ are quadratic fns, the only way that E can be zero is if each term in the quadratic is zero (since $\dot{u}$ is positive definite) (displacements u_i , velocities $\dot{u}_i$)

at any times: $E = \int_{t_0}^t \int_V \left[\rho (\dot{u}_1^* \ddot{u}_1^* + \dot{u}_2^* \ddot{u}_2^* + \dot{u}_3^* \ddot{u}_3^*) + \sigma_{ij}^* \dot{\epsilon}_{ij}^* \right] dV dt = 0$

$$\begin{aligned} \sigma_{ij}^* \dot{\epsilon}_{ij}^* &= \sigma_{ij}^* \dot{u}_{j,i}^* \quad (\text{since } \dot{\epsilon}_{ij}^* = \dot{u}_{j,i}^* - \dot{u}_{i,j}^* \text{ and since } u_{jii} \text{ was symmetric part of } \epsilon_{ij}) \\ &= (\sigma_{ij}^* \dot{u}_j^*)_{,i} - \sigma_{ij,i}^* \dot{u}_j^* \quad \text{using differentiation by parts} \end{aligned}$$

$$\int_{t_0}^t \int_V \left[\rho \ddot{u}_j^* - \sigma_{ij,i}^* \right] \dot{u}_j^* + \underbrace{(\sigma_{ij}^* \dot{u}_j^*)_{,i}}_{\nabla \cdot (\sigma_{ij})} dV dt = 0 \quad \text{after replacing the above}$$

$$\int_{t_0}^t \int_V \left[\rho \ddot{u}_j^* - \sigma_{ij,i}^* \right] \dot{u}_j^* + \int_{S^*} n_i \sigma_{ij}^* \dot{u}_j^* dS \quad \text{by divergence theorem}$$

but $\sigma_{ij,i}^* + f_j^* = \rho \ddot{u}_j^*$ and $n_i \sigma_{ij}^* = t_j^*$ and to be even more general we take into account singularities, discont. b.c.

$$E = E_0 + \int_{t_0}^t \left[\int_V f_j^* \dot{u}_j^* dV + \int_S t_j^* \dot{u}_j^* dS + \int_{S^*} + \int_{S_d} t_j^* \Delta \dot{u}_j^* dS \right] dt$$

singularities in the region discontinuities occurring in the displacement field

How $E = 0 \Rightarrow$

- 1) $E_0 = 0$ from specified initial conditions
- 2) $f_j^* = 0$ from specified body forces.
- 3) Compatibility conditions must be specified to insure $\int_{S^*} = 0$.
- 4) Must specify singularities - (how they become ∞) and therefore $\int_{S^*} = 0$
- 5) Boundary conditions must be specified so that $\int_{S^*} t_j^* \dot{u}_j^* = 0$
 - a. either t_j^* are all zero (ie specify traction vector at each point on the surface of body)
 - b. either $\dot{u}_j^*$ are all zero (ie specify displacement rate at each surface point)
 - c. either $t_j^* / \dot{u}_j^*$ combinations are zero ie $t_1^* = \dot{u}_2^* = \dot{u}_3^* = 0$ or $t_1^*, t_2^*, \dot{u}_3^* = 0$

therefore we have 8 different ways to ensure a unique solution (a-1, b=1, c=3)

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1) For unique soln ϵ_{ij}^* , $u_i^* = 0$

2) $\epsilon_{ij}^* = u_i^* = 0$ if $E = 0$ (because $E = 0$ only if every square of the positive definite fr. E vanishes separately)

3) But E can also be written as $E = E_0 + \int_{t_0}^t \left[\int_V f_j^* u_j^* dV + \int_S t_j^* u_j^* dS \right] dt$

(4) $f_j^* = 0$, which means f_j is specified
and (for example)
 $t_j^* = 0$ " " t_j " "

In statics if f are specified uniqueness of disp are specified up to rigid body effects (disp, rotation)

In dynamics uniqueness completely specifies solution.

Formulation of Typical Problems in Elast.

1) Disp. (3-D problem)

2) Stress formul (2-D problem)

$$\epsilon_{ij} = \frac{1}{2} (u_{j,i} + u_{i,j})$$

$$\sigma_{ij} = \lambda u_{k,k} \delta_{ij} + \mu (u_{j,i} + u_{i,j}) \quad (2)$$

$$\sigma_{ij,i} + f_j = \rho \ddot{u}_j \quad (3)$$

substitute σ_{ij} from (2) into (3) to get Displacement Formulation

$$\mu u_{j,ii} + (\lambda + \mu) u_{i,i,j} + f_j = \rho \ddot{u}_j$$

$$\mu \nabla^2 u_i + (\lambda + \mu) \nabla (\nabla \cdot u) + f_i = \rho \ddot{u}_i$$

$$\mu \nabla \cdot (\nabla u) + (\lambda + \mu) \nabla (\nabla \cdot u) + f = \rho \ddot{u}$$

$$\text{Div (grad } u) + \text{grad (div } u)$$

Stress formulation (we must also satisfy $\nabla \times \mathbf{q} \times \nabla = 0$)

$\epsilon_{jl,sk} + \epsilon_{sk,jl} - \epsilon_{sl,jk} - \epsilon_{jk,sl} = 0$
 this is the most convenient form to be used here. Beltrami & Michell
 converted them to stress forms

let us define them

$$\epsilon_{ij} = \frac{1+\nu}{E} \sigma_{ij} - \frac{\nu}{E} \sigma_{kk} \delta_{ij} \quad \text{substitute into the above equation giving}$$

$$\sigma_{ij,kl} + \sigma_{kl,ij} - \sigma_{ik,jl} - \sigma_{jl,ik} = \frac{\nu}{1+\nu} (\delta_{ij} \sigma_{\alpha\alpha,kl} + \delta_{kl} \sigma_{\alpha\alpha,ij} - \delta_{ik} \sigma_{\alpha\alpha,jl} - \delta_{jl} \sigma_{\alpha\alpha,ik})$$

There are 6 independent equations

$k=l=1$	$k=l=2$	$k=l=3$	$k=l=1$	$k=l=2$	$k=l=3$
$i=j=2$	$i=j=3$	$i=j=1$	$i=2, j=3$	$i=3, j=1$	$i=1, j=2$

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We will try to express $\nabla \times \mathbf{q} \times \nabla$ in terms of stress.

Derivation of Beltrami - Michell Cond.

$$\sigma_{ij,kl} + \sigma_{kl,ij} - \sigma_{ik,jl} - \sigma_{jl,ik} = \frac{\nu}{1+\nu} (\delta_{ij} \sigma_{\alpha\alpha,kl} + \delta_{kl} \sigma_{\alpha\alpha,ij} - \delta_{ik} \sigma_{\alpha\alpha,jl} - \delta_{jl} \sigma_{\alpha\alpha,ik}) \quad (A)$$

let $k=l$ and add also let $\sigma_{kk} = \Theta$

$$\sigma_{ij,kk} + \sigma_{kk,ij} - \sigma_{ik,jk} - \sigma_{jk,ik} = \frac{\nu}{1+\nu} (\delta_{ij} \sigma_{\alpha\alpha,kk} + \delta_{kk} \sigma_{\alpha\alpha,ij} - \delta_{ik} \sigma_{\alpha\alpha,jk} - \delta_{jk} \sigma_{\alpha\alpha,ik})$$

$$\begin{aligned} \nabla^2 \sigma_{ij} + \Theta_{,ij} - \sigma_{ik,jk} - \sigma_{jk,ik} &= \frac{\nu}{1+\nu} (\delta_{ij} \nabla^2 \Theta + 3 \Theta_{,ij} - \delta_{ik} \Theta_{,jk} - \delta_{jk} \Theta_{,ik}) \\ &= \frac{\nu}{1+\nu} (\delta_{ij} \nabla^2 \Theta + \Theta_{,ij}) - 2 \Theta_{,ij} \end{aligned}$$

from Eqs of motion

$$\begin{aligned} \sigma_{ij,i} &= \rho \ddot{u}_j - f_j & \text{now take } \frac{\partial}{\partial k} \text{ (eq of motion)} \\ \sigma_{ij,ik} &= \rho \ddot{u}_{j,k} - f_{j,k} & \text{put this into the compat eqn.} \end{aligned}$$

$$\nabla^2 \sigma_{ij} + \frac{1}{1+\nu} \Theta_{,ij} - \frac{\nu}{1+\nu} \delta_{ij} \nabla^2 \Theta = -(f_{ij} + f_{ji}) + \rho (\ddot{u}_{ij} + \ddot{u}_{ji}) \quad (B)$$

Next in (A) let $k=i, l=j$ and add

$$\text{then } 2\sigma_{ij,ij} - \sigma_{ii,jj} - \sigma_{jj,ii} = \frac{\nu}{1+\nu} (2\delta_{ij} \Theta_{,ij} - \delta_{ii} \Theta_{,jj} - \delta_{jj} \Theta_{,ii})$$

$$\sigma_{ij,ij} - \nabla^2 \phi = \frac{\nu}{1+\nu} \left(\nabla^2 \phi - 3 \nabla^2 \phi \right) \quad \text{or} \quad \sigma_{ij,ij} = \frac{1-\nu}{1+\nu} \nabla^2 \phi$$

if we go back to eq of motion and take $\frac{\partial}{\partial x_j}$ $\therefore \sigma_{ij,ij} = \rho \ddot{u}_{j,j} - f_{j,j}$

substitute for $\sigma_{ij,ij}$ into the above $\rho \ddot{u}_{j,j} - f_{j,j} = \frac{1-\nu}{1+\nu} \nabla^2 \phi$

$$\therefore \nabla^2 \phi = - \left(\frac{1+\nu}{1-\nu} \right) [f_{j,j} - \rho \ddot{u}_{j,j}] \quad \text{C}$$

Substitute into B eqn C

$$\nabla^2 \sigma_{ij} + \frac{1}{1+\nu} \phi_{,ij} = -\frac{\nu}{1-\nu} \delta_{ij} [f_{k,k} - \rho \ddot{u}_{k,k}] - (f_{i,j} + f_{j,i}) + \rho (\ddot{u}_{i,j} + \ddot{u}_{j,i})$$

$\rho \ddot{\epsilon}_{ii}$ $2 \ddot{\epsilon}_{ij}$

using the stress strain relation $\sigma_{ij} = \lambda \Delta \delta_{ij} + 2\mu \epsilon_{ij}$ or $\ddot{\sigma}_{ij} = \lambda \ddot{\Delta} \delta_{ij} + 2\mu \ddot{\epsilon}_{ij}$

$$\boxed{\nabla^2 \sigma_{ij} + \frac{1}{1+\nu} \phi_{,ij} = -\frac{\nu}{1-\nu} f_{k,k} \delta_{ij} - f_{i,j} - f_{j,i} - \frac{\rho}{E} \left(\frac{\nu}{1-\nu} \phi \delta_{ij} - 2(1+\nu) \ddot{\sigma}_{ij} \right)}$$

for the case of const body force & no accel. then

$$\boxed{\nabla^2 \sigma_{ij} + \frac{1}{1+\nu} \phi_{,ij} = 0} \quad \text{Beltrami Eqs.}$$

Look at the B eq w/no accel or body forces.

then B reduces to $\nabla^2 \sigma_{ij} + \frac{1}{1+\nu} \phi_{,ij} - \frac{\nu}{1+\nu} \delta_{ij} \nabla^2 \phi = 0$ but the first 2 terms are zero by Beltrami then $\Rightarrow$

$$\boxed{\nabla^2 \phi = 0 \quad \text{or} \quad \sigma_{kk} = \phi \text{ is a harmonic fn}}$$

Now take ∇^2 (Beltrami)

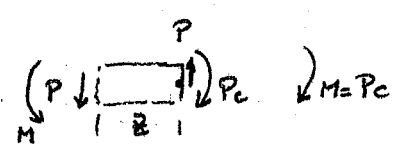
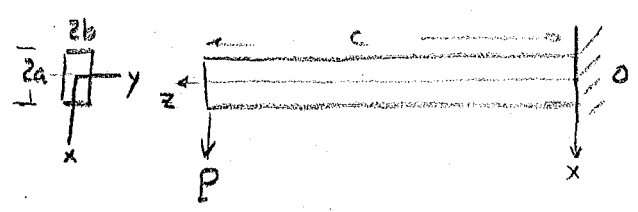
$$\nabla^2 (\nabla^2 \sigma_{ij}) + \frac{1}{1+\nu} \nabla^2 \phi_{,ij} = 0$$

0 since $\nabla^2 \phi = 0$

$$\therefore \boxed{\nabla^4 \sigma_{ij} = 0 \quad \text{or} \quad \sigma_{ij} \text{ must be biharmonic}}$$

∴ only biharmonic fns are the only admissible stress field

Consider a cantilevered beam under an end load. Quest: Is strength of material solution also an elasticity soln.



$$M + Pz = P_c z$$

$$M = P(c-z)$$

$$\therefore -M - Pz = -P_c z \quad M = P(c-z)$$

$$\sigma_z = \frac{-Mx}{I}$$

$$\sigma_{xx}, \sigma_{yy}, \sigma_{zy}, \sigma_{xy} = 0$$

$$\sigma_{zz} = \frac{Mx}{I} = \frac{P(c-z)x}{I}$$

$$\sigma_{zx} = \frac{VQ}{Ib} = \frac{P(a^2 - x^2)}{2I}$$



$$Q = \int_{-a}^a 2b x dx = \frac{(a^2 - x^2) 2b}{2}$$

$$Q = \int_x^a b dA$$

Must satisfy equil
Compat
B.C.

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CONTINUATION OF LAST LECTURE on Strength of Material Solution

Equil: (1) & (2) satisfied identically

$$\frac{\partial \sigma_{zx}}{\partial x} + \frac{\partial \sigma_{zy}}{\partial y} + \frac{\partial \sigma_{zz}}{\partial z} = 0$$

$$-\frac{Px}{I} + \frac{Px}{I} = 0 \quad \checkmark$$

check on BC

At z for x=±a, y=±b there must be no tractions

For x=±a, y=±b

$$t_n = (\sigma_{xx}, \sigma_{xy}, \sigma_{xz}) = \left(\frac{P(a^2 - x^2)}{2I}, 0, 0 \right) \bigg|_{x=\pm a} = 0 \quad (n=e_x)$$

$$t_n = (\sigma_{xy}, \sigma_{yy}, \sigma_{yz}) = (0, 0, 0) \quad (n=e_y)$$

on the end z=c

$$t_n = (\sigma_{zx}, \sigma_{zy}, \sigma_{zz}) = \left(\frac{P(a^2 - x^2)}{2I}, 0, 0 \right) \quad (n=e_z)$$

$$\therefore \int_{-a}^a \int_{-b}^b \frac{P(a^2 - x^2)}{2I} dx dy = P \quad \therefore \text{B.C. is satisfied}$$

$$= \frac{P \cdot 2b}{2I} \left(a^2 x - \frac{x^3}{3} \right) \bigg|_{-a}^a = \frac{Pb}{I} \left\{ \frac{2a^3}{3} - \left(-\frac{a^3}{3} + \frac{a^3}{3} \right) \right\} = \frac{4Pa^3}{3I}$$

we do not worry about the built in end.

$$I = \frac{1}{3} a^3 b$$

we will check compatibility using Beltrami Eqs.

$$\nabla^2 \sigma_{xx} + \frac{1}{1+\nu} \frac{\partial^2 \Theta}{\partial x^2} = 0$$

$$\nabla^2 \sigma_{yy} + \frac{1}{1+\nu} \frac{\partial^2 \Theta}{\partial y^2} = 0$$

$$\nabla^2 \sigma_{zz} + \frac{1}{1+\nu} \frac{\partial^2 \Theta}{\partial z^2} = 0$$

$$\nabla^2 \sigma_{yz} + \frac{1}{1+\nu} \frac{\partial^2 \Theta}{\partial y \partial z} = 0$$

$$\nabla^2 \sigma_{zx} + \frac{1}{1+\nu} \frac{\partial^2 \Theta}{\partial x \partial z} = 0$$

$$\nabla^2 \sigma_{xy} + \frac{1}{1+\nu} \frac{\partial^2 \Theta}{\partial x \partial y} = 0$$

where $\Theta = \sigma_{xx} + \sigma_{yy} + \sigma_{zz}$

$$\Theta = \frac{P(z-c)x}{I}$$

(1-4, 6) will be zero

$$(5) = -\frac{P}{I} + \frac{1}{1+\nu} \left(\frac{P}{I} \right) \neq 0$$

$$(7) = -\frac{\nu}{1+\nu} \frac{P}{I}$$

thus a Poisson's effect is evident in elasticity that is not found by strength of materials

Calculating the strains

$$\epsilon_{ij} = \frac{1+\nu}{E} \sigma_{ij} - \frac{\nu}{E} \sigma_{kk} \delta_{ij}$$

$$\epsilon_{xx} = \epsilon_{yy} = -\frac{Px(z-c)\nu}{EI} ; \quad \epsilon_{zz} = \frac{Px(z-c)}{EI}$$

$$\epsilon_{yz} = \epsilon_{xy} = 0 ; \quad \epsilon_{zx} = \frac{1+\nu}{E} \frac{P(a^2-x^2)}{2I}$$

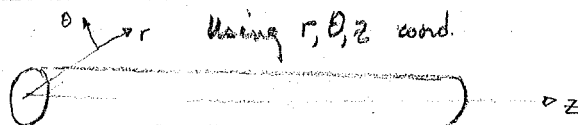
$$\frac{\partial^2 \epsilon_{yy}}{\partial z \partial x} + \frac{\partial}{\partial y} \left(\frac{\partial \epsilon_{zx}}{\partial y} - \frac{\partial \epsilon_{xy}}{\partial z} - \frac{\partial \epsilon_{yz}}{\partial x} \right) = \frac{\partial^2 \epsilon_{yz}}{\partial z \partial x} = -\frac{P\nu}{EI}$$

at the top of beam fibers will stretch causing the $\perp$ fibers to shorten
 at "bottom" " " " " contract " " " " " length
 this causes transverse shears to be set up which are not taken into account by strength of materials.

We next take up torsion of beams.

Coulomb Torsion

Check this again



Coulomb assumed a displacement solution & checked the compat, equil, etc.

He assumed $u_r = 0, u_z = 0, u_\theta = \alpha r z$ where $\alpha =$ proportionality factors.

$$\Rightarrow \epsilon_{rr} = 0, \epsilon_{\theta\theta} = 0, \epsilon_{zz} = 0, \epsilon_{r\theta} = 0, \epsilon_{zr} = 0, \epsilon_{\theta z} = \frac{1}{2} \alpha r$$

let's get the stresses

$$\sigma_{ij} = \lambda \epsilon_{kk} \delta_{ij} + 2\mu \epsilon_{ij} \Rightarrow \sigma_{\theta z} = \mu \alpha r \text{ only non vanishing shear}$$

with this $\sigma_{\theta z}$ we satisfy equil. We also assume torsion is end loaded not surface area loaded.

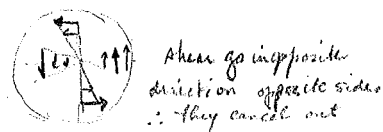
BC Cylindrical surface free of traction i.e. $\mathbf{e}_r \cdot \boldsymbol{\sigma} = 0$

but $\mathbf{e}_r \cdot \boldsymbol{\sigma} = (\sigma_{r\theta}, \sigma_{rz}, \sigma_{rr}) = 0$ thus we satisfy the BC on surface

$$\text{on the ends } T = \int_0^{2\pi} \int_0^a \sigma_{\theta z} \cdot r \cdot r dr d\theta = \int_0^a \int_0^{2\pi} \mu \alpha r^3 dr d\theta = \frac{\pi a^4}{2} \mu \alpha$$

$$\text{define polar moment of inertia } J = \int r^2 dA = \frac{\pi a^4}{2} \therefore T = J \mu \alpha \text{ (torque)}$$

$$\text{load on the end } \mathbf{e}_z \cdot \boldsymbol{\sigma} = (\sigma_{zr}, \sigma_{z\theta}, \sigma_{zz}) = \sigma_{\theta z}$$



shear goes in opposite direction opposite sides $\therefore$ they cancel out

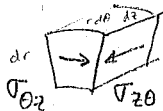
$$w_{r\theta} = \frac{1}{2} \left[\frac{1}{r} \frac{\partial(r u_\theta)}{\partial r} - \frac{1}{r} \frac{\partial u_r}{\partial \theta} \right] = \alpha z$$

α (is the twist) rotation/unit of length of cylinder

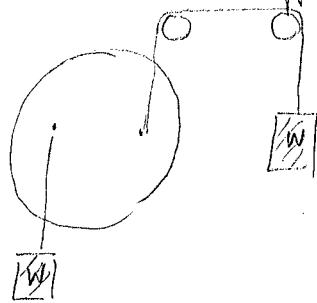
$$\alpha = \frac{\partial}{\partial z} (w_{r\theta})$$

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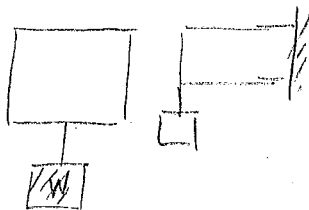
each cross section rotates as a body in Coulomb torsion



Suppose we look at different problems



such that this system is in T. Is this system the same as the last torsion problem

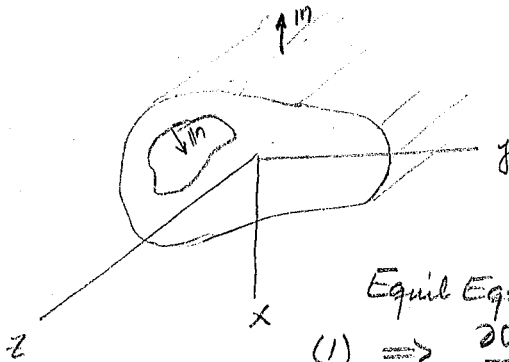


If $W = P$ is this system the same as a uniformly end loaded beam such that $\int \sigma_{xz} dA = P$.

The answer is given by St. Venant's principle. If the two systems are statically equivalent then the stress distribution will be the same everywhere except near the loading point (points, faces etc) where the stress distribution produces a local effect. (This has not been proven - there are contradictory cases.)

St. Venant Torsion problem.

No tractions on surface of the body except on the ends.



Let $\sigma_{zz}, \sigma_{xx}, \sigma_{yy}, \sigma_{xy} = 0$ and no body forces
Let $\sigma_{zx} \neq 0$; $\sigma_{zy} \neq 0$

Equil Eqs:

$$(1) \Rightarrow \frac{\partial \sigma_{zx}}{\partial z} = 0 \Rightarrow \sigma_{zx} = \sigma_{zx}(x, y)$$

$$(2) \Rightarrow \frac{\partial \sigma_{zy}}{\partial z} = 0 \Rightarrow \sigma_{zy} = \sigma_{zy}(x, y)$$

$$(3) \Rightarrow \frac{\partial \sigma_{zx}}{\partial x} + \frac{\partial \sigma_{zy}}{\partial y} = 0$$

Consider $\phi = \phi(x, y)$ such that $\sigma_{zx} = \frac{\partial \phi}{\partial y}$ $\sigma_{zy} = -\frac{\partial \phi}{\partial x}$

Hence we let $\phi(x, y)$ Stress function for torsion.

~~on cylindrical surface~~

B.C. for any surface $\mathbf{n} \cdot \mathbf{v} = 0$

$$\mathbf{n} \cdot [\sigma_{zx} \mathbf{e}_z \mathbf{e}_x + \sigma_{zy} \mathbf{e}_z \mathbf{e}_y + \sigma_{zx} \mathbf{e}_x \mathbf{e}_z + \sigma_{zy} \mathbf{e}_y \mathbf{e}_z] = 0$$

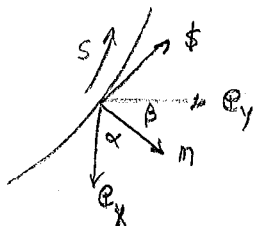
B.C. for cylindrical surface

$$\text{for a cylindrical surface } \mathbf{n} \cdot \mathbf{e}_z = 0$$

$$\therefore \mathbf{n} \cdot \mathbf{v} = 0 \Rightarrow (\mathbf{n} \cdot \mathbf{e}_x \sigma_{zx} + \mathbf{n} \cdot \mathbf{e}_y \sigma_{zy}) \mathbf{e}_z = 0$$

$$\text{or } \mathbf{n} \cdot \mathbf{e}_x \sigma_{zx} + \mathbf{n} \cdot \mathbf{e}_y \sigma_{zy} = 0$$

Look at the following section of the surface



$$\mathbf{n} \cdot \mathbf{e}_x = \mathbf{s} \cdot \mathbf{e}_y = \frac{dy}{ds} = \frac{dx}{dn} \quad \begin{array}{c} ds \\ \alpha \\ dy \end{array} \quad \begin{array}{c} dx \\ \alpha \\ dy \end{array}$$

$$\mathbf{n} \cdot \mathbf{e}_y = \mathbf{s} \cdot (-\mathbf{e}_x) = -\frac{dx}{ds} = -\frac{dy}{dn}$$

$$\therefore \mathbf{n} \cdot \mathbf{e}_x \sigma_{zx} + \mathbf{n} \cdot \mathbf{e}_y \sigma_{zy} = 0$$

$$\frac{dy}{ds} \cdot \frac{\partial \phi}{\partial y} + -\frac{dx}{ds} \cdot \frac{\partial \phi}{\partial x} = \frac{d\phi}{ds} = 0 \Rightarrow \phi = \text{constant along the}$$

boundary.

B.C. at the end faces $\mathbf{n} \cdot \mathbf{v} = \pm \mathbf{e}_z \cdot \mathbf{v} = \pm [\sigma_{zx} \mathbf{e}_x + \sigma_{zy} \mathbf{e}_y]$



Now on each end we want the resultant forces

we will show them to be $\equiv 0$

$$\therefore \iint_A \sigma_{zx} dA = \iint_A \frac{\partial \phi}{\partial y} dA = F_x$$

$$\iint_A \sigma_{zy} dA = - \iint_A \frac{\partial \phi}{\partial x} dA = F_y$$

Recalling Green's theorem

$$\iint_A \left(\frac{\partial Q}{\partial x} - \frac{\partial P}{\partial y} \right) dA = \oint_{\text{boundary}} P dx + Q dy$$

$$\Rightarrow \text{let } Q = 0 \quad -P = \phi \quad \text{for } F_x$$

$$\phi = Q \quad P = 0 \quad \text{for } F_y$$

Evaluation of

$$F_s \Rightarrow - \oint_{\text{boundary}} \phi dx = A_1 \oint_{\text{boundary}} \phi dx; - \oint_{\text{boundary}} \phi dy = A_1 \oint_{\text{boundary}} \phi dy \quad \text{but } \oint dx = 0 \quad \oint dy = 0 \quad \text{since } \phi = \text{const on surface} \Rightarrow F_x = F_y = 0$$

On the end faces there will be no resultant force

Resultant torque

$$\vec{T} = \iint \vec{r} \times (\vec{m} \cdot \vec{\sigma}) dA$$

$$\vec{r} = x\vec{e}_x + y\vec{e}_y$$

$$\vec{r} \times (\vec{m} \cdot \vec{\sigma}) = \pm [x\sigma_{zy} - y\sigma_{zx}] \vec{e}_z$$

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Resultant Torque

$$\vec{T} = \iint_A \vec{r} \times (\vec{m} \cdot \vec{\sigma}) dA$$

$$\vec{r} = x\vec{e}_x + y\vec{e}_y$$

$$\pm \vec{e}_z \cdot \vec{\sigma} = \vec{m} \cdot \vec{\sigma} = \pm [\vec{e}_x \sigma_{zx} + \vec{e}_y \sigma_{zy}]$$

$$\vec{r} \times (\vec{m} \cdot \vec{\sigma}) = \pm \vec{e}_z [x\sigma_{zy} - y\sigma_{zx}]$$

$$\therefore T = \int_A (x\sigma_{zy} - y\sigma_{zx}) dA = - \iint (x \frac{\partial \phi}{\partial x} + y \frac{\partial \phi}{\partial y}) dx dy$$

we rewrite

$$T = \iint_A [2\phi - \frac{\partial}{\partial x}(\phi x) - \frac{\partial}{\partial y}(\phi y)] dx dy$$

now we use green's theorem $\iint_A (\frac{\partial Q}{\partial x} - \frac{\partial P}{\partial y}) dA = \oint_C P dx + Q dy$

$$T = \iint_{A_{\text{net}}} 2\phi dA - \oint_{C_0} \phi_0 x dy + \oint_{C_0} \phi_0 y dx - \sum_i \phi_i \oint_{C_i} x dy + \sum_i \phi_i \oint_{C_i} y dx$$

$$- \phi_0 \oint_{C_0} [x dy - y dx] - \sum_i \phi_i \oint_{C_i} [x dy - y dx]$$

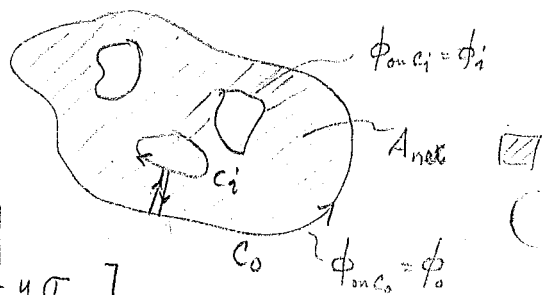
and by using green's theorem in reverse

$$- \phi_0 \iint_{A_{\text{gross}}} 2 dA$$

$$- \sum_i \phi_i \iint_{A_i} -2 dA$$

since this is a clockwise integration of Green's theorem

$$T = 2 \left[\iint_{A_{\text{net}}} \phi dA - \phi_0 A_{\text{gross}} + \sum_i \phi_i A_i \right] \quad \text{where } A_{\text{gross}} = A_{\text{net}} + \sum_i A_i$$



Strains and Displacements - we will now get strain and displacement solutions

$$\epsilon_{ij} = \frac{1+\nu}{E} \sigma_{ij} - \frac{\nu}{E} \sigma_{kk} \delta_{ij}$$

$$\epsilon_{xx} = \epsilon_{yy} = \epsilon_{zz} = \epsilon_{xy} = 0 \quad \text{and} \quad \epsilon_{yz} = \frac{1+\nu}{E} \sigma_{zy}, \quad \epsilon_{zx} = \frac{1+\nu}{E} \sigma_{zx}$$

$$\epsilon_{ij} = \frac{1}{2} (u_{ij} + u_{ji})$$

$$\epsilon_{xx} = \frac{\partial u_x}{\partial x} = 0 \Rightarrow u_x = u_x(y, z)$$

$$\epsilon_{yy} = \frac{\partial u_y}{\partial y} = 0 \Rightarrow u_y = u_y(x, z)$$

$$\epsilon_{zz} = \frac{\partial u_z}{\partial z} = 0 \Rightarrow u_z = u_z(x, y)$$

$$2\epsilon_{xy} = (u_{x,y} + u_{y,x}) = 0 \Rightarrow u_{x,y} = -f(z) \text{ only} \quad u_{y,x} = f(z)$$

integration of $u_{x,y}$ and $u_{y,x}$ gives

$$\therefore u_y(x, z) = x f(z) + g_y(z) \quad u_x(y, z) = -y f(z) + g_x(z)$$

look at $\frac{\partial u_x}{\partial z} = 2\epsilon_{zx} - \frac{\partial u_z}{\partial x}$

this equation $\frac{\partial^2 u_x}{\partial z^2} = \frac{\partial}{\partial z} (2\epsilon_{zx} - \frac{\partial u_z}{\partial x})$ (note that $\epsilon_{zx} = \frac{1}{2} (u_{x,z} + u_{z,x})$) Now take $\frac{\partial}{\partial z}$ of

$$\frac{\partial^2 u_x}{\partial z^2} = \frac{\partial}{\partial z} (2\epsilon_{zx} - \frac{\partial u_z}{\partial x})$$

since $u_z = u_z(x, y)$ and $\frac{\partial}{\partial z} u_z = 0$ and since $\sigma_{zx} = \sigma_{zx}(x, y)$ only $\Rightarrow \epsilon_{zx} = \epsilon_{zx}(x, y)$ only and $\frac{\partial \epsilon_{zx}}{\partial z} = 0$.

$$\therefore \frac{\partial^2 u_x}{\partial z^2} = 0 = -y f'' + g_x'' = 0 \Rightarrow f'' = 0 \text{ and } g_x'' = 0 \quad \forall y$$

$$\text{hence } f(z) = az + b \quad g_x(z) = cz + d.$$

We can get an analogous result for ϵ_{yz} : since $\epsilon_{zy} = \epsilon_{zy}(x, y)$ from fact that $\sigma_{zy}(x, y)$ only $\therefore \frac{\partial \epsilon_{zy}}{\partial z} = 0$ and $\frac{\partial^2 u_z}{\partial y \partial z} = 0 \Rightarrow$

$$u_y = axz + bx + cz + f \quad u_x = -ayz - by + cz + d.$$

B.C.

At the origin as a reference point $u_x, u_y = 0 \Rightarrow f, d = 0$ No r.b. transl.

at origin slopes wrt $z=0$ $u_{x,z} = u_{y,z} = 0 \Rightarrow c, e = 0$ No r.b. rot

Specify that rotation about the z -axis will be measured from the origin $w_{xy}|_{z=0} = 0$

$$\omega_{xy} \Big|_{z=0} = \frac{1}{2} \left(\frac{\partial u_y}{\partial x} - \frac{\partial u_x}{\partial y} \right) = \frac{1}{2} (az+b+az+b) = az+b \Big|_{z=0} = 0 \Rightarrow b=0$$

Let $a = \alpha$ (the twist) hence we go to displ. eqns and finally get.

$$u_y = \alpha x z \quad u_x = -\alpha y z$$

$$\text{note that } \frac{\partial u_z}{\partial y} + \frac{\partial u_y}{\partial z} = 2\varepsilon_{yz} = -2 \left(\frac{1+\nu}{E} \right) \frac{\partial \phi}{\partial x}$$

$$\frac{\partial u_z}{\partial y} = -2 \left(\frac{1+\nu}{E} \right) \frac{\partial \phi}{\partial x} - \alpha x \quad \left(\text{since } \frac{\partial u_y}{\partial z} = -\alpha x \right)$$

$$\text{also } \frac{\partial u_z}{\partial x} = 2 \left(\frac{1+\nu}{E} \right) \frac{\partial \phi}{\partial y} + \alpha y$$

$$\frac{\partial u_z}{\partial z} = 0 \quad \text{since } u_z = u_z(x, y) \text{ only}$$

we can now get u_z by integrating

Since we have a multiply connected region we must use Cauchy's theorem.

$$\begin{aligned} \therefore \oint_C du &= 0 \Rightarrow \oint_C \frac{\partial u_x}{\partial x} dx + \frac{\partial u_x}{\partial y} dy + \frac{\partial u_x}{\partial z} dz = \oint_C du_x = 0 \\ &\quad - \alpha z dy - \alpha y dz = -\alpha \oint_C z dx + y dz = -\alpha \cdot 0 = 0 \end{aligned}$$

by Green's theorem

hence by Cauchy's theorem the displ are single valued in x direction
we can do same in y direction

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we have found so far $u_x = -\alpha y z$ and $u_y = \alpha x z$

$$\text{now } \frac{\partial u_z}{\partial y} = -2 \left(\frac{1+\nu}{E} \right) \frac{\partial \phi}{\partial x} - \alpha x$$

$$\frac{\partial u_z}{\partial x} = 2 \left(\frac{1+\nu}{E} \right) \frac{\partial \phi}{\partial y} + \alpha y$$

$$\frac{\partial u_z}{\partial z} = 0$$

For single valuedness of u_z

$$\oint_C du_z = \oint_C \left(\frac{\partial u_z}{\partial x} dx + \frac{\partial u_z}{\partial y} dy + \frac{\partial u_z}{\partial z} dz \right) = \oint_{C_0} du_z + \sum_i \oint_{C_i} du_z$$

$$= \oint_C \left[\left(\frac{1}{\mu} \frac{\partial \phi}{\partial y} + \alpha y \right) dx - \left(\frac{1}{\mu} \frac{\partial \phi}{\partial x} + \alpha x \right) dy \right]$$

using Stokes Thm. the line integral is

$$= \iint_{A_{\text{gross}}} \left[\left(\frac{1}{\mu} \frac{\partial^2 \phi}{\partial x^2} + \alpha \right) + \left(\frac{1}{\mu} \frac{\partial^2 \phi}{\partial y^2} + \alpha \right) \right] dA - \sum_{i=0} \underbrace{\oint_{C_i} du_z}_{\text{cavities}}$$

For compatibility we must have

$$\frac{1}{\mu} \nabla_1^2 \phi + 2\alpha = 0 \quad \text{and} \quad \oint_{C_i} du_z = 0$$

or $\boxed{\nabla_1^2 \phi = -2\mu\alpha}$

now let us look at $\oint_{C_i} du_z = 0$

$$\oint_{C_i} du_z = \oint_{C_i} \left(\frac{\partial u_z}{\partial x} dx + \frac{\partial u_z}{\partial y} dy \right) + \alpha \oint_{C_i} (y dx - x dy)$$

From our previous lectures we related $m, \theta, \phi_x, \phi_y$ at the boundary so that

$$\frac{dy}{ds} = \frac{dx}{dn} \quad (1), \quad -\frac{dx}{ds} = \frac{dy}{dn} \quad (2) \quad \text{sub for } dy \text{ \& } dx \text{ in (1) \& (2) respectively}$$

hence the first part of the integral

$$\oint \left(\frac{\partial \phi}{\partial y} dx + \frac{\partial \phi}{\partial x} dy \right) = \oint \left(-\frac{\partial \phi}{\partial y} \frac{dy}{dn} - \frac{\partial \phi}{\partial x} \frac{dx}{dn} \right) ds = \oint -\frac{\partial \phi}{\partial n} ds$$

$$\oint y dx - x dy = - \oint (y dx - x dy) = 2 \oint dA = 2A_i$$

$$\therefore \oint_{C_i} du_z = -\frac{1}{\mu} \oint \frac{\partial \phi}{\partial n} ds + 2\alpha A_i = 0$$

$$\boxed{\oint_{C_i} \frac{\partial \phi}{\partial n} ds = 2\alpha A_i \mu}$$

on cavity bdy

Summary of the St Venant Torsion

Equilib - automatically satisfied by introducing $\phi(x, y)$ $\therefore \sigma_{zx} = \frac{\partial \phi}{\partial y}$, $\sigma_{zy} = -\frac{\partial \phi}{\partial x}$

B.C. we assumed that surface tractions on the surface = 0

$$\phi = K_i \text{ (const)} \quad (i=0, 1, \dots, N) \quad \begin{array}{l} K_0 \text{ on outer bdy} \\ K_i \text{ on } i^{\text{th}} \text{ cavity etc.} \end{array}$$

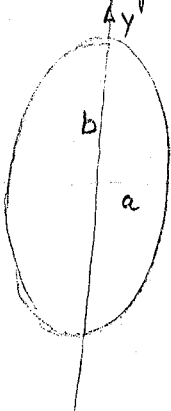
Torque $T = 2 \left[\iint_{A_{\text{net}}} \phi dA - \underbrace{K_0 A_0}_{\text{gross area}} + \sum_{i=1}^n K_i A_i \right] \quad \therefore A_0 - \sum_i A_i = A_{\text{net}}$

Compatibility $\nabla^2 \phi = -2\mu\alpha$

Additional conditions for shaft w/ cavities $\oint_{C_i} \phi \frac{\partial \phi}{\partial n} ds = 2\mu A_i \alpha$

Displacements $u_x = -\alpha y z$ $u_y = \alpha x z$ $u_z = f(x, y)$ this warping fn.

1st Example Torsion of a bar of elliptical cross section w/ no cavities



$$\frac{x^2}{a^2} + \frac{y^2}{b^2} = 1 \quad \text{Eqn of contour w/ } b > a$$

$\therefore$ since $\phi = \text{const}$ on the ellipse surface and since $\frac{x^2}{a^2} + \frac{y^2}{b^2} - 1 = 0$ on contour let us take $\phi = B \left(\frac{x^2}{a^2} + \frac{y^2}{b^2} - 1 \right)$

$\therefore$ Try $\phi = B \left(\frac{x^2}{a^2} + \frac{y^2}{b^2} - 1 \right)$ const on bdy

$\therefore$ since $\nabla^2 \phi = -2\mu\alpha = B \left[\frac{2}{a^2} + \frac{2}{b^2} \right] \therefore B = -\frac{\mu\alpha a^2 b^2}{a^2 + b^2}$

$T = 2 \iint \phi dA - K_0 A_0 = \iint \phi dA$ since $K_0 = 0$ on bdy

$T = 2 \iint \phi dA = -B\pi ab = \frac{\pi a^3 b^3 \mu\alpha}{a^2 + b^2} = D\alpha = \mu\alpha J$
 $J = \text{polar moment of inertia about } z \text{ axis}$

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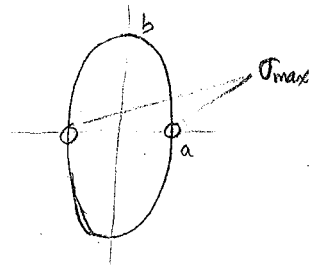
Final Exam : is due Monday the 11th of Dec will be given out next week

$$\sigma_{zx} = \frac{\partial \phi}{\partial y} = \frac{-2Ty}{\pi a b^3} \quad \sigma_{zy} = -\frac{\partial \phi}{\partial x} = \frac{2Tx}{\pi a^3 b}$$

$$\sigma_{res} = (\sigma_{zx}^2 + \sigma_{zy}^2)^{1/2} = \frac{2T}{\pi ab} \left(\frac{y^2}{b^4} + \frac{x^2}{a^4} \right)^{1/2} = \frac{2T}{\pi a^3 b^3} (b^4 x^2 + a^4 y^2)^{1/2}$$

$$\sigma_{max} (@ x = \pm a) = \frac{2T}{\pi a^2 b}$$

$\therefore$ if yield occurs at pts on surface where it is closest to origin



Now $u_x = -\alpha y z = -\frac{T(a^2 + b^2)}{\pi a^3 b^3 \mu} y z$

$$u_y = \alpha x z = \frac{T(a^2 + b^2)}{\pi a^3 b^3 \mu} x z$$

from our summary $\frac{\partial u_z}{\partial x} = \frac{1}{\mu} \frac{\partial \phi}{\partial y} + \alpha y = \frac{1}{\mu} \left(\frac{-2Ty}{\pi a b^3} \right) + \left(\frac{-T(a^2 + b^2)}{\pi a^3 b^3 \mu} \right) y$

$$\left| \frac{\partial u_z}{\partial x} = \frac{b^2 - a^2}{b^2 + a^2} \alpha y \right|$$

integrate

$$\frac{\partial u_z}{\partial y} = - \left(\frac{1}{\mu} \frac{\partial \phi}{\partial x} + \alpha x \right) = \frac{b^2 - a^2}{b^2 + a^2} \alpha x$$

Now $u_z = \frac{b^2 - a^2}{b^2 + a^2} \alpha x y + f_1(y)$

$$u_z = \frac{b^2 - a^2}{b^2 + a^2} \alpha x y + f_2(x)$$

$\therefore f_1(y) = f_2(x) = \text{const.}$

represents a rigid body disp in the z direct

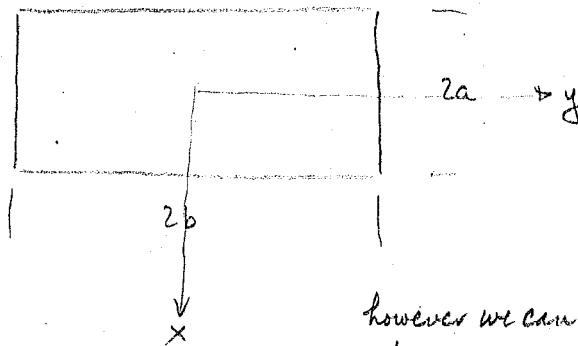
$\therefore u_z = \frac{b^2 - a^2}{b^2 + a^2} \alpha x y$

warping function



$$\left. \begin{aligned} u_x &= u_x(x, y) \\ u_y &= u_y(x, y) \\ u_z &= 0 \end{aligned} \right\} \text{plane strain problems.}$$

Torsion of a bar of rectangular cross-section



can we define a stress function ϕ s.t. $\nabla^2 \phi = -2\mu\alpha$?

we cannot find a single fn. of ϕ constant on body satisfying eq.

however we can construct solutions $\phi_0 = \mu\alpha(a^2 - x^2)$

$$\therefore \phi_0 = 0 \text{ on } x = \pm a$$

and we can construct a second ϕ_1 , $\therefore \nabla^2 \phi = \nabla^2 \phi_0 + \nabla^2 \phi_1 = -2\mu\alpha$
 thus $\nabla^2 \phi_1 = 0$ since $\nabla^2 \phi_0 = -2\mu\alpha$

$$\begin{aligned} \text{The B.C. @ } x = \pm a & \therefore \phi(\pm a, y) = \phi_1(\pm a, y) = 0 \\ y = \pm b & \phi(x, \pm b) = \mu\alpha(a^2 - x^2) + \phi_1(x, \pm b) = 0 \end{aligned}$$

since we want ϕ on bdy = 0

if we assume $\phi_1(x, y) = F_1(x) F_2(y)$ then $\nabla^2 \phi_1 = F_1'' F_2 + F_1 F_2'' = 0$

$$\text{thus } \frac{F_1''}{F_1} = -\frac{F_2''}{F_2} = \text{const} = -\beta^2$$

$$\begin{aligned} \therefore F_1'' + \beta^2 F_1 &= 0 \Rightarrow F_1 = A_1 \cos \beta x + B_1 \sin \beta x \\ F_2'' - \beta^2 F_2 &= 0 \Rightarrow F_2 = A_2 \cosh \beta y + B_2 \sinh \beta y \end{aligned}$$

Symmetry w/t $x, y \Rightarrow B_1 = 0$ and $B_2 = 0$ $\left(\begin{aligned} \sin \beta(-x) &= -\sin \beta x \\ \sinh \beta(-y) &= -\sinh \beta y \end{aligned} \right)$

$$\therefore \phi_1 = C \cos \beta x \cosh \beta y$$

$$\phi_1(\pm a, y) = C \cos \beta a \cosh \beta y = 0 \quad \therefore \beta a = \frac{(2k+1)\pi}{2} \quad (k=0, 1, 2, \dots)$$

$$\therefore \phi_1(x, y) = \sum_{k=0}^{\infty} C_k \cos \frac{(2k+1)\pi}{2a} x \cosh \frac{(2k+1)\pi}{2a} y$$

other B.C. $\phi_1(x, \pm b) = -\mu \alpha (a^2 - x^2)$

$$\therefore -\mu \alpha (a^2 - x^2) = \sum C_k \cos \frac{(2k+1)\pi}{2a} x \cosh \frac{(2k+1)\pi}{2a} b$$

$$\begin{aligned} \text{now } C_k \cosh \frac{(2k+1)\pi b}{2a} &= \frac{2}{a} \int_0^a -\mu \alpha (a^2 - x^2) \cos \frac{(2k+1)\pi}{2a} x \, dx \\ &= \frac{-32 \mu \alpha^2 (-1)^k}{(2k+1)^3 \pi^3} \end{aligned}$$

since only terms which will give results in fourier series is when $m = 2k+1$

$$\therefore C_k = \frac{-32 \mu \alpha a^2 (-1)^k}{(2k+1)^3 \pi^3}$$

$$\therefore \phi = \phi_0 + \phi_1 = -\mu \alpha \left[(x^2 - a^2) + \frac{32 a^2}{\pi^3} \sum_{k=0}^{\infty} (-1)^k \frac{\cos \frac{(2k+1)\pi x}{2a} \cosh \frac{(2k+1)\pi y}{2a}}{(2k+1)^3 \cosh \frac{(2k+1)\pi b}{2a}} \right]$$

σ_{yz} occurs at $x = \pm a, y = 0$

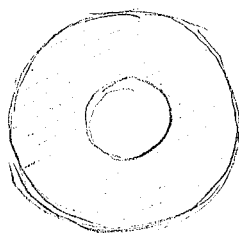
$$\sigma_{yz} = -\frac{\partial \phi}{\partial y} \bigg|_{\substack{x=a \\ y=0}} = 2\mu \alpha a \zeta \quad \text{where } \zeta = 1 - \frac{8}{\pi^2} \sum_{k=0}^{\infty} \frac{1}{(2k+1)^2 \cosh \frac{(2k+1)\pi b}{2a}}$$

b/a	ζ
1.0	.675
1.2	.759
1.5	.848
2.0	.930
2.5	.968
3.0	.985
5.0	.999
∞	1.0

for engineering purposes if $b/a > 2.5$ $\zeta \approx 1$

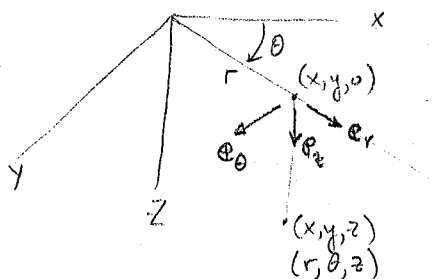
$$\therefore \sigma_{yz} = 2\mu \alpha a$$

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Orthogonal Curvilinear Coordinates will be discussed in order to do torsional problem of a hollow surface.

Cylindrical Coordinate systems.

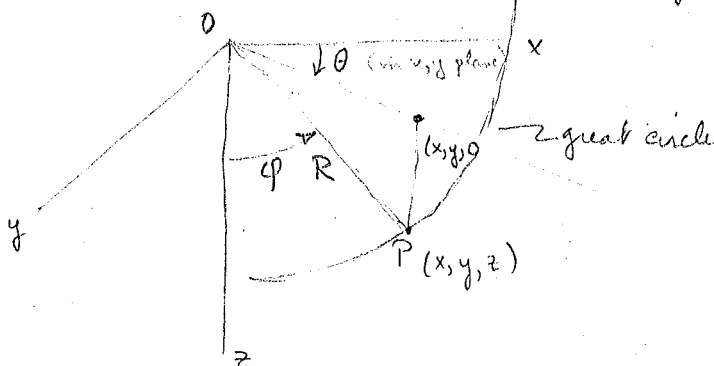


$$\begin{aligned} x &= r \cos \theta \\ y &= r \sin \theta \\ z &= z \end{aligned}$$

invertible

$$\begin{aligned} r &= \sqrt{x^2 + y^2} \\ \theta &= \arctan(y/x) \\ z &= z \end{aligned}$$

Spherical Coordinate System.



$$\begin{aligned} x &= R \sin \phi \cos \theta \\ y &= R \sin \phi \sin \theta \\ z &= R \cos \phi \end{aligned}$$

$$\begin{aligned} R &= \sqrt{x^2 + y^2 + z^2} \\ \theta &= \tan^{-1}(y/x) \\ \phi &= \arctan\left(\frac{\sqrt{x^2 + y^2}}{z}\right) \end{aligned}$$

General orthogonal curvilinear coordinates (α, β, γ)

$$\alpha = \alpha(x, y, z)$$

$$\beta = \beta(x, y, z)$$

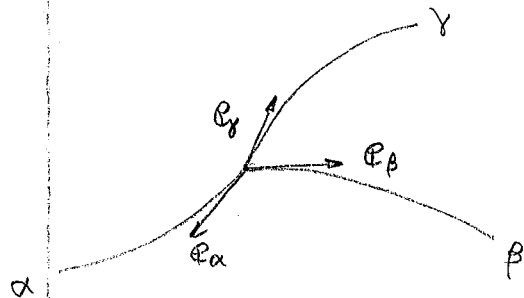
$$\gamma = \gamma(x, y, z)$$

look at $\alpha = \text{const}$ this defines a surface

look at $\beta = \text{const}$ " " " "

" " $\gamma = \text{const}$ " " " "

the intersection of these 3 surfaces defines a point p.



$$e_{\alpha_i} \cdot e_{\alpha_j} = \delta_{ij}, \quad e_{\alpha_i} \times e_{\alpha_j} = e_{\alpha_k} e_{ijk}$$

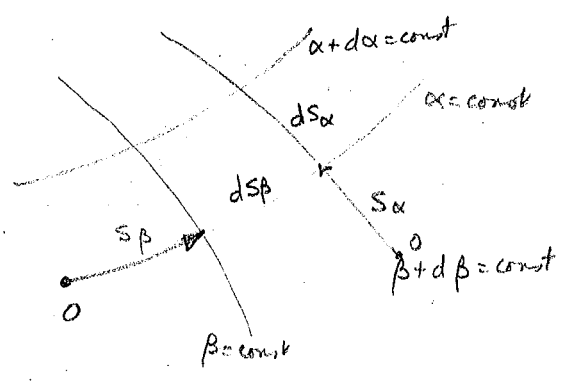
define the displacement vector $u = u_x e_x + u_y e_y + u_z e_z$

define a tensor $\sigma = \sigma_{\alpha\alpha} e_\alpha e_\alpha + \sigma_{\alpha\beta} e_\alpha e_\beta + \dots = \sigma_{\alpha_i \alpha_j} e_{\alpha_i} e_{\alpha_j}$

	x	y	z
α	$e_\alpha \cdot e_x$	$e_\alpha \cdot e_y$	
β		$e_\beta \cdot e_y$	
γ	$e_\gamma \cdot e_x$		$e_\gamma \cdot e_z$

This transforms items in (x, y, z) plane to (α, β, γ) plane

Now look at surface $\gamma = \text{const.}$



stretch function or stretch factor

$$ds_\alpha = h_\alpha(\alpha, \beta, \gamma) d\alpha \quad \frac{dx}{ds_\alpha} = \frac{1}{h_\alpha}$$

$$ds_\beta = h_\beta(\alpha, \beta, \gamma) d\beta$$

$$ds_\gamma = h_\gamma(\alpha, \beta, \gamma) d\gamma$$

since α, β, γ may not be a physical length (ie θ)

Looking at the position vector in the cartesian plane. $r = x e_x + y e_y + z e_z$ then $dr = dx e_x + dy e_y + dz e_z = e_\alpha ds_\alpha + e_\beta ds_\beta + e_\gamma ds_\gamma$ now we note that $\frac{\partial r}{\partial s_\alpha} = e_\alpha$

we can take $\frac{\partial r}{\partial s_\alpha} = \frac{\partial x}{\partial s_\alpha} e_x + \frac{\partial y}{\partial s_\alpha} e_y + \frac{\partial z}{\partial s_\alpha} e_z = e_\alpha$

now take the dot product of $e_\alpha \cdot e_\alpha = 1 = \left(\frac{\partial x}{\partial s_\alpha}\right)^2 + \left(\frac{\partial y}{\partial s_\alpha}\right)^2 + \left(\frac{\partial z}{\partial s_\alpha}\right)^2$

$$= \left(\frac{\partial x}{\partial \alpha} \cdot \frac{\partial \alpha}{\partial s_\alpha}\right)^2 + \left(\frac{\partial y}{\partial \alpha} \cdot \frac{\partial \alpha}{\partial s_\alpha}\right)^2 + \left(\frac{\partial z}{\partial \alpha} \cdot \frac{\partial \alpha}{\partial s_\alpha}\right)^2$$

$$= \frac{1}{h_\alpha^2} \left[\left(\frac{\partial x}{\partial \alpha}\right)^2 + \left(\frac{\partial y}{\partial \alpha}\right)^2 + \left(\frac{\partial z}{\partial \alpha}\right)^2 \right]$$

$\therefore h_\alpha = \sqrt{\left(\frac{\partial x}{\partial \alpha}\right)^2 + \left(\frac{\partial y}{\partial \alpha}\right)^2 + \left(\frac{\partial z}{\partial \alpha}\right)^2}$

$\therefore h_{\alpha_j} = \sqrt{\frac{\partial x_i}{\partial \alpha_j} \frac{\partial x_i}{\partial \alpha_j}} \quad \begin{matrix} \text{Sum over } i \\ \text{no sum over } j \end{matrix}$

$$h_r = \sqrt{\cos^2 + \sin^2} = 1$$

$$h_\theta = \sqrt{(-r \sin \theta)^2 + (r \cos \theta)^2} = r$$

$$h_z = 1$$

$$ds_\theta = h_\theta d\theta = r d\theta$$

$$ds_r = dr$$

$$ds_z = dz$$

for spherical

$$h_r = 1$$

$$h_\theta = R \sin \varphi$$

$$h_\varphi = R$$

$$\therefore ds_r = dr$$

$$ds_\theta = R \sin \varphi d\theta$$

$$ds_\varphi = R d\varphi$$

Direction cosines

$$l_{xy} = \mathbf{e}_x \cdot \mathbf{e}_y = \frac{\partial y}{\partial s_x} = \frac{1}{h_x} \cdot \frac{\partial y}{\partial x}$$

$$\therefore l_{x_i x_j} = \mathbf{e}_{x_i} \cdot \mathbf{e}_j = \frac{1}{h_{x_i}} \frac{\partial x_j}{\partial x_i} \quad \text{; not summed.}$$

$$\mathbf{e}_x = \frac{1}{h_x} \frac{\partial x}{\partial x} \mathbf{e}_x + \frac{1}{h_x} \frac{\partial y}{\partial x} \mathbf{e}_y + \frac{1}{h_x} \frac{\partial z}{\partial x} \mathbf{e}_z$$

for cylindrical coord.

$$\mathbf{e}_z = \mathbf{e}_z$$

$$\mathbf{e}_r = \cos \theta \mathbf{e}_x + \sin \theta \mathbf{e}_y$$

$$\mathbf{e}_\theta = -\sin \theta \mathbf{e}_x + \cos \theta \mathbf{e}_y$$

$$\frac{\partial \mathbf{e}_r}{\partial \theta} = \mathbf{e}_\theta$$

$$\frac{\partial \mathbf{e}_\theta}{\partial \theta} = -\mathbf{e}_r$$

for spherical coordinates

$$\mathbf{e}_R = \sin \varphi \cos \theta \mathbf{e}_x + \sin \varphi \sin \theta \mathbf{e}_y + \cos \varphi \mathbf{e}_z$$

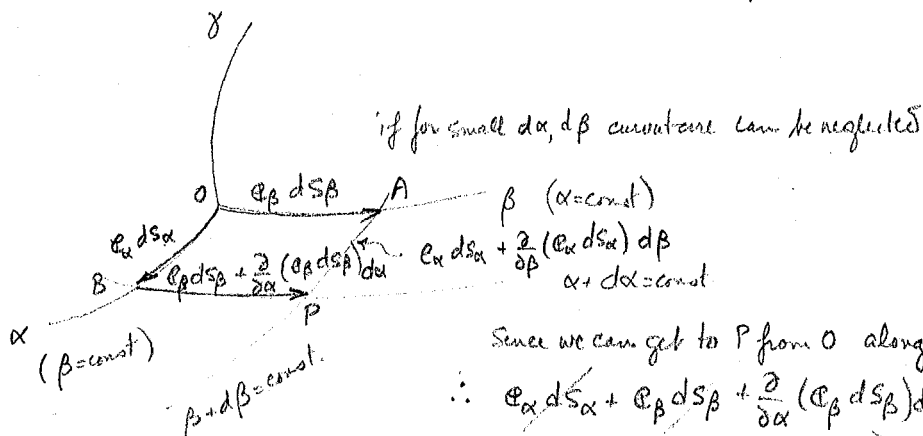
$$\mathbf{e}_\theta = -\sin \theta \mathbf{e}_x + \cos \theta \mathbf{e}_y$$

$$\mathbf{e}_\varphi = \cos \varphi \cos \theta \mathbf{e}_x + \cos \varphi \sin \theta \mathbf{e}_y - \sin \varphi \mathbf{e}_z$$

$$\frac{\partial \mathbf{e}_R}{\partial \theta} = \sin \varphi \mathbf{e}_\theta, \quad \frac{\partial \mathbf{e}_R}{\partial \varphi} = \mathbf{e}_\varphi$$

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Back to ^{orthogonal} curvilinear coord. on constant γ surfaces



Since we can get to P from O along OA+OP or along OB+BP
 $\therefore e_\alpha d\alpha + e_\beta d\beta + \frac{\partial}{\partial \alpha} (e_\beta d\beta) d\alpha = e_\beta d\beta + e_\alpha d\alpha + \frac{\partial}{\partial \beta} (e_\alpha d\alpha) d\beta$

now divide both sides by $d\alpha d\beta$ and remembering that $dS_\alpha = h_\alpha d\alpha$ we get

$$\frac{\partial}{\partial \alpha} (e_\beta h_\beta d\beta) d\alpha = \frac{\partial}{\partial \alpha} (e_\beta h_\beta) d\beta d\alpha = \frac{\partial}{\partial \beta} (e_\alpha h_\alpha d\alpha) d\beta = \frac{\partial}{\partial \beta} (e_\alpha h_\alpha) d\alpha d\beta$$

hence we get $h_\alpha \frac{\partial e_\alpha}{\partial \beta} + e_\alpha \frac{\partial h_\alpha}{\partial \beta} = e_\beta \frac{\partial h_\beta}{\partial \alpha} + h_\beta \frac{\partial e_\beta}{\partial \alpha}$

now take dot product w/ e_β and remembering that $\frac{\partial e_\beta}{\partial \alpha} \perp e_\beta$ and $e_\alpha \cdot e_\beta = 0$ since orthog.

then $\frac{\partial h_\beta}{\partial \alpha} = h_\alpha e_\beta \cdot \frac{\partial e_\alpha}{\partial \beta}$

remembering that $\frac{dS_\alpha}{d\alpha} = h_\alpha$
 rewrite as $\frac{1}{h_\alpha} \frac{\partial h_\beta}{\partial \alpha} = e_\beta \cdot \frac{\partial e_\alpha}{\partial \beta}$ or $\frac{\partial h_\beta}{\partial S_\alpha} = e_\beta \cdot \frac{\partial e_\alpha}{\partial \beta} \frac{h_\beta}{h_\beta} = h_\beta e_\beta \cdot \frac{\partial e_\alpha}{\partial \beta}$ and dividing by h_β then $\frac{1}{h_\beta} \frac{\partial h_\beta}{\partial S_\alpha} = \frac{\partial (\ln h_\beta)}{\partial S_\alpha}$ then $\frac{\partial e_\alpha}{\partial \beta} = \frac{\partial (\ln h_\beta)}{\partial S_\alpha}$ and by cyclic permutation

$$e_\gamma \cdot \frac{\partial e_\beta}{\partial S_\gamma} = \frac{\partial (\ln h_\gamma)}{\partial S_\beta}$$

$$e_\alpha \cdot \frac{\partial e_\gamma}{\partial S_\alpha} = \frac{\partial (\ln h_\alpha)}{\partial S_\gamma}$$

also we can get that since $e_\alpha \cdot e_\beta = 0$ then $\frac{\partial}{\partial S_\alpha}$ of $e_\alpha \cdot e_\beta = 0$ or

$$e_\alpha \cdot \frac{\partial e_\beta}{\partial S_\alpha} + \frac{\partial e_\alpha}{\partial S_\alpha} \cdot e_\beta = 0$$

$$e_\beta \cdot \frac{\partial e_\alpha}{\partial S_\alpha} + \frac{\partial e_\beta}{\partial S_\alpha} \cdot e_\alpha = 0$$

$$\left. \begin{array}{l} e_\beta \cdot \frac{\partial e_\alpha}{\partial S_\alpha} = -e_\alpha \cdot \frac{\partial e_\beta}{\partial S_\alpha} = -\frac{\partial (\ln h_\alpha)}{\partial S_\beta} \\ \text{Since } e_\alpha \cdot e_\beta = e_\beta \cdot e_\alpha \text{ we can use the above identities } \alpha, \beta \text{ get.} \end{array} \right\}$$

and by cyclic permutation $e_\beta \cdot \frac{\partial e_\alpha}{\partial S_\beta} = \frac{\partial (\ln h_\beta)}{\partial S_\alpha}$ also $e_\beta \cdot \frac{\partial e_\alpha}{\partial S_\gamma} = 0$

since $\nabla \cdot e_\alpha = 0$

Now since

$$\nabla = e_x \frac{\partial}{\partial x} + e_y \frac{\partial}{\partial y} + e_z \frac{\partial}{\partial z}$$

can write $\nabla = e_\alpha \frac{\partial}{\partial s_\alpha} + e_\beta \frac{\partial}{\partial s_\beta} + e_\gamma \frac{\partial}{\partial s_\gamma}$ since the s_j represent length changes we or using the stretch factors

$$\nabla = \frac{1}{h_\alpha} e_\alpha \frac{\partial}{\partial \alpha} + e_\beta \frac{1}{h_\beta} \frac{\partial}{\partial \beta} + \frac{1}{h_\gamma} e_\gamma \frac{\partial}{\partial \gamma}$$

to get any general strain we know that $\epsilon_{\alpha_i \alpha_j} = e_{\alpha_i} \cdot \epsilon \cdot e_{\alpha_j}$

we also know that $\epsilon = \frac{1}{2} (\nabla u + u \nabla)$

$$= \frac{1}{2} \left(e_\alpha \frac{\partial u}{\partial s_\alpha} + e_\beta \frac{\partial u}{\partial s_\beta} + e_\gamma \frac{\partial u}{\partial s_\gamma} + \frac{\partial u}{\partial s_\alpha} e_\alpha + \frac{\partial u}{\partial s_\beta} e_\beta + \frac{\partial u}{\partial s_\gamma} e_\gamma \right)$$

now if we look at one particular term (viz $\epsilon_{\alpha\alpha}$)

$$\epsilon_{\alpha\alpha} = e_\alpha \cdot \epsilon \cdot e_\alpha \quad \text{and let } u = u_\alpha e_\alpha + u_\beta e_\beta + u_\gamma e_\gamma$$

Substitution and differentiation w/some conditions on the derivatives of the unit vectors gives for example for $\frac{\partial u}{\partial s_\alpha} = \frac{\partial (u_\alpha e_\alpha + u_\beta e_\beta + u_\gamma e_\gamma)}{\partial s_\alpha}$

$$\frac{\partial u}{\partial s_\alpha} = \frac{\partial u_\alpha}{\partial s_\alpha} e_\alpha + \frac{\partial u_\beta}{\partial s_\alpha} e_\beta + \frac{\partial u_\gamma}{\partial s_\alpha} e_\gamma + u_\alpha \frac{\partial e_\alpha}{\partial s_\alpha} + u_\beta \frac{\partial e_\beta}{\partial s_\alpha} + u_\gamma \frac{\partial e_\gamma}{\partial s_\alpha}$$

hence we get

$$\epsilon_{\alpha\alpha} = \frac{1}{2} \left(\frac{\partial u}{\partial s_\alpha} \cdot e_\alpha + e_\alpha \cdot \frac{\partial u}{\partial s_\alpha} \right) = \frac{1}{2} \left[2 \frac{\partial u_\alpha}{\partial s_\alpha} + 2 u_\beta \frac{\partial e_\beta}{\partial s_\alpha} \cdot e_\alpha + 2 u_\gamma \frac{\partial e_\gamma}{\partial s_\alpha} \cdot e_\alpha \right]$$

$$\epsilon_{\alpha\alpha} = \frac{\partial u_\alpha}{\partial s_\alpha} + u_\beta \frac{\partial (\ln h_\beta)}{\partial s_\alpha} + u_\gamma \frac{\partial (\ln h_\gamma)}{\partial s_\alpha}$$

$$\text{and } \left[\epsilon_{\alpha\alpha} = \frac{1}{h_\alpha} \left[\frac{\partial u_\alpha}{\partial \alpha} + \frac{u_\beta}{h_\beta} \frac{\partial h_\alpha}{\partial \beta} + \frac{u_\gamma}{h_\gamma} \frac{\partial h_\alpha}{\partial \gamma} \right] \right] \quad \text{using } \partial s_{\alpha_i} = h_{\alpha_i} \partial \alpha_i$$

and for the shear strains

$$\left[\epsilon_{\alpha\beta} = \frac{1}{2} \left[\frac{h_\beta}{h_\alpha} \frac{\partial}{\partial \alpha} \left(\frac{u_\beta}{h_\beta} \right) + \frac{h_\alpha}{h_\beta} \frac{\partial}{\partial \beta} \left(\frac{u_\alpha}{h_\alpha} \right) \right] \right] \quad \text{can cyclic permute}$$

$$\text{obtained from } 2\epsilon_{\alpha\beta} = e_\alpha \cdot \epsilon \cdot e_\beta + e_\beta \cdot \epsilon \cdot e_\alpha = \frac{\partial u}{\partial s_\alpha} \cdot e_\beta + e_\beta \cdot \frac{\partial u}{\partial s_\alpha} = \frac{\partial u_\beta}{\partial s_\alpha} - \frac{\partial (\ln h_\beta)}{\partial s_\alpha} u_\alpha + \frac{\partial u_\alpha}{\partial s_\beta} - \frac{\partial (\ln h_\alpha)}{\partial s_\beta} u_\beta$$

Now we define the dilatation $\Delta = \nabla \cdot u = \epsilon_{\alpha\alpha} + \epsilon_{\beta\beta} + \epsilon_{\gamma\gamma}$ using cyclic permutation to get $\epsilon_{\beta\beta}, \epsilon_{\gamma\gamma}$

$$\therefore \Delta = \frac{1}{h_\alpha} \left[\frac{\partial u_\alpha}{\partial \alpha} + \frac{u_\beta}{h_\beta} \frac{\partial h_\alpha}{\partial \beta} + \frac{u_\gamma}{h_\gamma} \frac{\partial h_\alpha}{\partial \gamma} \right] + \frac{1}{h_\beta} \left[\frac{\partial u_\beta}{\partial \beta} + \frac{u_\alpha}{h_\alpha} \frac{\partial h_\beta}{\partial \alpha} + \frac{u_\gamma}{h_\gamma} \frac{\partial h_\beta}{\partial \gamma} \right]$$

$$+ \frac{1}{h_\gamma} \left[\frac{\partial u_\gamma}{\partial \gamma} + \frac{u_\alpha}{h_\alpha} \frac{\partial h_\gamma}{\partial \alpha} + \frac{u_\beta}{h_\beta} \frac{\partial h_\gamma}{\partial \beta} \right] = \frac{1}{h_\alpha h_\beta h_\gamma} \left[\frac{\partial}{\partial \alpha} [u_\alpha h_\beta h_\gamma] + \frac{\partial}{\partial \beta} [u_\beta h_\alpha h_\gamma] + \frac{\partial}{\partial \gamma} [u_\gamma h_\alpha h_\beta] \right]$$

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We summarize relations (34), (37) and a third relation which is proved in the homework. The following relations are also valid for a cyclic permutation of the subscripts α, β, γ . We have

$$\mathbf{e}_\beta \cdot \frac{\partial \mathbf{e}_\alpha}{\partial s_\alpha} = - \frac{\partial (\ln h_\alpha)}{\partial s_\beta} \quad (38)$$

$$\mathbf{e}_\beta \cdot \frac{\partial \mathbf{e}_\alpha}{\partial s_\beta} = \frac{\partial (\ln h_\beta)}{\partial s_\alpha} \quad (39)$$

$$\mathbf{e}_\beta \cdot \frac{\partial \mathbf{e}_\alpha}{\partial s_\gamma} = 0 \quad (40)$$

In Cartesian coordinates we define the operator

$$\nabla = \mathbf{e}_x \frac{\partial}{\partial x} + \mathbf{e}_y \frac{\partial}{\partial y} + \mathbf{e}_z \frac{\partial}{\partial z} \quad (41)$$

We may show (see homework) that the operator transformed to the orthogonal curvilinear coordinate system becomes

$$\nabla = \mathbf{e}_\alpha \frac{\partial}{\partial s_\alpha} + \mathbf{e}_\beta \frac{\partial}{\partial s_\beta} + \mathbf{e}_\gamma \frac{\partial}{\partial s_\gamma} \quad (42)$$

We recall the definition of the small strain tensor

$$\boldsymbol{\epsilon} = \frac{1}{2} (\nabla \mathbf{U} + \mathbf{U} \nabla) \quad (43)$$

and any component of strain in orthogonal curvilinear coordinates is

$$\epsilon_{\alpha_i \alpha_j} = \mathbf{e}_{\alpha_i} \cdot \boldsymbol{\epsilon} \cdot \mathbf{e}_{\alpha_j} \quad (44)$$

From the form (42) for ~~a vector~~ ^{the operator} and from definition (43) we write the small strain tensor in orthogonal curvilinear coordinates as

$$\boldsymbol{\epsilon} = \frac{1}{2} \left[\mathbf{e}_\alpha \frac{\partial \mathbf{U}}{\partial s_\alpha} + \mathbf{e}_\beta \frac{\partial \mathbf{U}}{\partial s_\beta} + \mathbf{e}_\gamma \frac{\partial \mathbf{U}}{\partial s_\gamma} + \frac{\partial \mathbf{U}}{\partial s_\alpha} \mathbf{e}_\alpha + \frac{\partial \mathbf{U}}{\partial s_\beta} \mathbf{e}_\beta + \frac{\partial \mathbf{U}}{\partial s_\gamma} \mathbf{e}_\gamma \right] \quad (45)$$

Note that each term in (45) is a product of two vectors (a dyad) and hence the order must be preserved. We now write one component of the ϵ tensor, $\epsilon_{\alpha\alpha}$, thus

$$\epsilon_{\alpha\alpha} = e_{\alpha} \cdot \epsilon \cdot e_{\alpha} \quad \text{with } \epsilon \text{ as given by (45). Note also}$$

$$U = U_{\alpha} e_{\alpha} + U_{\beta} e_{\beta} + U_{\gamma} e_{\gamma}$$

and

$$\begin{aligned} \epsilon_{\alpha\alpha} = \frac{1}{2} \left\{ \left[\frac{\partial U}{\partial S_{\alpha}} + \left(\frac{\partial U_{\alpha}}{\partial S_{\alpha}} + U_{\beta} e_{\alpha} \cdot \frac{\partial e_{\beta}}{\partial S_{\alpha}} + U_{\gamma} e_{\alpha} \cdot \frac{\partial e_{\gamma}}{\partial S_{\alpha}} \right) e_{\alpha} \right. \right. \\ + \left(\frac{\partial U_{\alpha}}{\partial S_{\beta}} + U_{\beta} e_{\alpha} \cdot \frac{\partial e_{\beta}}{\partial S_{\beta}} + U_{\gamma} e_{\alpha} \cdot \frac{\partial e_{\gamma}}{\partial S_{\beta}} \right) e_{\beta} \\ \left. \left. + \left(\frac{\partial U_{\alpha}}{\partial S_{\gamma}} + U_{\beta} e_{\alpha} \cdot \frac{\partial e_{\beta}}{\partial S_{\gamma}} + U_{\gamma} e_{\alpha} \cdot \frac{\partial e_{\gamma}}{\partial S_{\gamma}} \right) e_{\gamma} \right] \cdot e_{\alpha} \right\} \quad (46) \end{aligned}$$

$$\text{Note that } \frac{\partial U}{\partial S_{\alpha}} = \frac{\partial U_{\alpha}}{\partial S_{\alpha}} e_{\alpha} + \frac{\partial U_{\beta}}{\partial S_{\alpha}} e_{\beta} + \frac{\partial U_{\gamma}}{\partial S_{\alpha}} e_{\gamma} + U_{\beta} \frac{\partial e_{\alpha}}{\partial S_{\alpha}} + U_{\gamma} \frac{\partial e_{\alpha}}{\partial S_{\alpha}} + U_{\alpha} \frac{\partial e_{\alpha}}{\partial S_{\alpha}} \quad (47)$$

so that (46) becomes

$$\epsilon_{\alpha\alpha} = \frac{1}{2} \left[\frac{\partial U_{\alpha}}{\partial S_{\alpha}} + U_{\beta} e_{\alpha} \cdot \frac{\partial e_{\beta}}{\partial S_{\alpha}} + U_{\gamma} e_{\alpha} \cdot \frac{\partial e_{\gamma}}{\partial S_{\alpha}} + \frac{\partial U_{\alpha}}{\partial S_{\alpha}} + U_{\beta} e_{\alpha} \cdot \frac{\partial e_{\beta}}{\partial S_{\alpha}} + U_{\gamma} e_{\alpha} \cdot \frac{\partial e_{\gamma}}{\partial S_{\alpha}} \right]$$

We make use of (39) and its permutations to write

$$\epsilon_{\alpha\alpha} = \frac{\partial U_{\alpha}}{\partial S_{\alpha}} + U_{\beta} \frac{\partial (\ln h_{\alpha})}{\partial S_{\beta}} + U_{\gamma} \frac{\partial (\ln h_{\alpha})}{\partial S_{\gamma}} \quad (48)$$

Finally we write the components of normal strain as

$$\epsilon_{\alpha\alpha} = \frac{1}{h_{\alpha}} \left[\frac{\partial U_{\alpha}}{\partial \alpha} + \frac{U_{\beta}}{h_{\beta}} \frac{\partial h_{\alpha}}{\partial \beta} + \frac{U_{\gamma}}{h_{\gamma}} \frac{\partial h_{\alpha}}{\partial \gamma} \right] \quad (49)$$

$$\epsilon_{\beta\beta} = \frac{1}{h_{\beta}} \left[\frac{\partial U_{\beta}}{\partial \beta} + \frac{U_{\gamma}}{h_{\gamma}} \frac{\partial h_{\beta}}{\partial \gamma} + \frac{U_{\alpha}}{h_{\alpha}} \frac{\partial h_{\beta}}{\partial \alpha} \right] \quad (50)$$

$$\epsilon_{\gamma\gamma} = \frac{1}{h_{\gamma}} \left[\frac{\partial U_{\gamma}}{\partial \gamma} + \frac{U_{\alpha}}{h_{\alpha}} \frac{\partial h_{\gamma}}{\partial \alpha} + \frac{U_{\beta}}{h_{\beta}} \frac{\partial h_{\gamma}}{\partial \beta} \right] \quad (51)$$

Also, we have

$$\epsilon_{\alpha\beta} = \frac{1}{2} \left[\frac{h_{\beta}}{h_{\alpha}} \frac{\partial}{\partial \alpha} \left(\frac{U_{\beta}}{h_{\beta}} \right) + \frac{h_{\alpha}}{h_{\beta}} \frac{\partial}{\partial \beta} \left(\frac{U_{\alpha}}{h_{\alpha}} \right) \right] \quad (52)$$

We write the dilatation using the components of normal strain as

$$\Delta = \nabla \cdot \mathbf{U} = U_{\alpha\alpha} = \epsilon_{\alpha\alpha} + \epsilon_{\beta\beta} + \epsilon_{\gamma\gamma}$$

or

$$\begin{aligned} \Delta = & \frac{1}{h_\alpha} \frac{\partial U_\alpha}{\partial \alpha} + \frac{U_\alpha}{h_\alpha h_\beta} \frac{\partial h_\beta}{\partial \alpha} + \frac{U_\alpha}{h_\alpha h_\gamma} \frac{\partial h_\gamma}{\partial \alpha} \\ & + \frac{1}{h_\beta} \frac{\partial U_\beta}{\partial \beta} + \frac{U_\beta}{h_\beta h_\alpha} \frac{\partial h_\alpha}{\partial \beta} + \frac{U_\beta}{h_\beta h_\gamma} \frac{\partial h_\gamma}{\partial \beta} \\ & + \frac{1}{h_\gamma} \frac{\partial U_\gamma}{\partial \gamma} + \frac{U_\gamma}{h_\gamma h_\alpha} \frac{\partial h_\alpha}{\partial \gamma} + \frac{U_\gamma}{h_\gamma h_\beta} \frac{\partial h_\beta}{\partial \gamma} \end{aligned} \quad (1)$$

If we multiply the top and bottom of each term above by the proper combination of h 's such that the denominator becomes $h_\alpha h_\beta h_\gamma$, we get,

$$\Delta = \frac{1}{h_\alpha h_\beta h_\gamma} \left[\frac{\partial (h_\beta h_\gamma U_\alpha)}{\partial \alpha} + \frac{\partial (h_\gamma h_\alpha U_\beta)}{\partial \beta} + \frac{\partial (h_\alpha h_\beta U_\gamma)}{\partial \gamma} \right] \quad (2)$$

We have written the gradient in curvilinear coordinates as

$$\nabla = \frac{1}{h_\alpha} \frac{\partial}{\partial \alpha} \mathbf{e}_\alpha + \frac{1}{h_\beta} \frac{\partial}{\partial \beta} \mathbf{e}_\beta + \frac{1}{h_\gamma} \frac{\partial}{\partial \gamma} \mathbf{e}_\gamma \quad (3)$$

and

$$\nabla^2 \phi = \nabla \cdot \nabla \phi \text{ by definition.}$$

Using the above definition and relation (3) we may write

$$\nabla^2 \phi = \frac{1}{h_\alpha h_\beta h_\gamma} \left[\frac{\partial}{\partial \alpha} \left(\frac{h_\beta h_\gamma}{h_\alpha} \frac{\partial \phi}{\partial \alpha} \right) + \frac{\partial}{\partial \beta} \left(\frac{h_\gamma h_\alpha}{h_\beta} \frac{\partial \phi}{\partial \beta} \right) + \frac{\partial}{\partial \gamma} \left(\frac{h_\alpha h_\beta}{h_\gamma} \frac{\partial \phi}{\partial \gamma} \right) \right] \quad (4)$$

We write the stress equations of equilibrium as

$$\nabla \cdot \boldsymbol{\tau} + \mathbf{f} = \rho \ddot{\mathbf{U}} \quad (5)$$

We now write expressions for the divergence of a vector and a dyadic in orthogonal curvilinear coordinates as

$$\nabla \cdot \mathbf{U} = \nabla \cdot (U_\alpha \mathbf{e}_\alpha + U_\beta \mathbf{e}_\beta + U_\gamma \mathbf{e}_\gamma) \quad (6)$$

and

$$\nabla \cdot \tau = \nabla \cdot \left[\begin{aligned} &\sigma_{\alpha\alpha} e_\alpha e_\alpha + \sigma_{\alpha\beta} e_\alpha e_\beta + \sigma_{\alpha\gamma} e_\alpha e_\gamma \\ &+ \sigma_{\beta\alpha} e_\beta e_\alpha + \sigma_{\beta\beta} e_\beta e_\beta + \sigma_{\beta\gamma} e_\beta e_\gamma \\ &+ \sigma_{\gamma\alpha} e_\gamma e_\alpha + \sigma_{\gamma\beta} e_\gamma e_\beta + \sigma_{\gamma\gamma} e_\gamma e_\gamma \end{aligned} \right] \quad (7)$$

$$= \nabla \cdot \left[\begin{aligned} &e_\alpha (\sigma_{\alpha\alpha} e_\alpha + \sigma_{\alpha\beta} e_\beta + \sigma_{\alpha\gamma} e_\gamma) \\ &+ e_\beta (\sigma_{\beta\alpha} e_\alpha + \sigma_{\beta\beta} e_\beta + \sigma_{\beta\gamma} e_\gamma) \\ &+ e_\gamma (\sigma_{\gamma\alpha} e_\alpha + \sigma_{\gamma\beta} e_\beta + \sigma_{\gamma\gamma} e_\gamma) \end{aligned} \right] \quad (8)$$

We use the definition of ∇ as given in (3) and apply $\nabla \cdot$ to the expression (8) in [] to give us

$$\nabla \cdot \tau = \frac{1}{h_\alpha h_\beta h_\gamma} \left[\begin{aligned} &\frac{\partial}{\partial \alpha} \{ h_\beta h_\gamma (\sigma_{\alpha\alpha} e_\alpha + \sigma_{\alpha\beta} e_\beta + \sigma_{\alpha\gamma} e_\gamma) \} \\ &+ \frac{\partial}{\partial \beta} \{ h_\gamma h_\alpha (\sigma_{\beta\alpha} e_\alpha + \sigma_{\beta\beta} e_\beta + \sigma_{\beta\gamma} e_\gamma) \} \\ &+ \frac{\partial}{\partial \gamma} \{ h_\alpha h_\beta (\sigma_{\gamma\alpha} e_\alpha + \sigma_{\gamma\beta} e_\beta + \sigma_{\gamma\gamma} e_\gamma) \} \end{aligned} \right] \quad (9)$$

We expand (9) and write it as a sum of three vectors in the α, β , and γ directions. Several terms are broken into components. For components in the α direction of two typical terms we use the relations

$$e_\alpha \cdot \frac{\partial e_\beta}{\partial \alpha} = h_\alpha e_\alpha \cdot \frac{\partial e_\beta}{\partial s_\alpha} = h_\alpha \left[\frac{\partial (\ln h_\alpha)}{\partial s_\beta} \right] = \frac{1}{h_\beta} \frac{\partial h_\alpha}{\partial \beta} \quad (10)$$

and

$$e_\alpha \cdot \frac{\partial e_\beta}{\partial \beta} = -\frac{h_\beta}{h_\alpha} \frac{\partial (\ln h_\beta)}{\partial \alpha} = -\frac{1}{h_\alpha} \frac{\partial h_\beta}{\partial \alpha} \quad (11)$$

The expression (9) reduces to

$$\begin{aligned} \nabla \cdot \tau = \frac{1}{h_\alpha h_\beta h_\gamma} \left\{ \begin{aligned} &e_\alpha \left[\frac{\partial (h_\beta h_\gamma \sigma_{\alpha\alpha})}{\partial \alpha} + \frac{\partial}{\partial \beta} (h_\gamma h_\alpha \sigma_{\beta\alpha}) + \frac{\partial}{\partial \gamma} (h_\alpha h_\beta \sigma_{\gamma\alpha}) \right. \\ &\quad \left. + h_\gamma \sigma_{\alpha\beta} \frac{\partial h_\alpha}{\partial \beta} + h_\beta \sigma_{\alpha\gamma} \frac{\partial h_\alpha}{\partial \gamma} - h_\gamma \sigma_{\beta\gamma} \frac{\partial h_\alpha}{\partial \alpha} - h_\beta \sigma_{\gamma\alpha} \frac{\partial h_\alpha}{\partial \alpha} \right] \\ &+ e_\beta \left[\frac{\partial (h_\gamma h_\alpha \sigma_{\beta\beta})}{\partial \beta} + \frac{\partial}{\partial \gamma} (h_\alpha h_\beta \sigma_{\gamma\beta}) + \frac{\partial}{\partial \alpha} (h_\beta h_\gamma \sigma_{\alpha\beta}) \right. \\ &\quad \left. + h_\alpha \sigma_{\beta\gamma} \frac{\partial h_\beta}{\partial \gamma} + h_\gamma \sigma_{\beta\alpha} \frac{\partial h_\beta}{\partial \alpha} - h_\alpha \sigma_{\gamma\alpha} \frac{\partial h_\beta}{\partial \beta} - h_\gamma \sigma_{\alpha\alpha} \frac{\partial h_\beta}{\partial \beta} \right] \\ &+ e_\gamma \left[\frac{\partial (h_\alpha h_\beta \sigma_{\gamma\gamma})}{\partial \gamma} + \frac{\partial}{\partial \alpha} (h_\beta h_\gamma \sigma_{\alpha\gamma}) + \frac{\partial}{\partial \beta} (h_\gamma h_\alpha \sigma_{\beta\gamma}) \right. \\ &\quad \left. + h_\beta \sigma_{\gamma\alpha} \frac{\partial h_\gamma}{\partial \alpha} + h_\alpha \sigma_{\gamma\beta} \frac{\partial h_\gamma}{\partial \beta} - h_\beta \sigma_{\alpha\alpha} \frac{\partial h_\gamma}{\partial \gamma} - h_\alpha \sigma_{\beta\beta} \frac{\partial h_\gamma}{\partial \gamma} \right] \end{aligned} \right\} \quad (1) \end{aligned}$$

$$\nabla \cdot \sigma = \nabla \cdot \left[\sigma_{\alpha\alpha} e_\alpha e_\alpha + \sigma_{\alpha\beta} e_\alpha e_\beta + \dots \right]$$

since order α, β matters we cannot say that $\sigma_{\alpha\beta} = \sigma_{\beta\alpha}$

We will get notes on this part later.

In cylindrical coords.

$$h_r = 1, \quad h_\theta = r, \quad h_z = 1$$

$$\text{Grad} \quad \nabla = e_r \frac{\partial}{\partial r} + e_\theta \frac{1}{r} \frac{\partial}{\partial \theta} + e_z \frac{\partial}{\partial z} \quad ; \quad \text{if } H = H_r e_r + H_\theta e_\theta + H_z e_z$$

$$\text{Div}(H) = \nabla \cdot H = \frac{1}{r} \frac{\partial(r H_r)}{\partial r} + \frac{1}{r} \frac{\partial H_\theta}{\partial \theta} + \frac{\partial H_z}{\partial z}$$

$$\nabla^2 \phi = \frac{\partial^2 \phi}{\partial r^2} + \frac{1}{r} \frac{\partial \phi}{\partial r} + \frac{1}{r^2} \frac{\partial^2 \phi}{\partial \theta^2} + \frac{\partial^2 \phi}{\partial z^2} = \frac{1}{r} \frac{\partial}{\partial r} \left(r \frac{\partial \phi}{\partial r} \right) + \frac{1}{r^2} \frac{\partial^2 \phi}{\partial \theta^2} + \frac{\partial^2 \phi}{\partial z^2}$$

Equil

$$\nabla \cdot \sigma + f = \rho \frac{\partial^2 u_i}{\partial t^2}$$

This is independent of coordinate system

in curvilinear coord.

$$\frac{\partial \sigma_{rr}}{\partial r} + \frac{1}{r} \frac{\partial \sigma_{\theta r}}{\partial \theta} + \frac{\partial \sigma_{zr}}{\partial z} + \frac{\sigma_{rr} - \sigma_{\theta\theta}}{r} + f_r = \rho \frac{\partial^2 u_r}{\partial t^2}$$

$$\frac{\partial \sigma_{r\theta}}{\partial r} + \frac{2}{r} \sigma_{r\theta} + \frac{1}{r} \frac{\partial \sigma_{\theta\theta}}{\partial \theta} + \frac{\partial \sigma_{z\theta}}{\partial z} + f_\theta = \rho \frac{\partial^2 u_\theta}{\partial t^2}$$

$$\frac{\partial \sigma_{rz}}{\partial r} + \frac{1}{r} \frac{\partial \sigma_{\theta z}}{\partial \theta} + \frac{\partial \sigma_{zz}}{\partial z} + \frac{\sigma_{rz}}{r} + f_z = \rho \frac{\partial^2 u_z}{\partial t^2}$$

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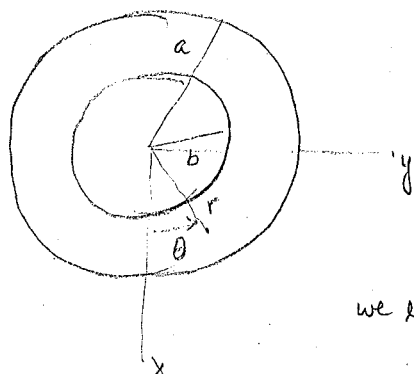
Cylindrical coordinates Continued

Components of strain

$$\epsilon_{rr} = \frac{\partial u_r}{\partial r} \quad \epsilon_{\theta\theta} = \frac{1}{r} \frac{\partial u_\theta}{\partial \theta} + \frac{u_r}{r} \quad \epsilon_{zz} = \frac{\partial u_z}{\partial z}$$

$$\epsilon_{\theta z} = \frac{1}{2} \left(\frac{1}{r} \frac{\partial u_z}{\partial \theta} + \frac{\partial u_\theta}{\partial z} \right) \quad \epsilon_{zr} = \frac{1}{2} \left(\frac{\partial u_z}{\partial r} + \frac{\partial u_r}{\partial z} \right) \quad \epsilon_{r\theta} = \frac{1}{2} \left(\frac{\partial u_\theta}{\partial r} - \frac{u_\theta}{r} + \frac{1}{r} \frac{\partial u_r}{\partial \theta} \right)$$

Torsion of a Hollow circular cylinder



Compatibility demands that ϕ satisfies

$$\nabla^2 \phi = -2\mu\alpha$$

$$\frac{1}{r} \frac{\partial}{\partial r} \left(r \frac{\partial \phi}{\partial r} \right) + \frac{1}{r^2} \frac{\partial^2 \phi}{\partial \theta^2} = -2\mu\alpha$$

we expect θ symmetry $\therefore \nabla^2 \phi = \frac{1}{r} \frac{\partial}{\partial r} \left(r \frac{\partial \phi}{\partial r} \right) = -2\mu\alpha$

$$\therefore \frac{d}{dr} \left(r \frac{d\phi}{dr} \right) = -2\mu\alpha r \quad \text{or} \quad r \frac{d\phi}{dr} = -\mu\alpha r^2 + C_1$$

since $\phi \neq \phi(\theta, r)$ but only $\phi(r)$

$$\text{now } \left[\phi = -\mu\alpha \frac{r^2}{2} + C_1 \ln r + C_2 \right]$$

for a multiply connected region

$$\oint_{C_i} \frac{d\phi}{dn} ds = 2\mu\alpha A_i$$



$$\left. \frac{d\phi}{dn} \right|_{C_i} = -\frac{d\phi}{dr} \quad \text{and} \quad ds = -b d\theta$$

$$\oint_0^{-2\pi} \left[-\mu\alpha r + \frac{C_1}{r} \right] (-b d\theta) = 2\mu\alpha A_i - 2\pi C_1 = 2\mu\alpha \pi b^2 \Rightarrow C_1 = 0$$

$$\text{now } \phi = -\frac{1}{2} \mu\alpha r^2 + C_2$$

$$\text{on } r=a \quad k_0 = -\frac{1}{2} \mu\alpha a^2 + C_2 \quad \& \quad r=b \quad k_1 = -\frac{1}{2} \mu\alpha b^2 + C_2$$

$$\text{now } k_0 - k_1 = -\frac{1}{2} \mu\alpha (a^2 - b^2) \quad \text{or} \quad k_0 = -\frac{1}{2} \mu\alpha (a^2 - b^2) + k_1$$

$$\text{hence } \left[C_2 = k_0 + \frac{1}{2} \mu\alpha a^2 = k_1 + \frac{1}{2} \mu\alpha b^2 \right]$$

$$\text{hence } \left[\phi = \frac{1}{2} \mu\alpha (a^2 - r^2) + k_0 \quad \text{or} \quad = \frac{1}{2} \mu\alpha (b^2 - r^2) + k_1 \right]$$

Now looking at the stresses to get the Torque.

$$\tau_{zr} = \frac{1}{r} \frac{\partial \phi}{\partial \theta} \rightarrow \tau_{zr} = 0$$

$$\tau_{z\theta} = -\frac{\partial \phi}{\partial r} = \mu \alpha r$$

$$T = 2 \left[\iint_{\text{net}} \phi dA - k_0 A_0 + k_1 A_1 \right]$$

$$\frac{T}{2} = \int_0^{2\pi} \int_b^a \left[\frac{1}{2} \mu \alpha (a^2 - r^2) + k_0 \right] r dr d\theta - \underbrace{k_0 \pi a^2 + k_1 \pi b^2}_{k_0 (\pi a^2 - \pi b^2) + \dots} = (k_1 - k_0) \pi b^2 = \frac{\pi b^2}{2} \mu \alpha (a^2 - b^2)$$

$$T = \frac{\pi}{2} \mu \alpha (a^4 - b^4)$$

$$\text{Alternately } T = \iint \tau_{z\theta} r dA = \int_0^{2\pi} \int_a^b \mu \alpha r^2 (r dr d\theta) = \mu \alpha \cdot 2\pi \int_a^b r^3 dr = T = \mu \alpha J$$

$$T_{\max} = \mu \alpha a$$

$J = \text{polar moment of inertia}$

Now looking at displacements we obtain

$$\left. \begin{aligned} u_x &= -\alpha y z \\ u_y &= \alpha x z \end{aligned} \right\} \rightarrow \begin{aligned} u_r &= 0 \\ u_\theta &= \alpha r z \end{aligned}$$

$$\text{without derivation we have } \frac{\partial u_z}{\partial s_\alpha} = \frac{1}{\mu} \frac{\partial \phi}{\partial s_\beta} + \frac{\alpha}{2} \frac{\partial r^2}{\partial s_\beta}$$

$$\text{let } s_\alpha = r \text{ let } s_\beta = r\theta$$

$$\frac{\partial u_z}{\partial s_\beta} = \frac{1}{\mu} \frac{\partial \phi}{\partial s_\alpha} - \frac{\alpha}{2} \frac{\partial r^2}{\partial s_\alpha}$$

$$\therefore \frac{\partial u_z}{\partial r} = \frac{1}{\mu} \frac{1}{r} \frac{\partial \phi}{\partial \theta} + \frac{\alpha}{2} \frac{1}{r} \frac{\partial r^2}{\partial \theta} = 0 \text{ because } \phi \neq \phi(\theta) \text{ and } r \neq r(\theta)$$

$$\therefore u_z = u_z(\theta)$$

$$\frac{\partial u_z}{r \partial \theta} = \frac{1}{\mu} \frac{\partial \phi}{\partial r} - \frac{\alpha}{2} \frac{\partial r^2}{\partial r} = \alpha r - \alpha r = 0$$

$\therefore u_z \neq u_z(\theta)$ hence $u_z = \text{const}$ we can take $u_z = 0$
since its only a rigid body displacement

B

Question	Score
1	20/20
2	17/20
3	20
4	20
5	16
6	10
7	
8	
9	
10	
Total	103/120 = 86%

STANFORD UNIVERSITY
OFFICIAL EXAMINATION BOOK

24 PAGE RULED

Name of Student CÉSAR LEVY

Date of Examination 11 Dec 78

Subject ME238A

HONORABLE CONDUCT
in academic work is the spirit of conduct in this University.

In recognition of and in the spirit of the Honor Code, I certify that I will neither receive nor give unpermitted aid on this examination and that I will report, to the best of my ability, all Honor Code violations observed by me.

(signed)

Cesar Levy
Name

SUGGESTIONS FOR CONDUCT

1. Occupy alternate seats where possible.
2. When in doubt as to the meaning of a question, consult the instructor, who will be found in his or her office.

Turn in at Prof. Hermann's office

DIVISION OF APPLIED MECHANICS
DEPARTMENT OF MECHANICAL ENGINEERING

ME 238A Theory of Elasticity

Autumn 1978

Final Examination

1. At a point the components of stress are:

OK $\sigma_{zx} = \sigma_{yz} = 0 ; \sigma_{xx} = 2\text{Pa} ; \sigma_{yy} = 4\text{Pa} ; \sigma_{zz} = -2\text{Pa} ; \sigma_{xy} = 2\sqrt{2}\text{Pa}$

- a) Determine the principal components of stress.
b) Determine the unit vector in the direction of the largest principal components of stress.
2. Consider the following stress field in a simply-connected medium with no body forces present:

$$\sigma_{xx} = a(x^2 - y^2) ; \sigma_{yy} = bx^2 + cy^2 ; \sigma_{zz} = d(x^2 + y^2) ;$$

$$\sigma_{xy} = exy ; \sigma_{xz} = \sigma_{yz} = 0 \quad \text{use Equil + Beltrami-Michell}$$

Determine conditions upon the constants a, b, c, d , and e so that this stress field represents an acceptable elasticity solution.

3. The following state of strain exists in a simply-connected body:

$$\epsilon_{xx} = \epsilon_{yy} = \epsilon_{zz} = T(x, y, z) ; \epsilon_{xy} = \epsilon_{yz} = \epsilon_{xz} = 0 \quad \text{use compat}$$

Show that if this strain field is to correspond to a single-valued displacement field, $T(x, y, z)$ is at most a linear function of x, y and z .

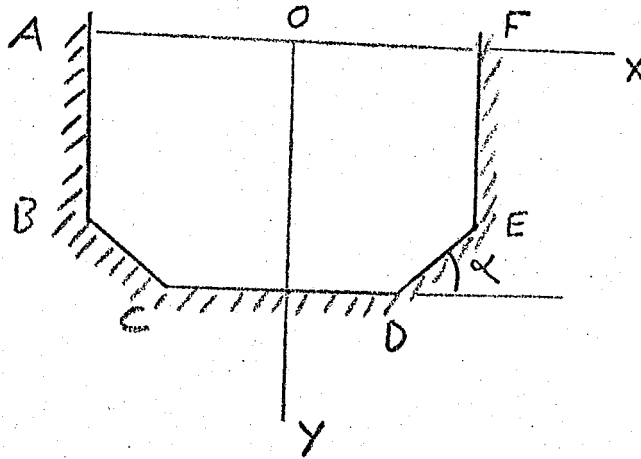
4. Consider a bar with constant cross-section A , length L , mass density ρ , Young's modulus E , and Poisson's ratio ν . It is hung in the vertical position and the upper end is appropriately fixed, while the lower end is free. The bar is subjected to a weight W hung a distance a from the upper end, as well as gravity. Neglect local effects near the point where the weight W is attached. Solve this elasticity problem as follows:

- a) Assume a state of stress.
b) Verify the satisfaction of equilibrium equations.
c) Verify the satisfaction of the boundary conditions.
d) Verify the satisfaction of the compatibility equations.
e) Calculate the components of strain.
f) Find the components of displacement by integrating the components of strain.

5. An elastic rod (Young's modulus E , Poisson's ratio ν) of length L and constant cross-sectional area A is placed in a tightly fitting, absolutely rigid tube, and it is compressed by an evenly distributed force F on the ends.

ME 238A Theory of Elasticity

- a) Determine the shortening ΔL of the rod.
 - b) Assuming $0 \leq \nu \leq \frac{1}{2}$, what is the range of ΔL ?
 - c) Determine the lengthening of the rod if the force F is tensile.
6. An elastic body is placed in a rigid container with lubricated walls A, B, C, D, E, F and is subjected to some body forces (not specified in detail). Write down the boundary conditions along the portion DE in terms of components of displacement referred to the coordinate system Oxy shown. Consider quantities in the xy-plane only.



70/20

1. Given

$$\sigma = \begin{pmatrix} 2 & 2\sqrt{2} & 0 \\ 2\sqrt{2} & 4 & 0 \\ 0 & 0 & -2 \end{pmatrix}$$

a. Find the principal stresses

To find principal stresses form $\sigma - \lambda \mathbf{I}$ and set $\det(\sigma - \lambda \mathbf{I}) = 0$. This gives λ_i 's the EIGENVALUES.

Thus

$$\det \begin{pmatrix} 2-\lambda & 2\sqrt{2} & 0 \\ 2\sqrt{2} & 4-\lambda & 0 \\ 0 & 0 & -2-\lambda \end{pmatrix} = -(2-\lambda)(4-\lambda)(2+\lambda) + 8(2+\lambda) = 0$$

$$= (2+\lambda)[- (8-6\lambda+\lambda^2) + 8] = (2+\lambda)[\lambda(6-\lambda)] = 0$$

hence $\lambda_1 = 6 \text{ Pa}$ $\lambda_2 = 0 \text{ Pa}$ $\lambda_3 = -2 \text{ Pa}$

Hence the principal stress tensor is

$$\sigma_p = \begin{pmatrix} 6 & 0 & 0 \\ 0 & 0 & 0 \\ 0 & 0 & -2 \end{pmatrix}$$

b Determine the unit vector in the direction of the largest principal stress

To do this let $\lambda = 6$ and substitute into $(\sigma - \lambda \mathbf{I}) \underline{v}^{(1)} = 0$ and $\underline{n}^{(1)} = \frac{\underline{v}^{(1)}}{\|\underline{v}^{(1)}\|}$

Therefore

$$\begin{pmatrix} -4 & 2\sqrt{2} & 0 \\ 2\sqrt{2} & -2 & 0 \\ 0 & 0 & -8 \end{pmatrix} \begin{pmatrix} v_x \\ v_y \\ v_z \end{pmatrix} = \underline{0}$$

This can be reduced to

$$\begin{pmatrix} \sqrt{2} & -1 & 0 \\ 0 & 0 & 0 \\ 0 & 0 & -8 \end{pmatrix} \begin{pmatrix} v_x \\ v_y \\ v_z \end{pmatrix} = \underline{0} \text{ by linear algebra}$$

This shows that $v_z = 0$ and $\sqrt{2} v_x = v_y$ hence we assume v_x to be the variable hence let it = 1 $\therefore v_y = \sqrt{2}$

thus $\underline{v}^{(1)} = \begin{pmatrix} 1 \\ \sqrt{2} \\ 0 \end{pmatrix}$, now $\|\underline{v}^{(1)}\| = \sqrt{1 + (\sqrt{2})^2} = \sqrt{3}$

let's take $\|\underline{v}\| = \sqrt{3}$ hence

$$m^{(1)} = \frac{1}{\sqrt{3}} \begin{pmatrix} 1 \\ \sqrt{2} \\ 0 \end{pmatrix} \quad \checkmark$$

17/20

2. Given:

$$\sigma = \begin{pmatrix} a(x^2-y^2) & e xy & 0 \\ e xy & b x^2 + c y^2 & 0 \\ 0 & 0 & d(x^2+y^2) \end{pmatrix} \text{ in a simply connected region with no body forces}$$

find conditions on a, b, c, d and e so that this stress field is an acceptable solution.

use the stress compatibility equations of Beltrami (simply connected, no body forces and I will assume no accelerations)

$$\nabla_i^2 \sigma_{ij} + \frac{1}{1+\nu} \sigma_{kk,ij} = 0$$

$$\sigma_{kk} = (a+b+d)x^2 + (c+d-a)y^2 \quad \text{hence}$$

$$\nabla_i^2 \sigma_{xx} + \frac{1}{1+\nu} \sigma_{kk,xx} = 2a - 2a + \frac{1}{1+\nu} 2(a+b+d) = 0 \Rightarrow a+b+d=0 \quad (1)$$

$$\nabla_i^2 \sigma_{xy} + \frac{1}{1+\nu} \sigma_{kk,xy} = 0 + \frac{1}{1+\nu} \cdot 0 = 0 \quad (2)$$

$$\nabla_i^2 \sigma_{xz} + \frac{1}{1+\nu} \sigma_{kk,xz} = 0 + \frac{1}{1+\nu} \cdot 0 = 0 \quad (3)$$

$$\nabla_i^2 \sigma_{yy} + \frac{1}{1+\nu} \sigma_{kk,yy} = 2b + 2c + \frac{1}{1+\nu} 2(c+d-a) = 0 \quad (4)$$

$$\nabla_i^2 \sigma_{yz} + \frac{1}{1+\nu} \sigma_{kk,yz} = 0 + \frac{1}{1+\nu} \cdot 0 = 0 \quad (5)$$

$$\nabla_i^2 \sigma_{zz} + \frac{1}{1+\nu} \sigma_{kk,zz} = 2d + 2d + \frac{1}{1+\nu} \cdot 0 = 4d = 0 \quad (6)$$

in doing the above we assume $\frac{1}{1+\nu} \neq 0$

from (6) $d=0$ and from (1) $a=-b$. from (4) $(2b+2c)(1+\frac{1}{1+\nu})=0$;

hence $b=-c$ or $\underline{c=a}$

not arbitrary see

$$\therefore \boxed{a, e \text{ are arbitrary } c=a=-b \text{ and } d=0}$$

hence 1 solution is to take $a=1$ $b=-1$ $c=1$ $d=0$ $e=0$

you need to check equilibrium eqs. -3

$$\sigma_{xx,x} + \sigma_{yx,y} + \sigma_{zx,z} = 2ax + ex + 0 = 0 \quad \therefore 2a+e=0 \quad \forall x,y,z$$

$$\sigma_{xy,x} + \sigma_{yy,y} + \sigma_{zy,z} = ey + 2ay = 0 \quad \therefore e+2c=0 \quad \forall y,x,z$$

$$\sigma_{xz,x} + \sigma_{yz,y} + \sigma_{zz,z} = 0 + 0 + 0 = 0 \quad \therefore e=-2a \text{ and } e=-2c$$

20

3. Given

$\phi = \begin{pmatrix} T & 0 & 0 \\ 0 & T & 0 \\ 0 & 0 & T \end{pmatrix}$ where $T = T(x, y, z)$. If this is to correspond to a single valued displacement field, T is a linear function of x, y, z in a simply connected body.

Proof

ϕ must satisfy the compatibility condition $\nabla \times \phi \times \nabla = 0$

hence

$$\psi_{xx} = 2\epsilon_{yz,yz} - \epsilon_{yy,zz} - \epsilon_{zz,yy} = 0 - T_{,yy} - T_{,zz} = 0 \quad \text{or} \quad \frac{\partial^2 T}{\partial y^2} + \frac{\partial^2 T}{\partial z^2} = 0 \quad (1)$$

$$\psi_{yy} = 2\epsilon_{xz,xz} - \epsilon_{xx,zz} - \epsilon_{zz,xx} = 0 - T_{,zz} - T_{,xx} = 0 \quad \text{or} \quad \frac{\partial^2 T}{\partial z^2} + \frac{\partial^2 T}{\partial x^2} = 0 \quad (2)$$

$$\psi_{zz} = 2\epsilon_{xy,xy} - \epsilon_{xx,yy} - \epsilon_{yy,xx} = 0 - T_{,yy} - T_{,xx} = 0 \quad \text{or} \quad \frac{\partial^2 T}{\partial y^2} + \frac{\partial^2 T}{\partial x^2} = 0 \quad (3)$$

$$\psi_{yz} = \epsilon_{xx,yz} - \frac{\partial}{\partial x} (-\epsilon_{yz,x} + \epsilon_{zx,y} + \epsilon_{xy,z}) = \epsilon_{xx,yz} = T_{,yz} \quad \text{or} \quad \frac{\partial^2 T}{\partial y \partial z} = 0 \quad (4)$$

$$\psi_{zx} = \epsilon_{yy,zx} - \frac{\partial}{\partial y} (-\epsilon_{zx,y} + \epsilon_{xy,z} + \epsilon_{yz,x}) = \epsilon_{yy,zx} = T_{,zx} \quad \text{or} \quad \frac{\partial^2 T}{\partial z \partial x} = 0 \quad (5)$$

$$\psi_{xy} = \epsilon_{zz,xy} - \frac{\partial}{\partial z} (-\epsilon_{xy,z} + \epsilon_{yz,x} + \epsilon_{zx,y}) = \epsilon_{zz,xy} = T_{,xy} \quad \text{or} \quad \frac{\partial^2 T}{\partial x \partial y} = 0 \quad (6)$$

From (1) we get that $\frac{\partial^2 T}{\partial y^2} = -\frac{\partial^2 T}{\partial z^2}$ and from (2) that $-\frac{\partial^2 T}{\partial z^2} = \frac{\partial^2 T}{\partial x^2} \Rightarrow \frac{\partial^2 T}{\partial y^2} = \frac{\partial^2 T}{\partial x^2}$
combining the above result and (3) gives that

$$\frac{\partial^2 T}{\partial x^2} = \frac{\partial^2 T}{\partial y^2} = 0 \quad \text{and} \quad \Rightarrow \quad \frac{\partial^2 T}{\partial z^2} = 0 \quad (7a, b, c)$$

$$\text{From } \frac{\partial^2 T}{\partial x^2} = 0 \Rightarrow \frac{\partial T}{\partial x} = f(y, z) \quad \text{and} \quad T = xf(y, z) + g(y, z) \quad (8)$$

Now put (8) into $\frac{\partial^2 T}{\partial y^2} = 0$ gives that $x \frac{\partial^2 f}{\partial y^2} + \frac{\partial^2 g}{\partial y^2} = 0$. Since this must be

$$\therefore \frac{\partial^2 f}{\partial y^2} = 0 \quad \text{and} \quad \frac{\partial^2 g}{\partial y^2} = 0$$

$\therefore$ from $\frac{\partial^2 f(y,z)}{\partial y^2} = 0$ we get $f(y,z) = y h(z) + l(z)$ (9)

from $\frac{\partial^2 g(y,z)}{\partial y^2} = 0$ we get $g(y,z) = y p(z) + q(z)$ (10)

Now using $\frac{\partial^2 T}{\partial z^2} = 0$ gives that $xy h''(z) + x l''(z) + y p''(z) + q''(z) = 0$. Since

this must be true $\forall x$ and $y \Rightarrow h''(z) = 0, l''(z) = 0, p''(z) = 0, q''(z) = 0$
this leads to

$h(z) = a_1 z + b_1$; $l(z) = a_2 z + b_2$; $p(z) = a_3 z + b_3$; $q(z) = a_4 z + b_4$ (11a-d)

Summary: so far

$T(x,y,z) = xy h(z) + x l(z) + y p(z) + q(z)$ (12)

Now applying (4) to (12) gives that $\frac{\partial^2 T}{\partial y \partial z} = x a_1 + a_3 = 0$. Since this must be true $\forall x \Rightarrow a_1 = 0, a_3 = 0$ (13a,b)

Now applying (5) to (12) incorporating (13a,b) gives that

$\frac{\partial^2 T}{\partial z^2 \partial x} = a_2 = 0$ (14)

Now applying (6) to (12) incorporating (13a,b) and (14) gives that

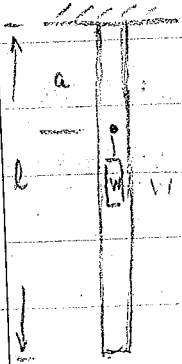
$\frac{\partial^2 T}{\partial x \partial y} = b_1 = 0$ (15)

hence we obtain the final result that

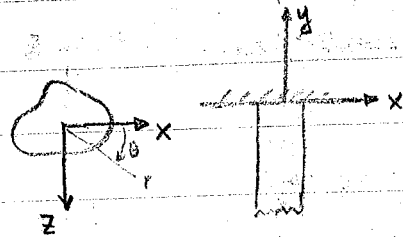
$T(x,y,z) = b_2 x + b_3 y + a_4 z + b_4$ which is a linear fn of x, y, z

QED! ✓

4. Consider a bar with cross section A , length L , mass density ρ , Young's Modulus E , and Poisson's ratio ν .

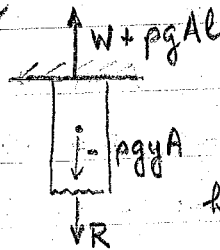


We will take the coordinate axis and we look at two cases, stresses above the weight and below the weight



From the free body diagram we get that the reaction at the built in end is $W + pgAl$. Hence for the section

above the weight

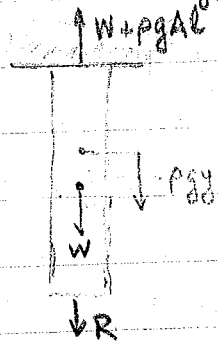


(Note: $f_y = -pg$, $f_x = 0$, $f_z = 0$)

hence $R = W + pgAl + pgA(l+y) = W + pgA(l+y)$

and $\sigma_{yy} = \frac{W}{A} + pg(l+y)$ $-a \leq y \leq 0$

also the section below the weight



Note: $f_y = -pg$, $f_x = 0$, $f_z = 0$

hence $R = W + pgAl - W + pgA(l+y) = pgA(l+y)$

and $\sigma_{yy} = pg(l+y)$ $l \leq y \leq -a$

- a) In defining the deformation caused by this

$$\sigma_{yy} = \frac{W}{A} + pg(l+y) \quad -a \leq y \leq 0$$

$$\sigma_{yy} = pg(l+y) \quad -l \leq y \leq -a$$

we are assuming that each cross section is in uniform tension due to reaction force R and this leads to

$$\sigma_{zz} = \sigma_{xx} = \sigma_{xy} = \sigma_{yz} = \sigma_{zx} = 0 \quad \forall x, y, z \text{ in the body}$$

Thus we notice that $\sigma_{y\max}$ is at $y=0$ and $\sigma_{y\min}$ is at $y=-l$.

b) To see if it satisfies the equilib equations

$$\sigma_{xx,x} + \sigma_{yx,y} + \sigma_{zx,z} + f_x \equiv 0 \quad \text{identically}$$

$$\sigma_{xy,x} + \sigma_{yy,y} + \sigma_{zy,z} + f_y = \rho g - \rho g = 0$$

the weight produces a constant stress which causes a stress jump to occur at $y = -a$. since $\sigma_{yy,y} = \rho g$ for both sections

$$\sigma_{xz,x} + \sigma_{yz,y} + \sigma_{zz,z} + f_z \equiv 0$$

Hence we satisfy equilib

why do you complicate?

c) To satisfy the boundary conditions

- $n = \cos \theta e_x + \sin \theta e_z$ on the surface. Hence $n \cdot \sigma = \cos \theta (\sigma_{xx} e_x + \sigma_{xy} e_y + \sigma_{xz} e_z) + \sin \theta (\sigma_{zx} e_x + \sigma_{zy} e_y + \sigma_{zz} e_z) = 0$ since $\sigma_{zx}, \sigma_{xy}, \sigma_{xx}, \sigma_{zy}, \sigma_{zz} = 0$. Thus we satisfy the boundary condition of no tractions on the surface.

- $n = -e_y$ on the bottom. Hence $n \cdot \sigma = -\sigma_{yx} e_x - \sigma_{yy} e_y - \sigma_{yz} e_z = 0$. Thus we satisfy the no traction b.c. on the end. Thus we satisfy the bc

d) To satisfy compatibility the only equation that will be affected from the Beltrami-Michell equations is

$$\nabla^2 \sigma_{yy} + \frac{1}{1+\nu} \Theta_{,yy} = 0$$

; The others are satisfied identically. We note that since σ_{yy} is linear in y the $\nabla^2(\text{linear fn}) = 0$ and $\Theta_{,yy} = 0$; hence we satisfy this equation also. We note that since $f_x = 0$, $f_y = -\rho g = \text{const}$ and $f_z = 0$, the differentials of the body forces that would appear on the right side of all the Beltrami-Michell equations give 0 right hand sides.

Hence we satisfy the compatibility equations

e) Now

$$\epsilon_{ij} = \frac{1+\nu}{E} \sigma_{ij} - \frac{\nu}{E} \sigma_{kk} \delta_{ij} \Rightarrow \epsilon_{xx} = -\frac{\nu}{E} \sigma_{yy} = \epsilon_{zz} \quad \text{and} \quad \epsilon_{yy} = \sigma_{yy}/E \quad \text{only}$$

$$\text{for } -a \leq y \leq 0 \quad \epsilon_{xx} = \epsilon_{zz} = -\frac{\nu}{E} \left[\frac{W}{A} + \rho g(l+y) \right] \quad \epsilon_{yy} = \frac{W}{AE} + \frac{\rho g}{E}(l+y)$$

$$\text{for } -l \leq y < -a \quad \epsilon_{xx} = \epsilon_{zz} = -\frac{\nu}{E} \rho g(l+y) \quad \text{and} \quad \epsilon_{yy} = \frac{\rho g}{E}(l+y)$$

f) Find the components of displacement

for $-a \leq y \leq 0$

$$\epsilon_{xx} = \frac{\partial u_x}{\partial x} = -\frac{\nu}{E} \left[\frac{W}{A} + \rho g(l+y) \right] \Rightarrow u_x = -\frac{\nu x}{E} \left[\frac{W}{A} + \rho g(l+y) \right] + \tilde{f}_1(y, z)$$

$$\epsilon_{yy} = \frac{\partial u_y}{\partial y} = \frac{1}{E} \left[\frac{W}{A} + \rho g(l+y) \right] \Rightarrow u_y = \frac{y}{E} \left[\frac{W}{A} + \rho g(l+y) \right] + \tilde{f}_2(x, z)$$

$$\epsilon_{zz} = \frac{\partial u_z}{\partial z} = -\frac{\nu}{E} \left[\frac{W}{A} + \rho g(l+y) \right] \Rightarrow u_z = -\frac{\nu z}{E} \left[\frac{W}{A} + \rho g(l+y) \right] + \tilde{f}_3(x, y)$$

We must use the fact that $\epsilon_{xy}, \epsilon_{xz}, \epsilon_{yz} = 0$ to get $\tilde{f}_1, \tilde{f}_2, \tilde{f}_3$

for $-l \leq y \leq -a$

$$\epsilon_{xx} = -\frac{\nu}{E} \rho g(l+y) = \frac{\partial u_x}{\partial x} \Rightarrow u_x = -\frac{\nu x}{E} \rho g(l+y) + \tilde{f}_4(y, z)$$

$$\epsilon_{yy} = \frac{\rho g}{E} (l+y) = \frac{\partial u_y}{\partial y} \Rightarrow u_y = \frac{\rho g}{E} \left(ly + \frac{y^2}{2} \right) + \tilde{f}_5(x, z)$$

$$\epsilon_{zz} = -\frac{\nu}{E} \rho g(l+y) = \frac{\partial u_z}{\partial z} \Rightarrow u_z = -\frac{\nu z}{E} \rho g(l+y) + \tilde{f}_6(x, y)$$

we must use that fact that $\epsilon_{xy}, \epsilon_{xz}, \epsilon_{yz} = 0$ to get $\tilde{f}_4, \tilde{f}_5, \tilde{f}_6$ AND the fact that the displacements are continuous at the pt $y = -a$ $\forall x, z$ to relate $\tilde{f}_1, \tilde{f}_2, \tilde{f}_3$ to $\tilde{f}_4, \tilde{f}_5, \tilde{f}_6$.

with conditions @ $(0, 0, 0)$ that $u_x, u_y, u_z = 0$ and no rotation about the y axis we get that

$$\tilde{f}_1(y, z) = \tilde{f}_2(x, z) = \tilde{f}_3(x, y) = 0$$

hence for $-a \leq y \leq 0$

$$u_x = -\frac{\nu}{E} \left[\frac{W}{A} + \rho g(l+y) \right] x \quad \checkmark$$

$$u_y = \rho g \frac{W}{EA} y + \frac{\rho g}{E} \left[\left(ly + \frac{y^2}{2} \right) + \frac{\nu}{2} (x^2 + z^2) \right]$$

$$u_z = -\frac{\nu}{E} \left[\frac{W}{A} + \rho g(l+y) \right] z \quad \checkmark$$

before the application of the above boundary conditions

$$\tilde{f}_1(y, z) = c_{13}z + c_{14} - y c_9$$

$$\tilde{f}_2(x, z) = \frac{\nu \rho g}{2E} (x^2 + z^2) + c_9 x + c_{11} z + c_{16}$$

$$\tilde{f}_3(x, y) = -c_{13}x + c_{15} - y c_{11}$$

for $-l \leq y \leq -a$

$$\tilde{f}_4(y, z) = c_5 z + c_6 - c_1 y$$

$$\tilde{f}_5(x, z) = \frac{\nu p g}{2E} (x^2 + z^2) + c_1 x + c_3 z + c_8$$

$$\tilde{f}_6(x, y) = -c_5 x + c_7 - y c_3$$

if we assume continuous displacements at $y = -a$ then

$$-\frac{\nu x W}{E A} = c_5 z + c_6 + c_1 a$$

$$-\frac{a W}{E A} = \frac{\nu p g}{2E} (x^2 + z^2) + c_1 x + c_3 z + c_8$$

$$-\frac{\nu z W}{E A} = -c_5 x + c_7 + a c_3$$

if $x=0, z=0$ $-c_1 a = c_6$ or $c_1 = -\frac{c_6}{a}$

$$c_8 = -\frac{a W}{E A}$$

$$c_7 = -a c_3 \text{ or } c_3 = -\frac{c_7}{a}$$

hence $\tilde{f}_4(y, z) = c_5 z + \frac{c_6}{a} (a + y)$

$$\tilde{f}_5(x, z) = \frac{\nu p g}{2E} (x^2 + z^2) - \frac{1}{a} (c_6 x + c_7 z) - \frac{a W}{E A}$$

$$\tilde{f}_6(x, y) = -c_5 x + \frac{c_7}{a} (a + y)$$

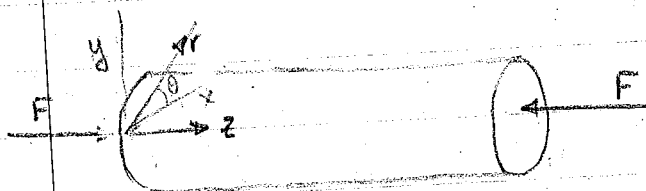
thus we satisfy the displacement field at $(0, -a, 0)$. If we want to define the displacement throughout at $(x, -a, 0)$ we have to define a relationship between x and w . If not, we get a discontinuity of displacement at $y = -a$.

enclosed in the sheets are the scratch papers that give the step by step evaluation of $\tilde{f}_1, \tilde{f}_2, \tilde{f}_3, \tilde{f}_4, \tilde{f}_5, \tilde{f}_6$

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5. An elastic rod of length L and cross-section A is placed in a tight fitting, absolutely rigid tube, and it is compressed by an evenly distributed load F on the ends

- Determine the shortening ΔL of the rod
- Assuming $0 \leq \nu \leq \frac{1}{2}$, what is the range of ΔL ?
- Determine the lengthening of the rod if the force F is tensile.



The boundary conditions on the rod are that u_r at $r=r_0=0$. The problem is symmetric with respect to θ $\therefore$ all derivatives (written in polar coordinates) with respect to $\theta = 0$.

We will use superposition

- We will assume problem # 1 has only σ_{zz} acting on it and check the values of $\epsilon_{rr}, \epsilon_{\theta\theta}$. We will then look at problem # 2 whose loads are such to cause the cancellation of strains caused by problem # 1

Problem #1 $\therefore \epsilon_{zz} = \frac{\sigma_{zz}}{E} \Rightarrow \epsilon_{rr} = -\frac{\nu \sigma_{zz}}{E} \quad \epsilon_{\theta\theta} = -\frac{\nu \sigma_{zz}}{E} \quad \sigma_{r\theta} = \sigma_{rz} = \sigma_{\theta z} = 0$
 $\sigma_{\theta\theta}, \sigma_{rr} = 0$

now in problem 2 we define $\sigma_{rr}, \sigma_{\theta\theta} \Rightarrow$

$$\left. \begin{aligned} \epsilon_{rr} = \frac{\nu \sigma_{zz}}{E} &= \frac{\sigma_{rr}}{E} - \frac{\nu \sigma_{\theta\theta}}{E} \\ \epsilon_{\theta\theta} = \frac{\nu \sigma_{zz}}{E} &= \frac{\sigma_{\theta\theta}}{E} - \frac{\nu \sigma_{rr}}{E} \end{aligned} \right\} \begin{aligned} \sigma_{rr} &= \sigma_{zz} \frac{(1+\nu)\nu}{1-\nu^2} \\ \sigma_{\theta\theta} &= \sigma_{zz} \frac{(1+\nu)\nu}{1-\nu^2} \end{aligned}$$

hence $\epsilon_{zz} = \frac{\sigma_{zz}}{E} - \frac{\nu}{E} (\sigma_{rr} + \sigma_{\theta\theta}) = \frac{(1-2\nu)(1+\nu)}{1-\nu} \frac{\sigma_{zz}}{E}$

but $\epsilon_{zz} = \frac{\Delta L}{L} \quad \sigma_{zz} = -\frac{F}{A}$

$\Delta L = -\frac{FL}{AE} \frac{(1-2\nu)(1+\nu)}{1-\nu}$

hence for $\nu = 0 \quad \Delta L = -\frac{FL}{AE}$ and $\nu = +\frac{1}{2} \quad \Delta L = 0 \quad \therefore -\frac{FL}{AE} \leq \Delta L \leq 0$

This solution allows for no strains in the r and θ directions. Note that this solution for $\epsilon_{rr}, \epsilon_{\theta\theta}, \epsilon_{zz}$ satisfies compatibility, boundary conditions and equilibrium in the part c

if F is tensile then $\Delta L = \frac{FL}{AE} \frac{(1-2\nu)(1+\nu)}{1-\nu}$ since the development was for any general σ_{zz} regardless of sign $\times$ For tensile F
 $\epsilon_{rr}, \epsilon_{\theta\theta} \neq 0$

If the body were allowed to have σ_θ stresses then $\epsilon_{zz} = \frac{(1-\nu^2)}{E} \sigma_{zz}$, $\epsilon_{rr} = 0$
 and $\epsilon_{\theta\theta} = -\frac{\nu}{E} (\sigma_{zz} + \nu \sigma_{zz}) = -\frac{\nu(1+\nu)}{E} \sigma_{zz}$ and for $\sigma_{zz} = -\frac{F}{A}$
 and $\Delta L = \epsilon_{zz} \cdot L = -\frac{(1-\nu^2)FL}{AE}$

$$\epsilon_{zz} = \frac{\sigma_{zz}}{E} \quad \epsilon_{rr} = -\frac{\nu \sigma_{zz}}{E} \quad \text{and} \quad \epsilon_{\theta\theta} = -\frac{\nu \sigma_{zz}}{E}$$

$$\therefore \frac{\Delta L}{L} = \frac{F}{AE} \quad \text{or} \quad \Delta L = \frac{FL}{AE}$$

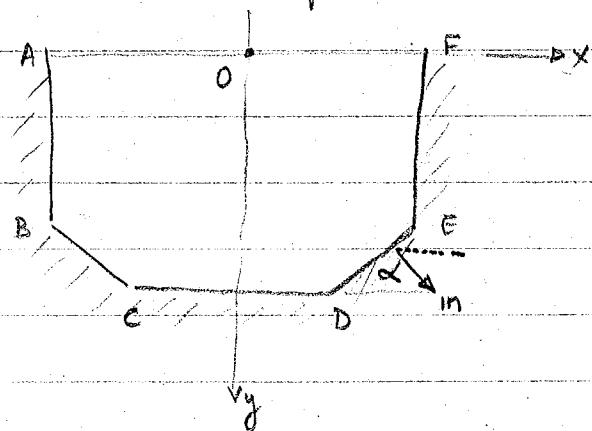
~~$$\epsilon_{zz} = \frac{\sigma_{zz}}{E} \frac{\nu}{E} (\sigma_{rr} + \sigma_{\theta\theta})$$~~

~~$$\frac{\Delta L}{L} = \frac{\sigma_{zz}}{E} (1-\nu^2) = \frac{F}{AE} (1-\nu^2)$$~~

~~$$\therefore \Delta L = \frac{FL(1-\nu^2)}{AE}$$~~

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6. An elastic body is placed in a rigid container with lubricated walls and is subjected to some body forces. Write down the body conditions along DE in terms of the components of displacements



This will be considered a plane strain problem since each cross section will be same as others $\therefore \epsilon_{zz} = 0 \Rightarrow \sigma_{zz} = \nu(\sigma_{xx} + \sigma_{yy})$
 $\therefore u_z = \text{function of } (x, y) \text{ only}$

B.C. displacement in the n direction must be zero

i.e. $u \cdot n = 0$ or $u_x \sin \alpha + u_y \cos \alpha = 0 \quad \forall x, y, z$ (except possibly the end points)

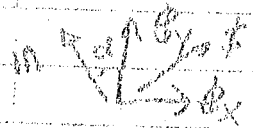
other B.C. $\sigma_{nn} = 0$

I hope this is what you mean. σ_{nn} is expressible in terms

of displacement

⑥

6) a) on DE, rigid boundary $\Rightarrow M \cdot n = 0$



$$n = \sin \alpha \mathbf{e}_y + \cos \alpha \mathbf{e}_x$$

$$M = M_x \mathbf{e}_x + M_y \mathbf{e}_y \quad M \cdot n = \sin \alpha M_y + \cos \alpha M_x = 0 \text{ on DE}$$

b) on DE no shear component of traction (since walls are lubed)
 $M \cdot s = M \cdot s \cdot s = 0 \Rightarrow \cos \alpha = s'_{xy}$ (with $s = \mathbf{e}_y$ & $n = \mathbf{e}_x$)

$$\begin{bmatrix} \mathbf{e}_x \\ \mathbf{e}_y \\ \mathbf{e}_z \end{bmatrix} \begin{bmatrix} \cos \alpha & 0 \\ \sin \alpha & 0 \\ 0 & 0 & 1 \end{bmatrix} \begin{bmatrix} \sigma_x \\ \sigma_y \\ \sigma_z \end{bmatrix}$$

$$s'_{xy} = 0 = \mu E \epsilon'_{xy} \Rightarrow \epsilon'_{xy} = 0$$

$$\epsilon'_{xy} = \frac{1}{2} (\sigma'_{xy} + \sigma'_{yx}) = -\sin \alpha \cos \alpha \epsilon_{xx} + \cos^2 \alpha \epsilon_{yy}$$

$$-\sin^2 \alpha \epsilon_{yy} + \sin \alpha \cos \alpha \epsilon_{xx} = 0$$

$$-\sin \alpha \cos \alpha \left(\frac{\partial u}{\partial x} \right) + \frac{1}{2} (\cos^2 \alpha - \sin^2 \alpha) \left(\frac{\partial u}{\partial y} + \frac{\partial v}{\partial x} \right) - \sin \alpha \cos \alpha \left(\frac{\partial v}{\partial y} \right) = 0$$

mult. through by 2 $\Rightarrow \left[\cos 2\alpha \left(\frac{\partial u}{\partial y} + \frac{\partial v}{\partial x} \right) - \sin 2\alpha \left(\frac{\partial u}{\partial x} + \frac{\partial v}{\partial y} \right) \right] = 0$

$$n \cdot \nabla \cdot s = 0$$

$$n = \sin \alpha \mathbf{j} + \cos \alpha \mathbf{i} \quad \cos \alpha \mathbf{j} + \sin \alpha \mathbf{i}$$

$$s = \cos \alpha \mathbf{i} + \sin \alpha \mathbf{j}$$

$$L = \begin{bmatrix} \cos \theta & \sin \theta & 0 \\ -\sin \theta & \cos \theta & 0 \\ 0 & 0 & 1 \end{bmatrix} \quad \sigma = \begin{bmatrix} \sigma_{xx} & \sigma_{xy} & \sigma_{xz} \\ \sigma_{yx} & \sigma_{yy} & \sigma_{yz} \\ \sigma_{zx} & \sigma_{zy} & \sigma_{zz} \end{bmatrix} \quad L^T = \begin{bmatrix} \cos \theta & -\sin \theta & 0 \\ \sin \theta & \cos \theta & 0 \\ 0 & 0 & 1 \end{bmatrix}$$

$$\begin{bmatrix} c & s & 0 \\ -s & c & 0 \\ 0 & 0 & 1 \end{bmatrix} \begin{bmatrix} \sigma_{xx}c + \sigma_{xy}s & -\sigma_{xx}s + \sigma_{xy}c & \cancel{\sigma_{xz}} \\ \cancel{\sigma_{yx}c} + \sigma_{yy}s & -\sigma_{yx}s + \sigma_{yy}c & \cancel{\sigma_{yz}} \\ \cancel{\sigma_{zx}c} + \cancel{\sigma_{zy}s} & -\cancel{\sigma_{zx}s} + \cancel{\sigma_{zy}c} & \sigma_{zz} \end{bmatrix}$$

$$\begin{bmatrix} \sigma_{xx}c^2 + \sigma_{xy}cs + \cancel{\sigma_{yx}cs} + \sigma_{yy}s^2 & -\sigma_{xx}sc + \sigma_{xy}c^2 - \sigma_{yx}s^2 + \sigma_{yy}cs^2 & 0 \\ -\sigma_{xx}sc - \sigma_{xy}s^2 + \cancel{\sigma_{yx}c^2} + \sigma_{yy}cs^2 & \sigma_{xx}s^2 - \sigma_{xy}sc - \sigma_{yx}cs + \sigma_{yy}c^2 & \\ \cancel{\sigma_{zx}c} + \cancel{\sigma_{zy}s} & -\cancel{\sigma_{zx}s} + \cancel{\sigma_{zy}c} & \sigma_{zz} \end{bmatrix}$$

$$\begin{aligned} \sigma_{xx}' &= \sigma_{xx}c^2 + \sigma_{yy}s^2 + 2\sigma_{xy}cs \\ \sigma_{xy}' &= (\sigma_{yy} - \sigma_{xx})sc + \sigma_{xy}(c^2 - s^2) \\ \sigma_{yy}' &= \sigma_{xx}s^2 + \sigma_{yy}c^2 - 2\sigma_{xy}sc \end{aligned}$$

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OFFICIAL EXAMINATION BOOK

24 PAGE RULED

Question	Score
1	
2	
3	
4	
5	
6	
7	
8	
9	
10	
Total	83/100

Name of Student

Cesar Levy

Date of Examination

6 Nov 78

Subject

Elasticity M238A

HONORABLE CONDUCT

in academic work is the spirit of conduct in this University.

In recognition of and in the spirit of the Honor Code, I certify that I will neither receive nor give unpermitted aid on this examination and that I will report, to the best of my ability, all Honor Code violations observed by me.

(signed)

Cesar Levy
Name

SUGGESTIONS FOR CONDUCT

1. Occupy alternate seats where possible.
2. When in doubt as to the meaning of a question, consult the instructor, who will be found in his or her office.

DIVISION OF APPLIED MECHANICS

DEPARTMENT OF MECHANICAL ENGINEERING

238A Theory of Elasticity

Autumn 1978

Midterm Exam

1. Consider a state of stress in a body for which $\sigma_{xx} = 3A$; $\sigma_{zz} = -A$;

(15) $\sigma_{yy} = \sigma_{xy} = \sigma_{yz} = \sigma_{zx} = 0$ and where $A > 0$. Determine

[15] a) The maximum normal stress $3A$

(15) b) The shear component of traction in the x-z plane *on principal planes*
⊥ to princip dir $\tau = 0$

[20] c) The traction across a plane equally inclined to the positive x- and z-axes and containing the y-axis.

2. Consider the displacement field

(24) $u_x = By$; $u_y = u_z = 0$

[25] Determine the relative elongation (stretch) at 45° to both the x- and y-axes.

3. Consider a state of strain in a simply-connected body given by

$$\epsilon_{xx} = k_1(x^2 - y^2)$$

$$\epsilon_{yy} = k_2xy$$

$$\epsilon_{xy} = k_3xy$$

(24) All other $\epsilon_{ij} = 0$

[25] For this strain field to be associated with a single-valued displacement field, what relation(s) must be satisfied among the coefficients

k_1 , k_2 , k_3 ?

is a principal direction

is the normal stress along principal stress axis
 by substituting σ_{xx} and σ_{yy} in a normal stress equation
 we can express it directly in the x or y plane

$$\sigma_{xx} = \sigma_{xx} \cos^2 \theta + \sigma_{yy} \sin^2 \theta$$

$$\sigma_{yy} = \sigma_{xx} \sin^2 \theta + \sigma_{yy} \cos^2 \theta$$



$$\sigma_{xx} = \sigma_{xx} \cos^2 \theta + \sigma_{yy} \sin^2 \theta$$



$$\sigma_{xx} = \sigma_{xx} \cos^2 \theta + \sigma_{yy} \sin^2 \theta$$

$$\sigma_{yy} = \sigma_{xx} \sin^2 \theta + \sigma_{yy} \cos^2 \theta$$

$$\sigma_{xx} = \sigma_{xx} \cos^2 \theta + \sigma_{yy} \sin^2 \theta$$

is the normal stress

$$\sigma_{xx} = \sigma_{xx} \cos^2 \theta + \sigma_{yy} \sin^2 \theta$$

is the normal stress

$$\sigma_{xx} = \sigma_{xx} \cos^2 \theta + \sigma_{yy} \sin^2 \theta$$

$$\sigma_{yy} = \sigma_{xx} \sin^2 \theta + \sigma_{yy} \cos^2 \theta$$

is the normal stress

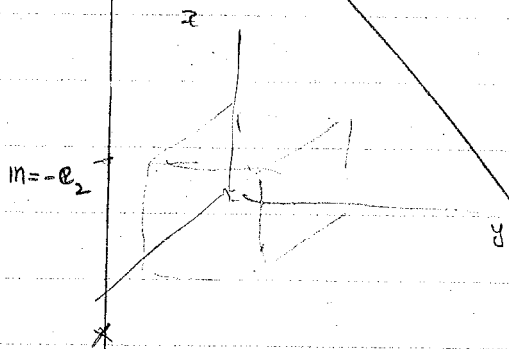
is the normal stress

is the normal stress

$$\sigma_{xx} = \sigma_{xx} \cos^2 \theta + \sigma_{yy} \sin^2 \theta$$

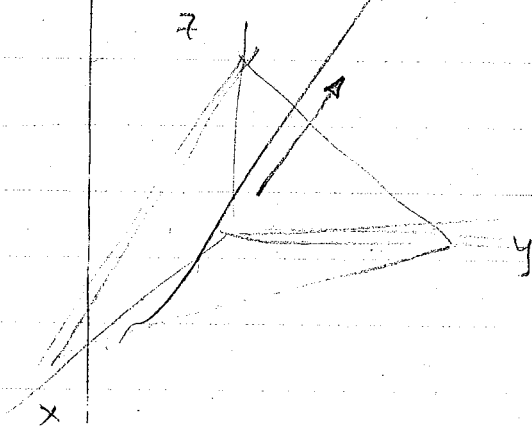
is the normal stress

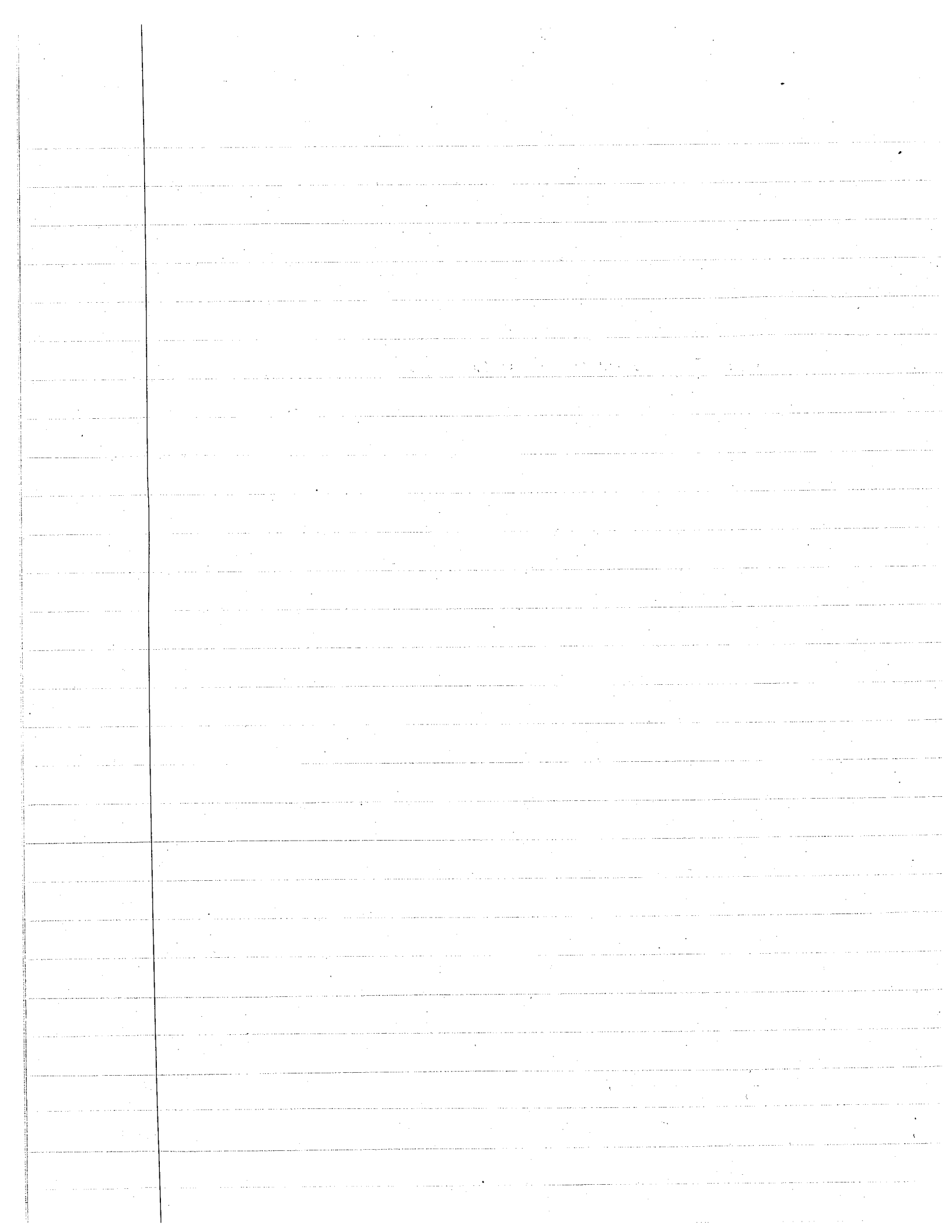
$$\sigma = \begin{pmatrix} 3A & 0 & 0 \\ 0 & 0 & 0 \\ 0 & 0 & -A \end{pmatrix}$$



The shear comp on x, z plane

$$n \cdot \sigma = -t_2 = -(\sigma_{21}e_1 + \sigma_{22}e_2 + \sigma_{23}e_3)$$





3. To solve, use compat

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$$\psi_{xx} = 2 \cdot 0 - 0 - 0 = 0 \checkmark$$

$$\psi_{yy} = 2 \cdot 0 - 0 - 0 = 0 \checkmark$$

$$\psi_{zz} = 2 \cdot K_3 + 2K_1 - 0 = 0 \text{ only if } K_1 = -K_3 \checkmark$$

$$\psi_{yz} = 0 + 0 - 0 - 0 = 0 \checkmark$$

$$\psi_{zx} = 0 + 0 - 0 - 0 = 0$$

$$\psi_{xy} = 0 - 0 + 0 - 0 = 0$$

no restriction of K_2 but $K_1 = -K_3$

$$2. \quad \Phi = \Phi_{ij} = \frac{1}{2} \left(\frac{\partial u_i}{\partial x_j} + \frac{\partial u_j}{\partial x_i} - \frac{\partial u_k}{\partial x_i} \frac{\partial u_k}{\partial x_j} \right)$$

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and $m \cdot \Phi \cdot m$

$$m = \frac{1}{\sqrt{2}} (e_1 + e_2)$$

$$\Phi_{11} = \frac{1}{2} (0 + 0 - [0]) = 0$$

$$\Phi_{12} = \frac{1}{2} (B + 0 - 0) = \frac{1}{2} B$$

$$\Phi_{13} = \frac{1}{2} (0 + 0 - B \cdot 0) = 0$$

$$\Phi_{21} = \frac{1}{2} (B - 0) = \frac{1}{2} B$$

$$\Phi_{22} = \frac{1}{2} (0 + 0 - B^2) = -\frac{B^2}{2}$$

$$\Phi_{23} = \frac{1}{2} (0 + 0 + 0) = 0$$

$$\begin{pmatrix} 0 & \frac{1}{2} B & 0 \\ \frac{1}{2} B & -\frac{B^2}{2} & 0 \\ 0 & 0 & 0 \end{pmatrix}$$

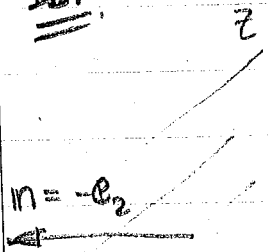
$$\Phi_{ij} = \frac{1}{2} B e_y e_x + \frac{1}{2} B e_x e_y - \frac{B^2}{2} e_y e_y$$

$$m \cdot \Phi \cdot m = \frac{1}{\sqrt{2}} (e_1 + e_2) \cdot \left[\frac{1}{2} B e_1 e_2 + \frac{1}{2} B e_2 e_1 - \frac{B^2}{2} e_2 e_2 \right] \cdot \frac{1}{\sqrt{2}} (e_1 + e_2) =$$

$$= \frac{1}{8} [B e_2 + B e_1 - B^2 e_2] \cdot (e_1 + e_2)$$

$$m \cdot \Phi \cdot m = \frac{1}{8} [B + B - B^2] = \frac{B}{8} [2 - B]$$

1b.



15

$$\therefore t_n = n \cdot \sigma = -t_2 = -[\sigma_{21}^0 e_1 + \sigma_{22}^0 e_2 + \sigma_{23}^0 e_3]$$

x

$$\therefore t_n = 0$$

$$\therefore \text{shear traction} = 0$$

since $n \cdot t_n = \text{normal component of traction} = 0$

$$\text{if } t_n = 0 = a n + b \pi \Rightarrow a = b = 0$$

1a

$$n_x^{(1)} = n_z^{(1)} \begin{vmatrix} 0 & 0 \\ 0 & -\lambda \end{vmatrix} = 0$$

$$n_y^{(2)} = n_z \begin{vmatrix} 3A - \lambda^{(1)} & 0 \\ 0 & 0 \end{vmatrix} = 0$$

$$n_z^{(3)} = n_z$$

Use invariants

$$I = \sigma_{11} + \sigma_{33} = \sigma_1 + \sigma_2 + \sigma_3 = 2A$$

$$II = \sigma_{11} \sigma_{33} = -3A^2 = \sigma_1 \sigma_2 + \sigma_2 \sigma_3 + \sigma_3 \sigma_1$$

$$III = 0 \Rightarrow \sigma_1 \sigma_2 \sigma_3 = 0 \quad \text{one of the } \sigma_i = 0 \text{ assume } \sigma_3 = 0$$

$$\therefore \sigma_1 + \sigma_2 = 2A$$

$$\sigma_1 + \sigma_2 = \sigma_1 - \frac{3A^2}{\sigma_1} = 2A$$

$$\sigma_1 \sigma_2 = -3A^2$$

$$\therefore \sigma_1^2 - 3A^2 - 2A\sigma_1 = 0$$

$$\text{or } \frac{2A \pm \sqrt{4A^2 - 4(-3A^2)}}{2} = \sigma_{1,2}$$

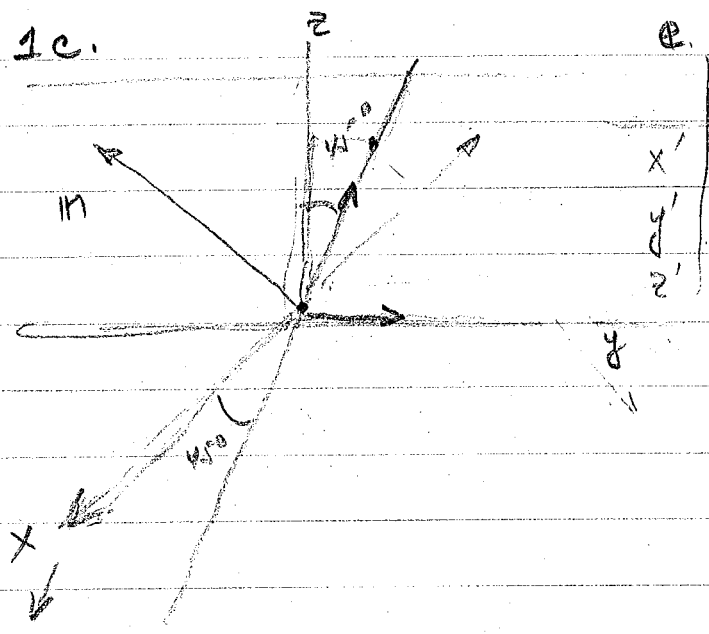
$$\sigma_{1,2} = \frac{2A \pm \sqrt{16A^2}}{2} = \frac{2A \pm 4A}{2} = \frac{6A}{2}$$

$$3A \text{ \& } -A$$

$$\therefore \text{max stress} = 3A$$

$$\text{min stress} = -A$$

1c.



$\mathcal{E}$

$$\begin{bmatrix} x' \\ y' \\ z' \end{bmatrix}$$

y

$$\begin{aligned} \mathbf{e}'_1 &= \frac{\mathbf{e}_2 - \mathbf{e}_1}{\sqrt{2}} \\ \mathbf{e}'_2 &= \mathbf{e}_y \end{aligned}$$

$$\text{in. } \mathbf{e}'_1 \times \mathbf{e}'_2 = \begin{vmatrix} -\frac{1}{\sqrt{2}} & 0 & \frac{1}{\sqrt{2}} \\ 0 & 1 & 0 \\ a & b & c \end{vmatrix} = 0 \quad \text{for non coplanarity}$$

$$c \left(-\frac{1}{\sqrt{2}} \right) - \frac{a}{\sqrt{2}} = 0$$

$\therefore$

$$c = -a \quad \therefore$$

$$\mathbf{e} \text{ in} = \frac{1}{\sqrt{2}} (\mathbf{e}_1 + \mathbf{e}_2) \quad \checkmark$$

$$\begin{aligned} \therefore \text{in. } \mathbf{t}_n &= \frac{1}{\sqrt{2}} \mathbf{t}_1 + \frac{1}{\sqrt{2}} \mathbf{t}_2 = \frac{1}{\sqrt{2}} (\mathbf{e}_1 + \mathbf{e}_2) [\mathbf{e}_i \mathbf{t}_i] \\ &= \frac{1}{\sqrt{2}} (3A \mathbf{e}_1) + \frac{1}{\sqrt{2}} 0 = \frac{3A}{\sqrt{2}} \mathbf{e}_1 \end{aligned}$$

$$\frac{-c}{\sqrt{2}} - \frac{a}{\sqrt{2}} = 0 \Rightarrow c = -a$$

$$\therefore n = a \left[\frac{1}{\sqrt{2}}, \frac{-1}{\sqrt{2}} \right]$$

$$\underbrace{n \cdot e_1 \times e_2}_{n} = 1 \quad \therefore$$

$$\begin{vmatrix} -\frac{1}{\sqrt{2}} & 0 & \frac{1}{\sqrt{2}} \\ 0 & 1 & 0 \\ a & b & c \end{vmatrix} = c \left(\frac{-1}{\sqrt{2}} \right) - \frac{a}{\sqrt{2}} = 1$$

$$\therefore (-c-a) = \sqrt{2}$$

$$\therefore (c+a) = -\sqrt{2}$$

$$\text{also } c^2 + a^2 = 1 \quad \therefore \Rightarrow c^2 + a^2 + 2ca = 2$$

$$2ca = 1$$

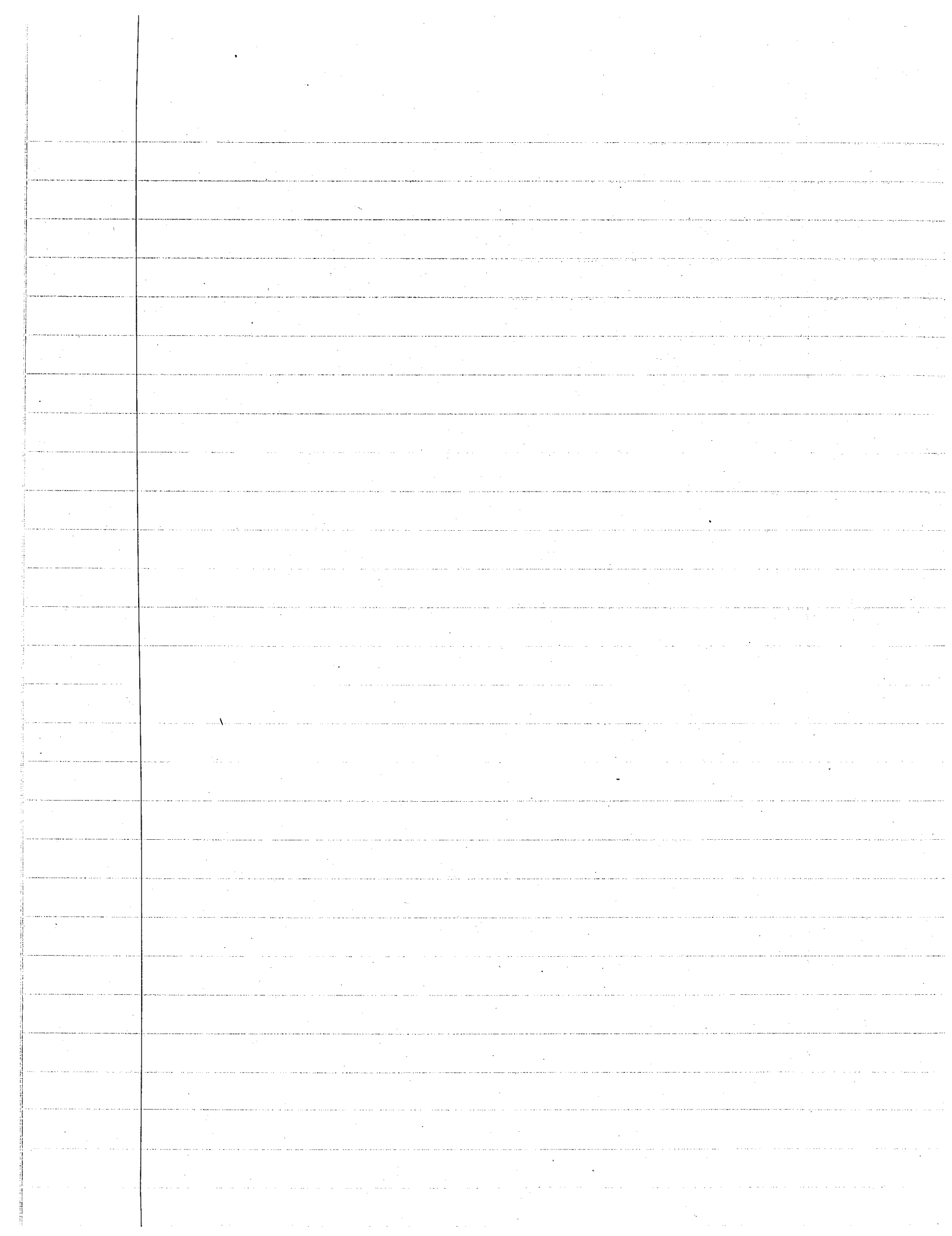
$$- n \cdot e_1' = 0 \quad a \left(-\frac{1}{\sqrt{2}} \right) + c \left(\frac{1}{\sqrt{2}} \right) = 0 \quad a = -c$$

$$- n \cdot e_2' = 0 \quad b = 0$$

$$- n \cdot n = 1 \quad a^2 + c^2 = 1 \quad \therefore 2c^2 = 1 \quad c = \pm \frac{1}{\sqrt{2}}$$

$$a = \mp \frac{1}{\sqrt{2}}$$

$$\therefore n = \mp \frac{1}{\sqrt{2}} \pm \frac{1}{\sqrt{2}}$$



DIVISION OF APPLIED MECHANICS
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238A Theory of Elasticity

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Problem Set No. 1

DEFINITIONS

1. Kronecker Delta:

$$\delta_{ij} = \begin{cases} 1 & \text{if } i=j \\ 0 & \text{if } i \neq j \end{cases}$$

2. Idemfactor (Identity Dyadic): $\mathbb{I} = \mathbf{e}_1 \mathbf{e}_1 + \mathbf{e}_2 \mathbf{e}_2 + \mathbf{e}_3 \mathbf{e}_3 = \mathbf{e}_i \mathbf{e}_i$
where $\mathbf{e}_1, \mathbf{e}_2$ and $\mathbf{e}_3$ are mutually orthogonal unit vectors.

3. Direct Product of Two Dyadics: If $\Phi = \mathbf{a} \mathbf{b}$ and $\Psi = \mathbf{c} \mathbf{d}$, then $\Phi \cdot \Psi = (\mathbf{b} \cdot \mathbf{c}) \mathbf{a} \mathbf{d}$

4. Double Dot Product of Two Dyadics: If $\Phi = \mathbf{a} \mathbf{b}$ and $\Psi = \mathbf{c} \mathbf{d}$,
then $\Phi : \Psi = (\mathbf{a} \cdot \mathbf{c})(\mathbf{b} \cdot \mathbf{d})$

VERIFY THESE EXPRESSIONS FOR YOURSELF (Don't hand in)

1. $\nabla \cdot (\nabla \phi) = \nabla^2 \phi = \phi_{,ii}$ (ϕ a scalar)

2. $\nabla \times (\nabla \phi) = 0$

3. $v_i \delta_{ij} = v_j$

4. $\mathbf{e}_i \cdot \mathbf{e}_j = \delta_{ij}$ where $\mathbf{e}_1, \mathbf{e}_2, \mathbf{e}_3$ are mutually orthogonal unit vectors.

5. $\mathbb{I} = \delta_{ij} \mathbf{e}_i \mathbf{e}_j$

6. $\mathbf{a} \times \mathbf{b} = a_i b_j \epsilon_{ijk} \mathbf{e}_k$

$\mathbf{a} = a_i \mathbf{e}_i$ $\mathbf{a} \times \mathbf{b} = a_i b_j (\mathbf{e}_i \times \mathbf{e}_j) = \epsilon_{ijk} \mathbf{e}_k a_i b_j$
 $\mathbf{b} = b_j \mathbf{e}_j$

PROBLEMS

1. Let $\mathbf{f} = f_i \mathbf{e}_i$

a. Show that $\mathbf{f} \cdot \mathbb{I} = \mathbf{f}$

b. Show that $\mathbb{I} \cdot \mathbf{f} = \mathbf{f}$

2. Let $\Psi = \psi_{ij} \mathbf{e}_i \mathbf{e}_j$

a. Show that $\Psi \cdot \mathbb{I} = \Psi$

b. Show that $\mathbb{I} \cdot \Psi = \Psi$

3. Show that $\epsilon_{ijk} a_i a_j = 0$

4. Show that $\epsilon_{ijk} \epsilon_{kji} = -6$

5. Let $\Psi = \psi_{ij} \mathbf{e}_i \mathbf{e}_j$

a. Show that $\nabla \times \Psi = \psi_{ij,k} \epsilon_{kji} \mathbf{e}_i \mathbf{e}_j$ b. Show that $\Psi \times \nabla = \psi_{i,j,k} \epsilon_{ekj} \mathbf{e}_i \mathbf{e}_j$

6. Let $\Phi = \phi_{ij} \mathbf{e}_i \mathbf{e}_j$ and $\Psi = \psi_{kl} \mathbf{e}_k \mathbf{e}_l$

a. Show that $\Phi \cdot \Psi = \phi_{ij} \psi_{ij}$ b. Show that $\Phi : \Psi = \phi_{ij} \psi_{ij}$

$$1. \quad f = f_i e_i$$

$$\therefore f \cdot e_i = e_i \cdot f = e_i (f_j e_j) = f_j (e_i \cdot e_j) = (f_j e_j) \cdot e_i = f_i$$

$$\begin{aligned} a. \quad \therefore f &= f_i e_i = f_1 e_1 + f_2 e_2 + f_3 e_3 = (f \cdot e_1) e_1 + (f \cdot e_2) e_2 + (f \cdot e_3) e_3 \\ &= f \cdot [e_1 e_1 + e_2 e_2 + e_3 e_3] = f \cdot I \end{aligned}$$

or

$$b. \quad e_i f_i = f = e_1 [e_1 \cdot f] + e_2 [e_2 \cdot f] + e_3 [e_3 \cdot f] = [e_1 e_1 + e_2 e_2 + e_3 e_3] \cdot f = I \cdot f$$

$$2.a. \quad \Psi = \Psi_{ij} e_i e_j$$

$$\Psi \cdot e_i = \Psi_{ij} e_i e_j \cdot e_i = \Psi_{ii} e_i$$

$$\begin{aligned} \Psi &= \Psi_{11} e_1 e_1 + \Psi_{12} e_1 e_2 + \Psi_{13} e_1 e_3 \\ &= [\Psi \cdot e_1] e_1 + [\Psi \cdot e_2] e_2 + [\Psi \cdot e_3] e_3 \\ &= \Psi \cdot [e_1 e_1 + e_2 e_2 + e_3 e_3] = \Psi \cdot I \end{aligned}$$

$$b. \quad \Psi = e_i \Psi_{ij} e_j$$

$$e_i \cdot \Psi = e_i \cdot e_j \Psi_{ij} e_j = \Psi_{ij} e_j$$

$$\Psi = e_1 \Psi_{1j} e_j + e_2 \Psi_{2j} e_j + e_3 \Psi_{3j} e_j$$

$$\begin{aligned} \Psi &= e_1 [e_1 \cdot \Psi] + e_2 [e_2 \cdot \Psi] + e_3 [e_3 \cdot \Psi] \\ &= [e_1 e_1 + e_2 e_2 + e_3 e_3] \cdot \Psi = I \cdot \Psi \end{aligned}$$

$$3. \quad e_{ijk} a_i a_j = 0 \quad \text{Show this is true}$$

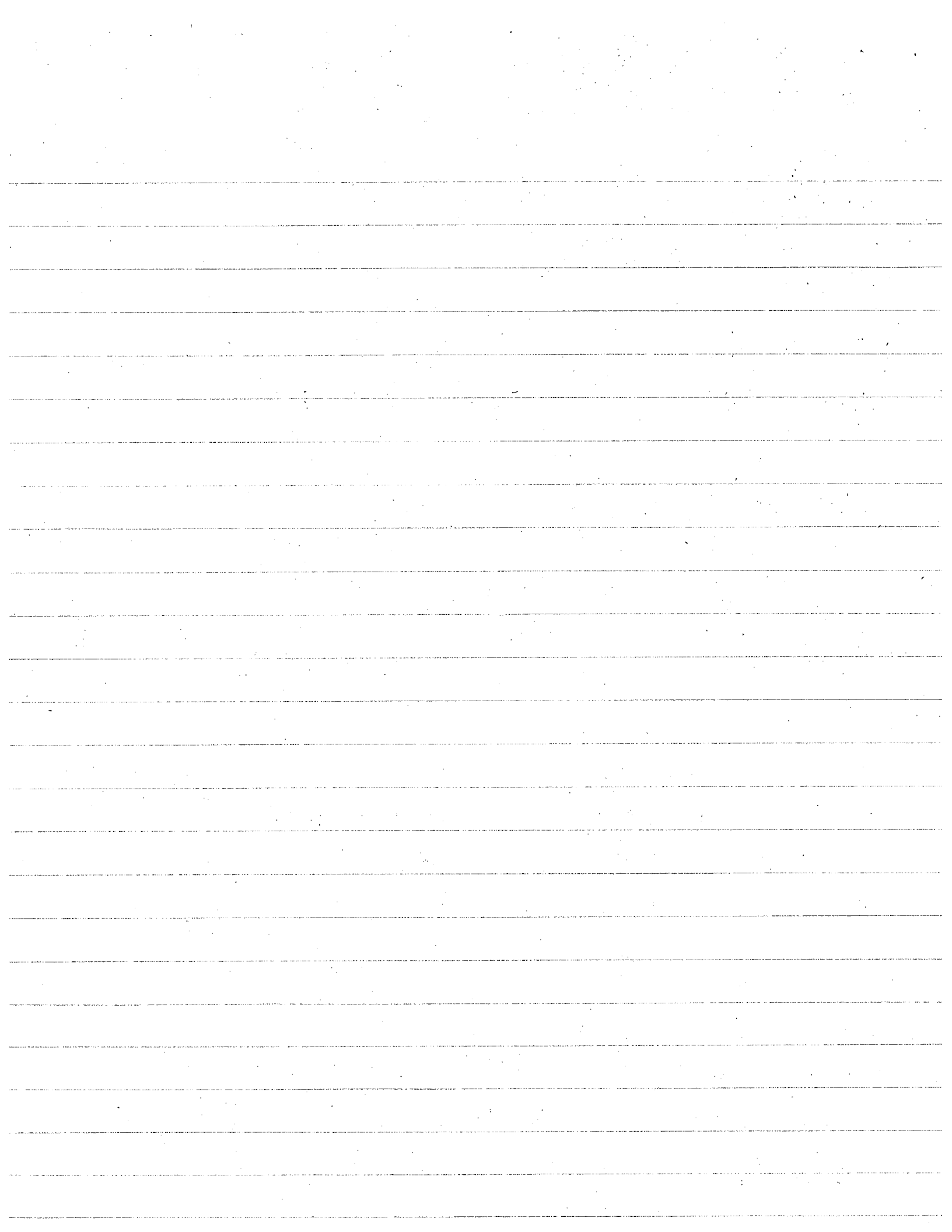
$$\text{note that } e_{ijk} = \begin{cases} +1 & \text{ijk cyclic} \\ 0 & \text{others} \\ -1 & \text{ijk anticyclic} \end{cases}$$

The only terms that will contribute are the cyclic and anti cyclic terms

ie (123, 231, 312) cyclic and (321, 132, 213) anti-cyclic

$$\text{note that } e_{123} a_1 a_2 + e_{213} a_2 a_1 = 0 \quad \text{since } e_{123} = -e_{213}$$

$$e_{231} a_2 a_3 + e_{321} a_3 a_2 = 0 \quad \text{"} \quad e_{231} = -e_{321}$$



thus $e_{ijk} a_i a_j = 0$

4. $e_{ijk} e_{kji} = -6$ show this

note that $e_{kji} = -e_{ijk}$ since its indices are anti-cyclic wrt e_{ijk} .

thus only the indicial terms of e_{ijk} equal to 123, 312, 231 and 321, 132, 213 give any contribution and this turns out to be true of e_{kji} also. Hence $e_{ijk} e_{kji} = e_{ijk} (-e_{ijk}) = -(1 \cdot 1 + 1 \cdot 1 + 1 \cdot 1) - [(-1 \cdot -1) + (-1 \cdot -1) + (-1 \cdot -1)] = -6$

5. $\nabla \times \psi$ for $\psi = \psi_{ij} e_i e_j$

$$\nabla = e_1 \frac{\partial}{\partial x_1} + e_2 \frac{\partial}{\partial x_2} + e_3 \frac{\partial}{\partial x_3}$$

$$\therefore \nabla \times \psi = \left(e_k \frac{\partial}{\partial x_k} \right) \times \psi_{ij} e_i e_j = \left[e_1 \frac{\partial}{\partial x_1} + e_2 \frac{\partial}{\partial x_2} + e_3 \frac{\partial}{\partial x_3} \right] \times (\psi_{11} e_1 e_1 + \psi_{12} e_1 e_2 + \psi_{13} e_1 e_3$$

$$+ \psi_{21} e_2 e_1 + \psi_{22} e_2 e_2 + \psi_{23} e_2 e_3 + \psi_{31} e_3 e_1 + \psi_{32} e_3 e_2 + \psi_{33} e_3 e_3)$$

$$\begin{aligned} & \psi_{21,1} e_3 e_1 + \psi_{22,1} e_3 e_2 + \psi_{23,1} e_3 e_3 - \psi_{31,1} e_2 e_1 - \psi_{32,1} e_2 e_2 - \psi_{33,1} e_2 e_3 \\ & + \psi_{31,2} e_3 e_1 + \psi_{32,2} e_3 e_2 + \psi_{33,2} e_3 e_3 - \psi_{11,2} e_3 e_1 - \psi_{12,2} e_3 e_2 - \psi_{13,2} e_3 e_3 \\ & + \psi_{11,3} e_2 e_1 + \psi_{12,3} e_2 e_2 + \psi_{13,3} e_2 e_3 - \psi_{21,3} e_1 e_1 - \psi_{22,3} e_1 e_2 - \psi_{23,3} e_1 e_3 \end{aligned}$$

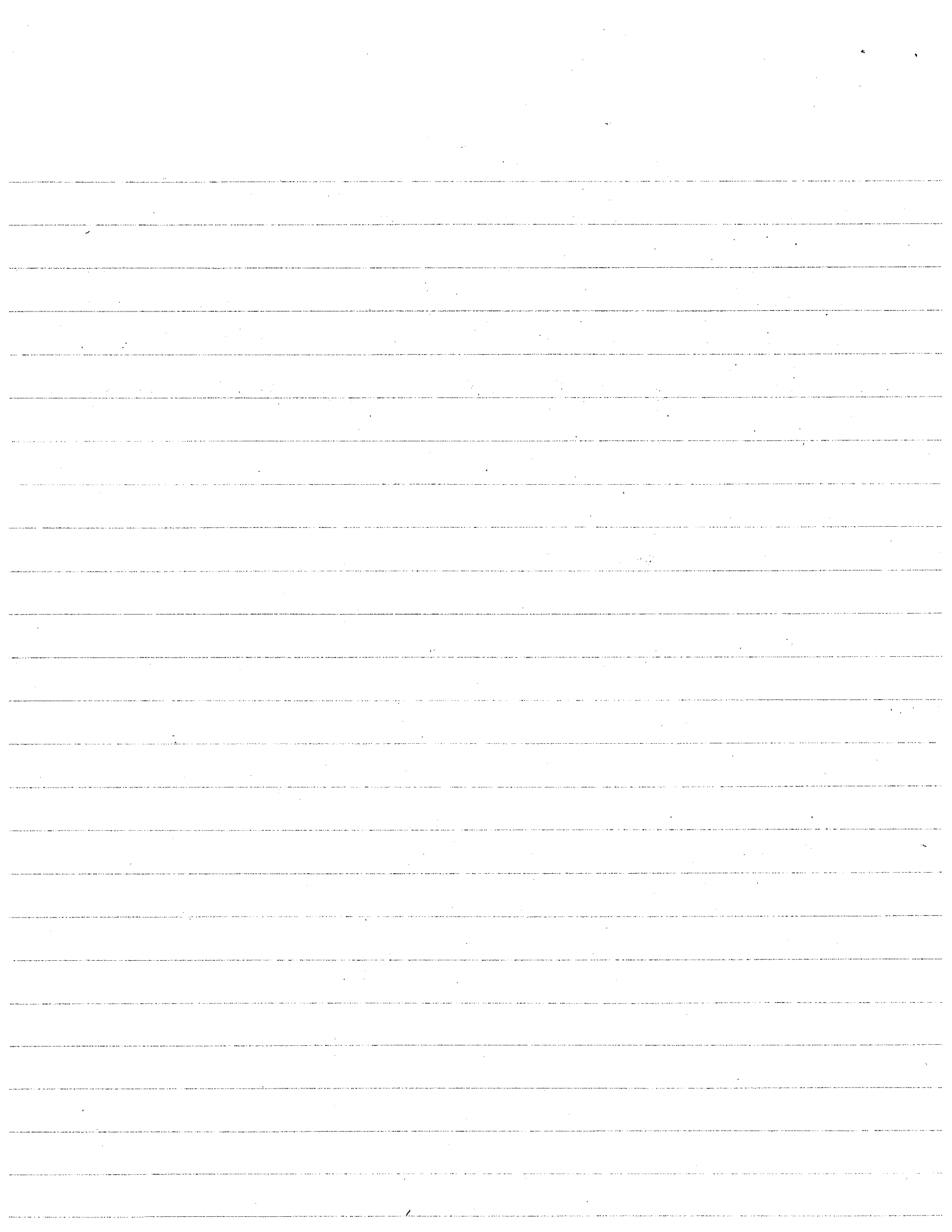
note any typical term is $-\psi_{11,2} e_3 e_1 = \psi_{11,2} (e_2 \times e_1) e_3 = \psi_{11,2} (e_3 e_3) e_1$

$$\text{or in general } e_k \frac{\partial}{\partial x_k} \times \psi_{ij} e_i e_j = \frac{\partial \psi_{ij}}{\partial x_k} (e_k \times e_i) e_j$$

since indices are dummy then $\psi_{ij,k} e_{kil} e_l e_j$ can be written as $\psi_{lj,k} e_{kli} e_i e_j$ when $i \rightarrow l$ & $l \rightarrow i$

$$\psi \times \nabla = \psi_{ij} e_i e_j \times e_k \frac{\partial}{\partial x_k} = e_i (e_j \times e_k) \frac{\partial \psi_{ij}}{\partial x_k} = e_i (e_{jkl} e_l) \frac{\partial \psi_{ij}}{\partial x_k}$$

since indices are dummy then $\psi_{ij,k} e_{jkl} e_i e_l$ can be written as $\psi_{il,k} e_{lkj} e_i e_j$



and since $e_i e_j = e_j e_i$ we get final result $\psi_{il,k} e_{ekj} e_i e_j$

6. $\Phi = \phi_{ij} e_i e_j$ and $\Psi = \psi_{kl} e_k e_l$

by definition $\Phi \cdot \Psi = \phi_{ij} e_i e_j \cdot \psi_{kl} e_k e_l$ or $e_i (\phi_{ij} \psi_{kl} e_l) e_j \cdot e_k$

$e_j \cdot e_k = 1$ when $j=k=m$ $e_j \cdot e_k = 0$ $j \neq k \therefore$

$\Phi \cdot \Psi = \phi_{im} \psi_{ml} e_i e_l$ $\therefore$ since indices are dummy replace $l \rightarrow m$

$\therefore \phi_{im} \psi_{mj} e_i e_j$

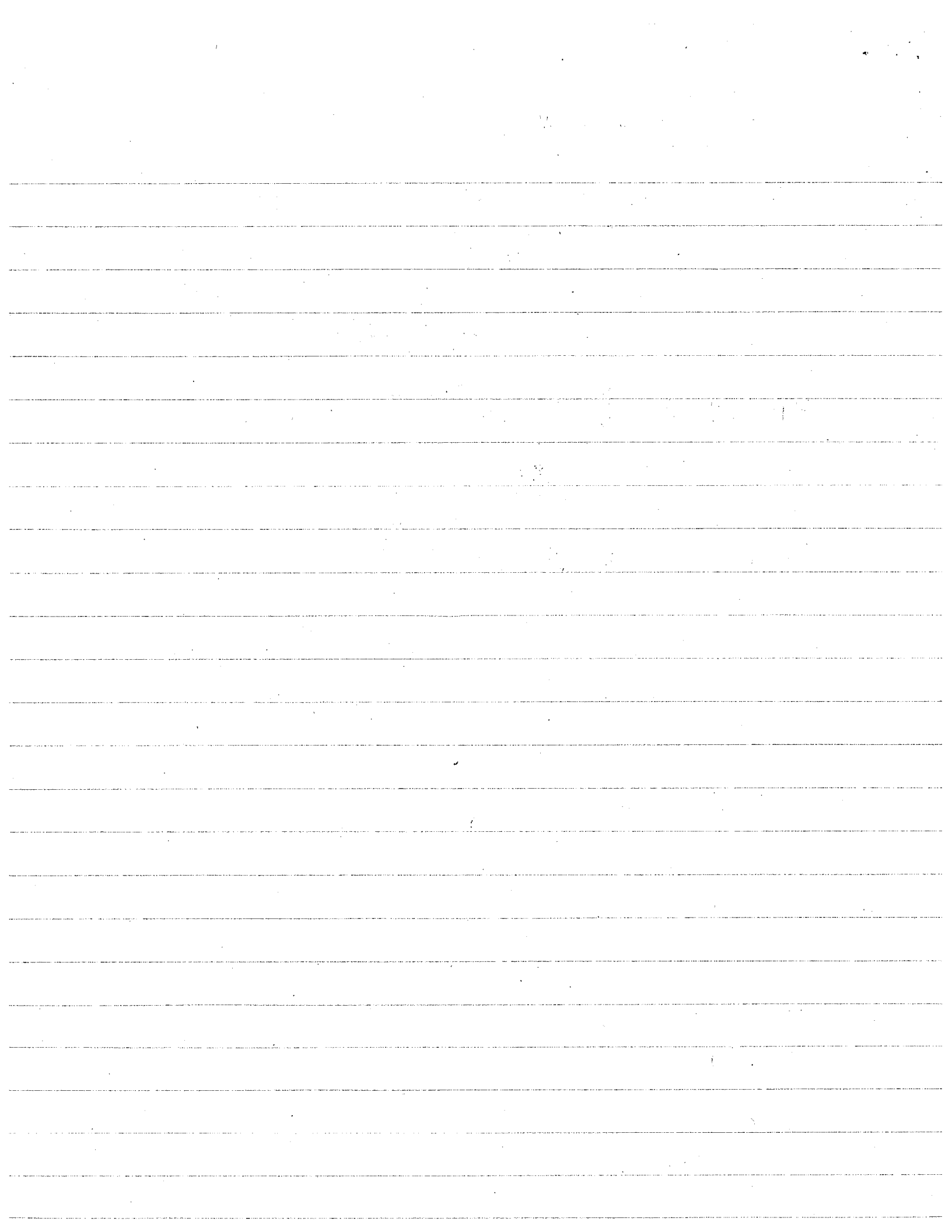
$\Phi = \phi_{ij} e_i e_j$ and $\Psi = \psi_{kl} e_k e_l$

$\Phi : \Psi = e_i \phi_{ij} e_j \cdot e_k \psi_{kl} e_l = \phi_{ij} \psi_{kl} (e_i \cdot e_k) (e_j \cdot e_l)$

$e_i \cdot e_k = 1$ when $j=k=m$ $= 0$ when $j \neq k$

and $e_j \cdot e_l = 1$ when $j=l=n$ or 0 when $j \neq l$

$\therefore \Phi : \Psi = \phi_{nm} \psi_{mn}$



Problem set #1.

100/100

1. Let $f = f_i e_i$

a. Show that $f \cdot \mathbb{I} = f$

10 Since $f_i = f \cdot e_i$ then $f = (f \cdot e_i) e_i = f \cdot e_i e_i = f \cdot \delta_{ij} e_i e_j = f \cdot \mathbb{I} \checkmark$

10 b. Show that $\mathbb{I} \cdot f = f$

Since $f_i = f \cdot e_i = e_i \cdot f$ and $f = f_i e_i = e_i f_i$ then $f = e_i (e_i \cdot f) = e_i e_i \cdot f = \mathbb{I} \cdot f \checkmark$

2. Let $\Psi = \Psi_{ij} e_i e_j$

10 a. Show that $\Psi \cdot \mathbb{I} = \Psi$

Note that $\Psi \cdot e_i = \Psi_{ij} e_j$ thus $\Psi = [\Psi \cdot e_1] e_1 + [\Psi \cdot e_2] e_2 + [\Psi \cdot e_3] e_3$

and $\Psi = \Psi \cdot [e_1 e_1 + e_2 e_2 + e_3 e_3] = \Psi \cdot \mathbb{I} \checkmark$

10 b. Show that $\Psi = \mathbb{I} \cdot \Psi$

Note that since $\Psi = \Psi_{ij} e_i e_j$ then $\Psi = e_i \Psi_{ij} e_j$ and $e_i \cdot \Psi = \Psi_{ij} e_j$

thus $\Psi = e_1 [e_1 \cdot \Psi] + e_2 [e_2 \cdot \Psi] + e_3 [e_3 \cdot \Psi] = [e_1 e_1 + e_2 e_2 + e_3 e_3] \cdot \Psi = \mathbb{I} \cdot \Psi$

3. Show that $\epsilon_{ijk} a_i a_j = 0$

10 since $\epsilon_{ijk} = \begin{cases} +1 & \text{for } i, j, k \text{ cyclic} \\ 0 & \text{for all others} \\ -1 & \text{for } i, j, k \text{ anticyclic} \end{cases}$

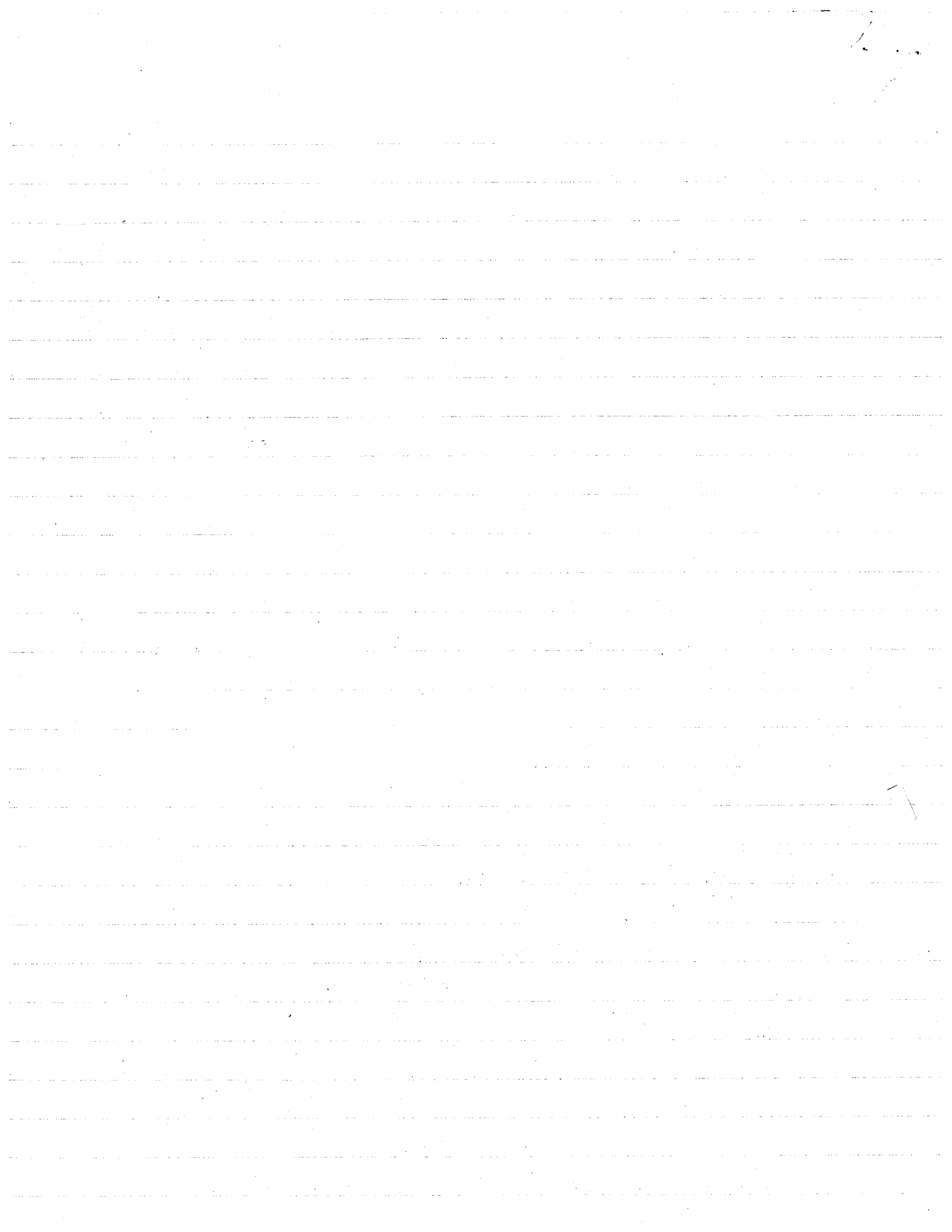
The only terms to contribute are $(123, 231, 312)$ for the cyclic terms and $(321, 132, 213)$ for the anticyclic terms. Thus look at all $a_i a_j$ and $a_j a_i$ terms for $k=3$ (eg $\epsilon_{123} a_1 a_2$ and $\epsilon_{213} a_2 a_1$). We note that they cancel, since the factor ϵ for one term will be negative of the other term and when summed give zero.

Thus for each cyclic terms there is an anti cyclic term. Hence $\epsilon_{ijk} a_i a_j = 0$.

note: there are 3 scalar eqns for $k=1, 2, 3$

4. Show that $\epsilon_{ijk} \epsilon_{kji} = -6$

10 Note that $\epsilon_{kji} = -\epsilon_{ijk}$ thus $\epsilon_{ijk} \epsilon_{kji} = -\epsilon_{ijk} \epsilon_{ijk}$. Since only 6 terms contribute $[ij(123, 231, 312) \text{ and } (321, 132, 213)]$, we can see that $-(\epsilon_{ijk})^2$ will give -1 for each of these terms. Since there are 6, $\epsilon_{ijk} \epsilon_{kji} = -6$. All other terms



give zero since either one of the terms ϵ_{ijk} or ϵ_{kji} will be zero. Thus $\epsilon_{ijk}\epsilon_{kji} = -6$.

5. Let $\Psi = \psi_{ij} \mathbf{e}_i \mathbf{e}_j$

a. Show that $\nabla \times \Psi = \psi_{ljk} \epsilon_{kli} \mathbf{e}_i \mathbf{e}_j$

10 $\nabla = \mathbf{e}_k \frac{\partial}{\partial x_k}$ thus $\nabla \times \Psi = \mathbf{e}_k \frac{\partial}{\partial x_k} \times \psi_{ij} \mathbf{e}_i \mathbf{e}_j = \psi_{ij,k} (\mathbf{e}_k \times \mathbf{e}_i) \mathbf{e}_j$

but $\mathbf{e}_k \times \mathbf{e}_i = \epsilon_{kil} \mathbf{e}_l \therefore \nabla \times \Psi = \psi_{ij,k} \epsilon_{kil} \mathbf{e}_l \mathbf{e}_j$

since indices are dummies then if I interchange all i's with all l's I get

$\nabla \times \Psi = \psi_{lj,k} \epsilon_{kli} \mathbf{e}_i \mathbf{e}_j$ ✓

b. Show that $\Psi \times \nabla = \psi_{il,k} \epsilon_{lki} \mathbf{e}_i \mathbf{e}_j$

10 $\nabla = \mathbf{e}_k \frac{\partial}{\partial x_k}$ thus $\Psi \times \nabla = \psi_{ij} \mathbf{e}_i (\mathbf{e}_j \times \mathbf{e}_k) \frac{\partial}{\partial x_k} = \mathbf{e}_i (\mathbf{e}_j \times \mathbf{e}_k) \psi_{ij,k}$

but $\mathbf{e}_j \times \mathbf{e}_k = \epsilon_{jkl} \mathbf{e}_l \therefore \Psi \times \nabla = \mathbf{e}_i \mathbf{e}_l \epsilon_{jkl} \psi_{ij,k}$ ✓

since indices are dummies then if I interchange all j's with all l's I get

$\Psi \times \nabla = \mathbf{e}_i \mathbf{e}_j \epsilon_{lkj} \psi_{il,k}$

6. Let $\Phi = \phi_{ij} \mathbf{e}_i \mathbf{e}_j$ and $\Psi = \psi_{kl} \mathbf{e}_k \mathbf{e}_l$

10 a. Show that $\Phi \cdot \Psi = \phi_{im} \psi_{mj} \mathbf{e}_i \mathbf{e}_j$

$\Phi \cdot \Psi = \phi_{ij} \mathbf{e}_i \mathbf{e}_j \cdot \psi_{kl} \mathbf{e}_k \mathbf{e}_l = \phi_{ij} \psi_{kl} \mathbf{e}_i \mathbf{e}_l (\delta_{jk}) = \phi_{ij} \psi_{jl} \mathbf{e}_i \mathbf{e}_l$

since j and l are dummy variables replace all j's by m's and then all l's by j's;

hence $\phi_{im} \psi_{mj} \mathbf{e}_i \mathbf{e}_j = \Phi \cdot \Psi$

10 b. Show that $\Phi : \Psi = \phi_{mn} \psi_{mn}$

$\Phi : \Psi = (\phi_{ij} \mathbf{e}_i \mathbf{e}_j) : (\psi_{kl} \mathbf{e}_k \mathbf{e}_l) = \phi_{ij} \psi_{kl} (\mathbf{e}_i \cdot \mathbf{e}_k) (\mathbf{e}_j \cdot \mathbf{e}_l)$
 $= \phi_{ij} \psi_{kl} \delta_{ik} \delta_{jl} = \phi_{ij} \psi_{ij}$ ✓

since j and i are dummy variables, replace all i's by m's and all j's by n's for result

$\therefore \Phi : \Psi = \phi_{mn} \psi_{mn}$

1-27

[The following text is extremely faint and largely illegible. It appears to be a series of lines, possibly a list or a document, but the content cannot be accurately transcribed.]

DIVISION OF APPLIED MECHANICS
DEPARTMENT OF MECHANICAL ENGINEERING

238A Theory of Elasticity

Autumn 1978

Problem Set No. 2

Due 1 Nov. 78

- ✓ 1. The components of stress at a point in a body referred to a rectangular Cartesian system of coordinates are given by

$$\begin{array}{ll} \sigma_{xx} = 5 \times 10^6 \text{ Pa} & \sigma_{xy} = 5 \times 10^6 \text{ Pa} \\ \sigma_{yy} = 0 & \sigma_{yz} = -7.5 \times 10^6 \text{ Pa} \\ \sigma_{zz} = -3 \times 10^6 \text{ Pa} & \sigma_{zx} = 8 \times 10^6 \text{ Pa} \end{array}$$

Show that the magnitude of traction acting across a plane [whose normal has direction cosines $1/2, 1/2, 1/\sqrt{2}$ with respect to the x, y, z axes, respectively] is $11.2 \times 10^6 \text{ Pa}$ and that the normal and tangential components of traction across the same plane have the magnitudes of $2.65 \times 10^6 \text{ Pa}$ and $10.9 \times 10^6 \text{ Pa}$, respectively.

- ✓ 2. At a point of a body located on the boundary whose direction cosines are l, m and n , with respect to a rectangular Cartesian coordinate system x, y, z , the surface traction is known to have a magnitude a and be parallel to x . Furthermore, it is known that $\sigma_{xy} = \sigma_{xz} = \sigma_{zz} = 0$.

Show that the remaining components of stress at that point are

$$\sigma_{zy} = 0, \sigma_{yy} = 0, \sigma_{xx} = a/l.$$

- ✓ 3. Verify the expressions for the three invariants of stress in terms of components of stress using indicial tensor notation.
- ✓ 4. Show that if the traction at a point across any three planes [whose normals are not coplanar] is in each case perpendicular to the plane, then the stress is hydrostatic (i.e., isotropic). Write the equation of motion for such a state of stress in indicial notation.

- ✓ 5. In several applications of analysis of stress at a point, it is shown that a special role is played by traction acting across planes which have like orientation with respect to all three principal directions. There are 8 such planes and they form an octahedron. The normal and shear components of traction across these planes are referred to as the octahedral normal stress σ_{oct} and octahedral shear stress τ_{oct} .

Show that

$$\sigma_{\text{oct}} = \frac{1}{3} (\sigma_1 + \sigma_2 + \sigma_3)$$

$$= \frac{1}{3} (\sigma_{xx} + \sigma_{yy} + \sigma_{zz})$$

$$\tau_{\text{oct}} = \frac{1}{3} \sqrt{(\sigma_1 - \sigma_2)^2 + (\sigma_2 - \sigma_3)^2 + (\sigma_3 - \sigma_1)^2}$$

$$= \frac{1}{3} \sqrt{(\sigma_{xx} - \sigma_{yy})^2 + (\sigma_{yy} - \sigma_{zz})^2 + (\sigma_{zz} - \sigma_{xx})^2 + 6(\sigma_{xy}^2 + \sigma_{yz}^2 + \sigma_{zx}^2)}$$

where $\sigma_1, \sigma_2, \sigma_3$ are the principal components of stress.

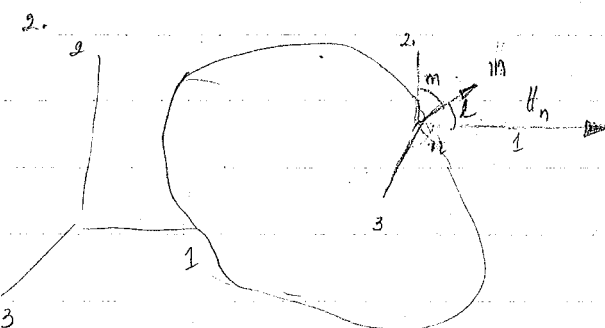
- ✓ 6. It is sometimes useful to decompose the stress tensor into its isotropic (or spherical) and its deviatoric (or distortional) components. The isotropic component is defined as having a matrix whose 3 terms on the main diagonal are all equal and whose terms off the diagonal all vanish. Show that the invariants of the stress deviator I_o, II_o, III_o are expressed in terms of the invariants I, II, III of the complete stress tensor as

$$I_o = 0$$

$$II_o = II - \frac{1}{3} I^2$$

$$III_o = III - \frac{1}{3} I (II - \frac{2}{9} I^2)$$

need to
write these
2 up



$$t_n = m \cdot [t_j e_j]$$

where $t_1 = \sigma_{xx} e_x + \sigma_{xy} e_y + \sigma_{xz} e_z = \sigma_{xx} e_x$

$$t_2 = \sigma_{yx} e_x + \sigma_{yy} e_y + \sigma_{yz} e_z = \sigma_{yy} e_y + \sigma_{yz} e_z$$

$$t_3 = \sigma_{zx} e_x + \sigma_{zy} e_y + \sigma_{zz} e_z = \sigma_{zy} e_y$$

$$\therefore t_n = m \cdot \sigma = l t_1 + m t_2 + n t_3$$

$$= l \sigma_{xx} e_x + m (\sigma_{yy} e_y + e_z \sigma_{yz}) + n \sigma_{zy} e_y$$

now since $t_n \parallel e_x \Rightarrow t_n = \alpha e_x + \beta e_y + \gamma e_z$ where $\beta = \gamma = 0$

$$\Rightarrow (m \sigma_{yy} + n \sigma_{zy}) = 0 \text{ and } \sigma_{yz} = 0 \text{ but } \sigma_{yz} = \sigma_{zy} \Rightarrow \sigma_{yy} = 0$$

$$\therefore \|t_n\|^2 = a^2 = \alpha^2 + \beta^2 + \gamma^2 = l^2 \sigma_{xx}^2 \text{ or } \boxed{\sigma_{xx} = \frac{a}{l}}$$

in order to find σ_{yy} let me take $t_n \cdot e_y = [l \sigma_{xx} e_x + m (\sigma_{yy} e_y) + n \sigma_{zy} e_z]$

$$t_{n_i} = m_i \cdot [t_j e_j] =$$

$$\exists. n_i \cdot n_j \neq 0$$

$$[t_1, l_1 + t_2 m_1 + t_3 n_1] = \left\{ \alpha n_1 \right\}$$

$$[t_1, l_2 + t_2 m_2 + t_3 n_2] = \left\{ \beta n_2 \right\}$$

$$[t_1, l_3 + t_2 m_3 + t_3 n_3] = \left\{ \gamma n_3 \right\}$$

$$\begin{pmatrix} l_1 & m_1 & n_1 \\ l_2 & m_2 & n_2 \\ l_3 & m_3 & n_3 \end{pmatrix} \begin{pmatrix} t_1 \\ t_2 \\ t_3 \end{pmatrix} = \begin{pmatrix} \alpha n_1 \\ \beta n_2 \\ \gamma n_3 \end{pmatrix}$$

3

$$\begin{vmatrix} \sigma_{11} - \lambda & \sigma_{12} & \sigma_{13} \\ \sigma_{21} & \sigma_{22} - \lambda & \sigma_{23} \\ \sigma_{31} & \sigma_{32} & \sigma_{33} - \lambda \end{vmatrix} = \begin{vmatrix} \sigma_{11} \sigma_{22} \sigma_{33} - \lambda (\sigma_{11} + \sigma_{22} + \sigma_{33}) + \lambda^2 & \sigma_{11} \sigma_{23} - \lambda (\sigma_{11} + \sigma_{23}) + \lambda^2 \\ \sigma_{11} \sigma_{33} - \lambda (\sigma_{11} + \sigma_{33}) + \lambda^2 & \sigma_{11} \sigma_{33} - \lambda (\sigma_{11} + \sigma_{33}) + \lambda^2 \\ \sigma_{11} \sigma_{33} - \lambda (\sigma_{11} + \sigma_{33}) + \lambda^2 & \sigma_{11} \sigma_{33} - \lambda (\sigma_{11} + \sigma_{33}) + \lambda^2 \end{vmatrix}$$

$$= (\sigma_{11} - \lambda)(\sigma_{33} - \lambda)(\sigma_{22} - \lambda) + \sigma_{12} \sigma_{23} \sigma_{31} + \sigma_{13} \sigma_{32} \sigma_{21} - \sigma_{13} (\sigma_{22} - \lambda)(\sigma_{31}) - \sigma_{12} \sigma_{21} (\sigma_{33} - \lambda) - \sigma_{11} \sigma_{32} \sigma_{23} = 0$$

$$\text{or } I = (\sigma_{11} + \sigma_{22} + \sigma_{33}) = \sigma_{kk}$$

$$II = [\sigma_{13} \sigma_{31} - \sigma_{12} \sigma_{21} - \sigma_{32} \sigma_{23} + \sigma_{11} \sigma_{22} + \sigma_{22} \sigma_{33} + \sigma_{11} \sigma_{33}]$$

$$III = [\sigma_{11} \sigma_{22} \sigma_{33} + \sigma_{12} \sigma_{23} \sigma_{31} + \sigma_{13} \sigma_{32} \sigma_{21} - \sigma_{13} \sigma_{22} \sigma_{31} - \sigma_{12} \sigma_{21} \sigma_{33} - \sigma_{11} \sigma_{32} \sigma_{23}]$$

go to (2)

now $\Pi = \frac{1}{2} e_{mik} e_{nje} \sigma_{ij} \sigma_{kl} \delta_{mn}$

$e_{123}, e_{231}, e_{312}$ are +

$e_{321}, e_{213}, e_{132}$ are -

δ_{11}

$\Pi = \frac{1}{2} e_{mik} e_{nje} \sigma_{ij} \sigma_{kl}$

$$\frac{1}{2} [e_{123}^+ (e_{123}^+ \sigma_{22} \sigma_{33} + e_{231}^+ \sigma_{23} \sigma_{31} + e_{312}^+ \sigma_{21} \sigma_{32} + e_{321}^+ \sigma_{22} \sigma_{31} + e_{213}^+ \sigma_{21} \sigma_{33} + e_{132}^+ \sigma_{23} \sigma_{32}) + e_{231} (e_{123} \sigma_{22} \sigma_{33} + e_{231} \sigma_{23} \sigma_{31} + e_{312} \sigma_{21} \sigma_{32} + e_{321} \sigma_{22} \sigma_{31} + e_{213} \sigma_{21} \sigma_{33} + e_{132} \sigma_{23} \sigma_{32}) + e_{321} \sigma_{22} \sigma_{31} + e_{213} \sigma_{21} \sigma_{33} + e_{132} \sigma_{23} \sigma_{32}]$$

$$\frac{1}{2} [(\sigma_{22} \sigma_{33} + \sigma_{23} \sigma_{31} + \sigma_{21} \sigma_{32} - \sigma_{22} \sigma_{31} - \sigma_{21} \sigma_{33} - \sigma_{23} \sigma_{32}) + (\sigma_{32} \sigma_{13} + \sigma_{33} \sigma_{11} + \sigma_{31} \sigma_{12} - \sigma_{32} \sigma_{11} - \sigma_{31} \sigma_{13} - \sigma_{33} \sigma_{12}) + (\sigma_{12} \sigma_{23} + \sigma_{13} \sigma_{21} + \sigma_{11} \sigma_{22} - \sigma_{12} \sigma_{21} - \sigma_{11} \sigma_{23} - \sigma_{13} \sigma_{22}) - (\sigma_{22} \sigma_{13} + \sigma_{23} \sigma_{11} + \sigma_{21} \sigma_{12} - \sigma_{22} \sigma_{11} - \sigma_{21} \sigma_{13} - \sigma_{23} \sigma_{12}) - (\sigma_{12} \sigma_{33} + \sigma_{13} \sigma_{31} + \sigma_{11} \sigma_{32} - \sigma_{12} \sigma_{31} - \sigma_{11} \sigma_{33} - \sigma_{13} \sigma_{32}) - (\sigma_{32} \sigma_{23} + \sigma_{33} \sigma_{21} + \sigma_{31} \sigma_{22} - \sigma_{32} \sigma_{21} - \sigma_{31} \sigma_{23} - \sigma_{33} \sigma_{22})]$$

$$\frac{1}{2} [2\sigma_{22} \sigma_{33} + 2\sigma_{33} \sigma_{11} + 2\sigma_{11} \sigma_{22} - 2\sigma_{13}^2 - 2\sigma_{12}^2 - 2\sigma_{32}^2] = \Pi$$

now $\frac{1}{6} e_{mik} e_{nje} \sigma_{ij} \sigma_{kl} \sigma_{mn} = \frac{1}{6} [\sigma_{11} \sigma_{22} \sigma_{33} + \sigma_{12} \sigma_{23} \sigma_{31} + \sigma_{13} \sigma_{21} \sigma_{32} - \sigma_{13} \sigma_{22} \sigma_{31} - \sigma_{12} \sigma_{21} \sigma_{33} - \sigma_{11} \sigma_{23} \sigma_{32}]$

$$+ (\sigma_{21} \sigma_{32} \sigma_{13} + \sigma_{22} \sigma_{33} \sigma_{11} + \sigma_{23} \sigma_{31} \sigma_{12} - \sigma_{23} \sigma_{32} \sigma_{11} - \sigma_{22} \sigma_{31} \sigma_{13} - \sigma_{21} \sigma_{33} \sigma_{12}) + (\sigma_{31} \sigma_{12} \sigma_{23} + \sigma_{32} \sigma_{13} \sigma_{21} + \sigma_{33} \sigma_{11} \sigma_{22} - \sigma_{33} \sigma_{12} \sigma_{21} - \sigma_{32} \sigma_{11} \sigma_{23} - \sigma_{31} \sigma_{13} \sigma_{22}) - (\sigma_{31} \sigma_{32} \sigma_{13} + \sigma_{32} \sigma_{23} \sigma_{11} + \sigma_{33} \sigma_{21} \sigma_{12} - \sigma_{33} \sigma_{22} \sigma_{11} - \sigma_{32} \sigma_{21} \sigma_{13} - \sigma_{31} \sigma_{23} \sigma_{12}) - (\sigma_{21} \sigma_{12} \sigma_{33} + \sigma_{22} \sigma_{13} \sigma_{31} + \sigma_{23} \sigma_{11} \sigma_{32} - \sigma_{23} \sigma_{12} \sigma_{31} - \sigma_{22} \sigma_{11} \sigma_{33} - \sigma_{21} \sigma_{13} \sigma_{32}) - (\sigma_{11} \sigma_{32} \sigma_{23} + \sigma_{12} \sigma_{33} \sigma_{21} + \sigma_{13} \sigma_{33} \sigma_{22} - \sigma_{13} \sigma_{32} \sigma_{21} - \sigma_{12} \sigma_{31} \sigma_{23} - \sigma_{11} \sigma_{33} \sigma_{22})]$$

$$\frac{1}{6} [6\sigma_{11} \sigma_{33} \sigma_{22} + 6\sigma_{12} \sigma_{23} \sigma_{31} + 6\sigma_{13} \sigma_{32} \sigma_{21} - 6\sigma_{13} \sigma_{22} \sigma_{31} - 6\sigma_{12} \sigma_{21} \sigma_{33} - 6\sigma_{11} \sigma_{23} \sigma_{32}] = \text{III}$$

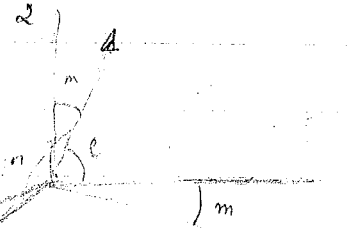
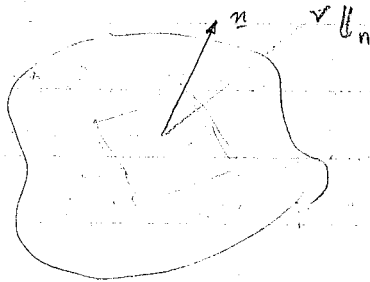
problem #1 on other side

$m =$

$$t_x = (5 \times 10^6) e_x + (5 \times 10^6) e_y + (8 \times 10^6) e_z$$

$$t_y = (5 \times 10^6) e_x + (0) e_y + (-7.5 \times 10^6) e_z$$

$$t_z = (8 \times 10^6) e_x + (-7.5 \times 10^6) e_y + (-3 \times 10^6) e_z$$



$$t_n = \frac{1}{2} t_1 + \frac{1}{2} t_2 + \frac{1}{\sqrt{2}} t_3 = n \cdot \sigma$$

$$\frac{t_n}{10^6} = \left[2.5 + 2.5 + 4\sqrt{2} \right] e_1 + \left[2.5 + \frac{(-7.5)}{\sqrt{2}} \right] e_2 + \left[4 + \frac{(-7.5)}{2} + \frac{(-3)}{\sqrt{2}} \right] e_3$$

$$= \left(\frac{10.657 \times 10^6}{10.7 \times 10^6} \right) e_1 + \left[\frac{-2.803 \times 10^6}{-2.8 \times 10^6} \right] e_2 + \left[\frac{-1.871 \times 10^6}{-1.9 \times 10^6} \right] e_3$$

$$\|t_n\| = 11.177 \times 10^6 \text{ Pa} \quad \text{or} \quad 11.2 \times 10^6 \text{ Pa}$$

now we know that

$$t_n = (t_n \cdot e'_x) e'_x + (t_n \cdot e'_y) e'_y + (t_n \cdot e'_z) e'_z$$

=

$$m \cdot t_n = \quad \text{now} \quad t_n = p \cdot n + (q) \cdot \pi$$

$$m \cdot m = \left(\frac{1}{4} + \frac{1}{4} + \frac{1}{2} \right) = 1$$

$$m \cdot t_n = p + q (m \cdot \pi) = p$$

$$\|t_n\| = (p^2 + q^2)^{1/2}$$

$$m \cdot t_n = \left\{ \frac{1}{2} (10.657) + \frac{1}{2} (-2.803) + \frac{\sqrt{2}}{2} (-1.871) \right\} \times 10^6$$

$$= 2.605 \times 10^6 \quad \text{or} \quad 2.61$$

$$\therefore (t_n^2 - p^2)^{1/2} = q = 10.869 \times 10^6 \text{ Pa}$$

Since $\hat{t}_n$ is the plane is $\perp$ to the plane $\Rightarrow \hat{t}_n$ is proportional to n the normal to the plane $\therefore$

4. $\hat{t}_{n_1} = \alpha n_1 \Rightarrow \sigma \cdot n_1 - \alpha n_1 = (\sigma - \mathbb{I}\alpha) \cdot n_1 = 0$ use $\hat{t}_{n_i} = n_i \cdot \sigma = \alpha_j \delta_{ji} n_i$
 $\hat{t}_{n_2} = \beta n_2 \Rightarrow \sigma \cdot n_2 - \beta n_2 = (\sigma - \mathbb{I}\beta) \cdot n_2 = 0$ and take
 $\hat{t}_{n_3} = \gamma n_3 \Rightarrow \sigma \cdot n_3 - \gamma n_3 = (\sigma - \mathbb{I}\gamma) \cdot n_3 = 0$ $n_k \cdot n_l \times n_i \cdot \sigma = \alpha_j \delta_{ji} (n_k \cdot n_l \times n_i)$

now take approp $n_k \cdot n_l \times n_i \cdot \sigma - n_l \cdot n_k \times n_i \cdot \sigma = (\alpha_j - \alpha_i) n_k \cdot n_l \times n_i =$

$\therefore n_2 \cdot (\sigma - \mathbb{I}\alpha) \cdot n_1 = n_1 \cdot (\sigma - \mathbb{I}\alpha) \cdot n_2 = 0$ this gives the component of $\hat{t}_{n_1}$ in n_2

$n_1 \cdot (\sigma - \mathbb{I}\beta) \cdot n_2 = n_2 \cdot (\sigma - \mathbb{I}\beta) \cdot n_1 = 0$ this gives the component of $\hat{t}_{n_2}$ in n_1
(non coplanarity condition)

$\Rightarrow$ since $n_k \cdot n_l \times n_i = n_l \cdot n_k \times n_i \Rightarrow$ for $n_k \cdot n_l \times n_i \neq 0 \Rightarrow \alpha_j = \alpha_i$

$\Rightarrow$ subtract them and remembering that σ & $\mathbb{I}$ are symmetric

$n_1 \cdot [(\sigma - \mathbb{I}\alpha) - (\sigma - \mathbb{I}\beta)] \cdot n_2 = 0$

$n_1 \cdot [\mathbb{I}(\beta - \alpha)] \cdot n_2 = (\beta - \alpha) n_1 \cdot \mathbb{I} \cdot n_2 = (\beta - \alpha) n_1 \cdot n_2 = 0$

since n_1 is not coplanar with $n_2 \Rightarrow \beta - \alpha = 0$ or $\beta = \alpha$

we can then do this with $\frac{\text{the others}}{n_1, n_2}$ to prove $\alpha = \gamma \therefore \alpha = \beta = \gamma$

thus we have shown that for three planes having n_1, n_2, n_3 as normals and that they are noncoplanar $\sigma = \mathbb{I}\alpha$. But we didn't choose the planes in any special manner $\therefore$ the results can be generalized to any 3 planes whose normals are not coplanar. Thus $\sigma = \mathbb{I}\alpha$ or

$\sigma = \begin{pmatrix} \alpha & 0 & 0 \\ 0 & \alpha & 0 \\ 0 & 0 & \alpha \end{pmatrix}$ this is equivalent to hydrostatic stress

from the principle of linear momentum and

$\int_S \hat{t}_n ds = \int_S n \cdot \sigma ds = \int_V \nabla \cdot \sigma dV = \alpha \int_V \nabla \cdot n dV$

$\therefore \int_S \hat{t}_n ds + \int_V f dV = \int_V \rho \frac{\partial^2 u_i}{\partial t^2} dV \Rightarrow \int_V [\alpha \nabla \cdot n + f - \rho \frac{\partial^2 u_i}{\partial t^2}] dV = 0$

$\therefore$ since $\nabla \cdot n = 0 \Rightarrow$ for any volume $f = \rho \frac{\partial^2 u_i}{\partial t^2}$ or $f_i = \rho \frac{\partial^2 u_i}{\partial t^2}$

$a n_1 + b n_2 + c n_3 = 0$

$a + b q + c r = 0$

$q a + b + c s = 0$

$a r + b s + c = 0$

$\begin{pmatrix} 1 & q & r \\ q & 1 & s \\ r & s & 1 \end{pmatrix} \begin{pmatrix} a \\ b \\ c \end{pmatrix} = \begin{pmatrix} 0 \\ 0 \\ 0 \end{pmatrix}$

$\det A = (1 + qsr - r^2 - q^2 - s^2)$

$\begin{pmatrix} 1 & q & r \\ 0 & 1-q^2 & r \\ 0 & r & 1-s^2 \end{pmatrix}$

$$\sigma \cdot m_1 \times m_2 = \alpha (m_1 \times m_2)$$

$$m_3 \cdot (\sigma \cdot m_1 \times m_2) =$$

$$t_{n_1} = \sigma \cdot m_1 = \alpha m_1$$

$$t_{n_1} \times m_2 = \sigma \cdot m_1 \times m_2 = \alpha m_1 \times m_2$$

$$m_3 \cdot t_{n_1} \times m_2 = (m_3 \cdot \sigma \cdot m_1 \times m_2) = \alpha (m_3 \cdot m_1 \times m_2)$$

$$m_1 \cdot t_{n_2} \times m_3 = m_1 \cdot \sigma \cdot m_2 \times m_3 = \beta (m_1 \cdot m_2 \times m_3) = \beta (m_3 \cdot m_1 \times m_2)$$

$$\text{for } m_1 \cdot m_2 \times m_3 = -m_1 \cdot m_3 \times m_2 = m_3 \cdot m_1 \times m_2$$

$$\therefore m_3 \cdot t_{n_1} \times m_2 - m_1 \cdot t_{n_2} \times m_3 = (\alpha - \beta) (m_3 \cdot m_1 \times m_2)$$

$$m_3 \cdot t_{n_1} \times m_2 - m_1 \cdot t_{n_2} \times m_3 = (\alpha - \beta) (m_3 \cdot m_1 \times m_2)$$

$$m_3 \cdot m_1 \cdot \sigma \cdot (m_3 \cdot m_1 \times m_2) - \sigma \cdot (m_3 \cdot m_1 \times m_2) =$$

$$\sigma \cdot [m_3 \cdot m_1 \times m_2 - m_1 \cdot m_2 \times m_3]$$

$$\text{if coplanar } m_1 \cdot m_2, m_2 \cdot m_3, \Rightarrow m_1 \cdot m_2$$

$$\cos a_1, \cos a_2, \cos a_3$$

$$\therefore a_1 + a_2 = a_3$$

$$0 = (\alpha - \beta) m_3 \cdot m_1 \times m_2$$

$$\text{but for non coplanarity } m_3 \cdot m_1 \times m_2 \neq 0 \Rightarrow \alpha = \beta$$

$$t_{n_1} = \sigma \cdot m_1 = \alpha m_1$$

$$\sigma \cdot [m_1 \cdot m_2 \times m_3] = \alpha (m_1 \cdot m_2 \times m_3)$$

$$t_{n_1} \cdot m_2 = \sigma \cdot m_1 \cdot m_2 = \alpha m_1 \cdot m_2$$

$$\sigma \cdot [m_2 \cdot m_1 \times m_3] = \beta (m_2 \cdot m_1 \times m_3)$$

$$\cos(a_1 + a_2) = \cos a_3$$

$$\cos a_1 \cos a_2 - \sin a_1 \sin a_2 = \cos a_3$$

$$(m_1 \cdot m_2) (m_2 \cdot m_3) - (m_1 \times m_2) \cdot (m_2 \times m_3)$$

$$\psi = m_1 \cdot m_2 \quad \psi = m_2 \cdot m_3$$

$$\psi : \psi$$

$$\begin{cases} a_1 \sigma_{x1} + a_2 \sigma_{x2} = \alpha a_1 \\ a_1 t_1 + a_2 t_2 + a_3 t_3 = \alpha a_1 + \alpha a_2 \\ t_1 e_1 + a_2 e_2 + a_3 e_3 \end{cases}$$

$$\sigma \cdot m_1 = \alpha m_1$$

$$\sigma \cdot m_1 \cdot m_2 =$$

$$\left(\begin{array}{c} \text{---} \end{array} \right) \left(\begin{array}{c} \text{---} \end{array} \right) =$$

$$1 = ap + bq$$

$$q = ar + b$$

$$p = a + br$$

$$1 = a^2 + 2abr + b^2$$

$$1 = a^2 + 2abr + b^2$$

$$\beta = \alpha \text{ or } m_1 \cdot m_2 = 0$$

$$\text{if } m_1 \cdot m_2 = 0 \Rightarrow m_1 \perp m_2$$

$$\gamma = \beta \text{ or } m_3 \cdot m_2 = 0$$

$$m_2 \perp m_3$$

$$\alpha = \gamma \text{ or } m_1 \cdot m_3 = 0$$

$$m_1 \perp m_3$$

the

$$\text{if coplanar } (m_1, m_2)$$

Assume

$$m_1 = \text{must be a linear combination of the other 2}$$

$$\therefore m_1 = m m_2 + p m_3$$

$$\therefore m_1 \cdot m_2 = m + p m_3 \cdot m_2$$

$$\text{but } m_i \cdot m_j = \delta_{ij} \Rightarrow m, p = 0 \Rightarrow$$

$$\Rightarrow m_1 = a m_2 + b m_3$$

$$\Rightarrow m_1 \cdot m_2 = a + b m_2 \cdot m_3$$

$$\Rightarrow 0 = a(\alpha - \beta) + b(\alpha - \beta) m_2 \cdot m_3$$

$$(\alpha - \beta) a$$

$$\text{or } m_2 \cdot m_3 = -\frac{a}{b}$$

$$m_1 = a m_2 + b m_3 \text{ coplanar}$$

$$\therefore m_1 \cdot m_1 = 1 = a(m_2 \cdot m_2) + b(m_3 \cdot m_3)$$

$$m_2 \cdot m_1 = 1 = a + b$$

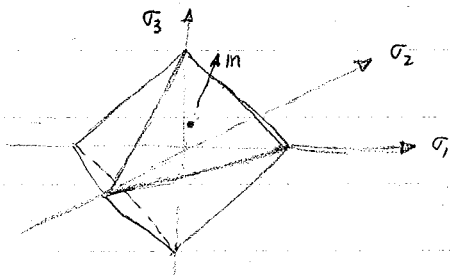
$$p = a + br$$

$$q = ar + b$$

$$\therefore (\alpha - \beta) m_1 \cdot m_2 = 0$$

$$\{a + b m_2 \cdot m_3\}$$

let us look at



For an octahedral plane the normal $n = a_1 e_1 + a_2 e_2 + a_3 e_3$

where e_i are the unit vectors in the principal directions

so that $|n|^2 = a_1^2 + a_2^2 + a_3^2$ and $a_i = n \cdot e_i$; but all directional cosines are the same in all directions $\therefore a_i^2 = \frac{1}{3} \quad i=1,2,3$

$$\text{or } n = \frac{1}{\sqrt{3}} (e_1 + e_2 + e_3)$$

now we know that $\sigma = \begin{pmatrix} \sigma_1 & 0 & 0 \\ 0 & \sigma_2 & 0 \\ 0 & 0 & \sigma_3 \end{pmatrix} \therefore t_n = \sigma \cdot n = \frac{1}{\sqrt{3}} (\sigma_1 e_1 + \sigma_2 e_2 + \sigma_3 e_3)$

now since $t_n = a n + b \pi$ then $n \cdot t_n = a = \frac{1}{\sqrt{3}} (1, 1, 1) \cdot \frac{1}{\sqrt{3}} (\sigma_1, \sigma_2, \sigma_3) = \frac{1}{3} (\sigma_1 + \sigma_2 + \sigma_3)$
 $= \sigma_{oct} n + \tau_{oct} \pi$

and since $|t_n|^2 = a^2 + b^2 \therefore b = \sqrt{|t_n|^2 - a^2}$

$|t_n|^2 = \frac{1}{3} (\sigma_1^2 + \sigma_2^2 + \sigma_3^2) \therefore b = \sqrt{\frac{3}{9} (\sigma_1^2 + \sigma_2^2 + \sigma_3^2) - \frac{1}{9} (\sigma_1 + \sigma_2 + \sigma_3)^2}$ after expansion

$$\tau_{oct} = b = \frac{1}{3} \sqrt{(\sigma_1 - \sigma_2)^2 + (\sigma_2 - \sigma_3)^2 + (\sigma_3 - \sigma_1)^2}$$

since from the problem #3 we will assume that we can the cubic equation in terms of the principle stresses $\sigma_1, \sigma_2, \sigma_3$ the $(\sigma - \sigma_1)(\sigma - \sigma_2)(\sigma - \sigma_3) = 0$ or $\sigma^3 + (\sigma_1 + \sigma_2 + \sigma_3)\sigma^2 + (\sigma_1\sigma_2 + \sigma_1\sigma_3 + \sigma_2\sigma_3)\sigma - (\sigma_1\sigma_2\sigma_3) = 0$ or $I_1 = \sigma_1 + \sigma_2 + \sigma_3, I_2 = \sigma_1\sigma_2 + \sigma_1\sigma_3 + \sigma_2\sigma_3, I_3 = \sigma_1\sigma_2\sigma_3$

since $-\lambda^3 + I\lambda^2 - II\lambda + III = -\sigma^3 + I\sigma^2 - II\sigma + III_3 \Rightarrow I = I_1, II = II_2, III = III_3$

or from above

$$a = \frac{1}{3} I_1 = \frac{1}{3} I = \frac{1}{3} (\sigma_{11} + \sigma_{22} + \sigma_{33}) = \sigma_{oct}$$

$$\text{now } 9\tau_{oct}^2 = 3(\sigma_1^2 + \sigma_2^2 + \sigma_3^2) - [(\sigma_1 + \sigma_2)^2 + 2\sigma_3(\sigma_1 + \sigma_2) + \sigma_3^2] = \cancel{\sigma_1^2 + \sigma_2^2} + 2\sigma_1\sigma_2 + 2\sigma_3\sigma_1 + 2\sigma_3\sigma_2 + \cancel{\sigma_3^2}$$

$$= -2(\sigma_1\sigma_2 + \sigma_3\sigma_1 + \sigma_3\sigma_2) + 2(\sigma_1^2 + \sigma_2^2 + \sigma_3^2)$$

$$(\sigma_1^2 + \sigma_2^2 - 2\sigma_1\sigma_2) + (\sigma_1^2 + \sigma_3^2 - 2\sigma_3\sigma_1) + (\sigma_2^2 + \sigma_3^2 - 2\sigma_3\sigma_2)$$

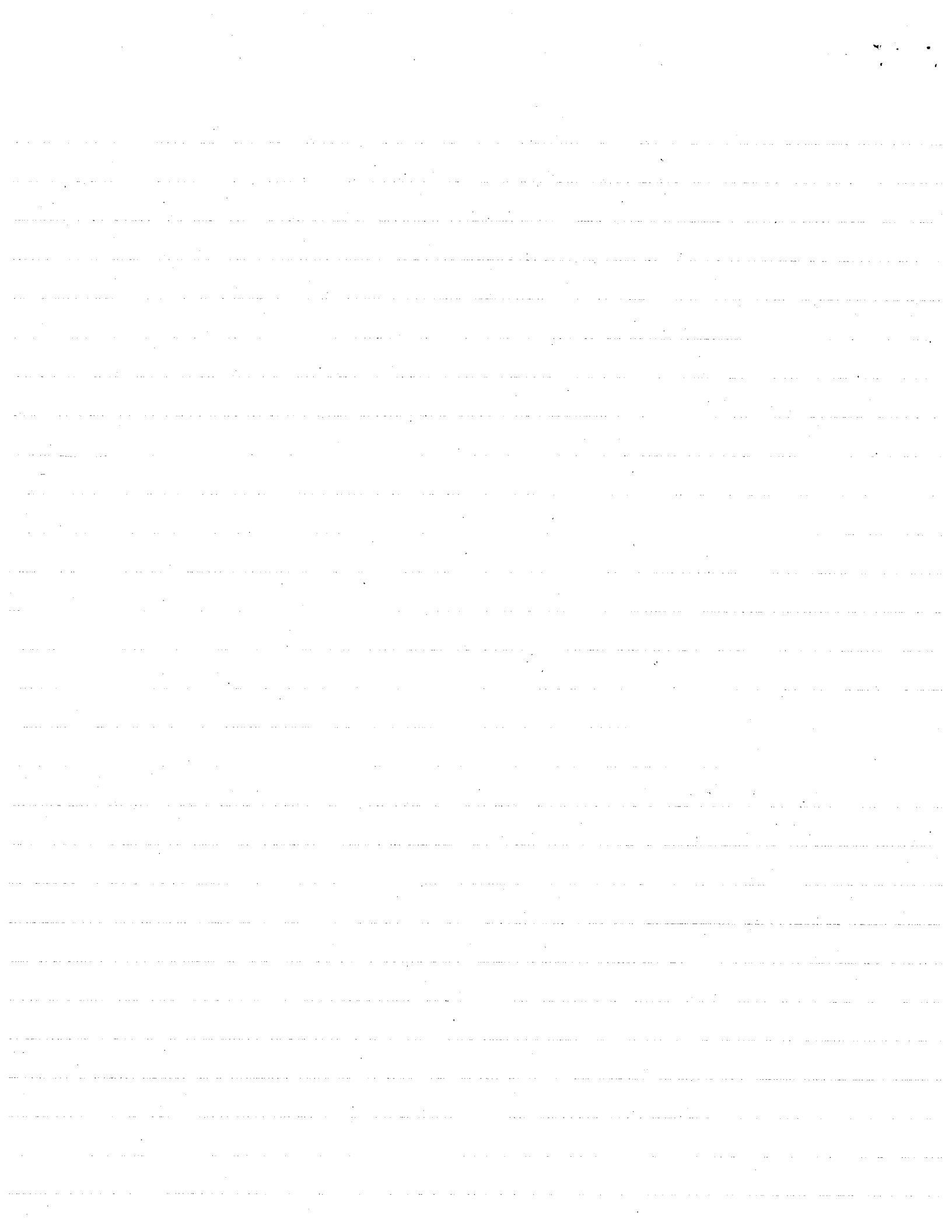
$$9\tau_{oct}^2 = 2I_1^2 - 6I_2 = 2(\sigma_1 + \sigma_2 + \sigma_3)^2 - 6(\sigma_1\sigma_2 + \sigma_1\sigma_3 + \sigma_2\sigma_3)$$

$$= 2[\sigma_1^2 + \sigma_2^2 + \sigma_3^2 + 2\sigma_1\sigma_2 + 2\sigma_3\sigma_1 + 2\sigma_3\sigma_2] - 6(\sigma_1\sigma_2 + \sigma_1\sigma_3 + \sigma_2\sigma_3)$$

$$= (\sigma_1^2 + \sigma_2^2 - 2\sigma_1\sigma_2) + \dots + \dots$$

$$\therefore 9\tau_{oct}^2 = 2I_1^2 - 6I_2 = 2I^2 - 6II = 2(\sigma_{11} + \sigma_{22} + \sigma_{33})^2 - 6(\sigma_{11}\sigma_{22} + \sigma_{22}\sigma_{33} + \sigma_{11}\sigma_{33} - \sigma_{13}^2 - \sigma_{12}^2 - \sigma_{32}^2)$$

$$= 2[\sigma_{11}^2 + \sigma_{22}^2 + \sigma_{33}^2 + 2\sigma_{11}\sigma_{22} + 2\sigma_{11}\sigma_{33} + 2\sigma_{22}\sigma_{33}] - 6\sigma$$



$$\begin{aligned}
 \text{or } 4\tau_{oct}^2 &= 2(\sigma_{11}^2 + \sigma_{22}^2 + \sigma_{33}^2) - 2(\sigma_{11}\sigma_{22} + \sigma_{22}\sigma_{33} + \sigma_{11}\sigma_{33}) + 6(\sigma_{13}^2 + \sigma_{12}^2 + \sigma_{23}^2) \\
 &= (\sigma_{11}^2 + \sigma_{22}^2 - 2\sigma_{11}\sigma_{22}) + (\sigma_{11}^2 + \sigma_{33}^2 - 2\sigma_{11}\sigma_{33}) + (\sigma_{22}^2 + \sigma_{33}^2 - 2\sigma_{22}\sigma_{33}) + 6(\sigma_{13}^2 + \sigma_{12}^2 + \sigma_{23}^2) \\
 \tau_{oct} &= \frac{1}{3} \sqrt{(\sigma_{11} - \sigma_{22})^2 + (\sigma_{11} - \sigma_{33})^2 + (\sigma_{22} - \sigma_{33})^2 + 6(\sigma_{13}^2 + \sigma_{12}^2 + \sigma_{23}^2)} \quad \text{qed.}
 \end{aligned}$$

6. if we define $\tau_{ij} = \sigma_0 \delta_{ij} + \tau'_{ij}$ where $\sigma_0 = \frac{1}{3}(\sigma_1 + \sigma_2 + \sigma_3) = \frac{1}{3}(\sigma_{11} + \sigma_{22} + \sigma_{33}) = \frac{1}{3}I_1$
then $\tau'_{ij} = \tau_{ij} - \sigma_0 \delta_{ij}$ To determine the principal stress deviations $\Rightarrow \tau'_{ij} n_j = \lambda \delta_{ij} n_j$

find $\tau'_{ij} - \lambda \delta_{ij} = \tau_{ij} - \sigma' \delta_{ij}$ where $\sigma' = \sigma_0 + \lambda$

$$\begin{aligned}
 \therefore \tau'_{ij} &= \begin{pmatrix} \sigma_{11}' - \lambda' & \sigma_{12} & \sigma_{13} \\ \sigma_{21} & \sigma_{22}' - \lambda' & \sigma_{23} \\ \sigma_{31} & \sigma_{32} & \sigma_{33}' - \lambda' \end{pmatrix} = (\sigma_{11}' - \lambda')(\sigma_{22}' - \lambda')(\sigma_{33}' - \lambda') + \sigma_{12}\sigma_{23}\sigma_{31} + \sigma_{13}\sigma_{32}\sigma_{21} \\
 &\quad - \sigma_{13}^2(\sigma_{22}' - \lambda') - \sigma_{12}^2(\sigma_{33}' - \lambda') - \sigma_{23}^2(\sigma_{11}' - \lambda') \\
 &\quad - \lambda^3 + \lambda^2(\sigma_{11}' + \sigma_{22}' + \sigma_{33}') + [\sigma_{11}'\sigma_{22}'\sigma_{33}' + \sigma_{12}\sigma_{23}\sigma_{31} + \sigma_{13}\sigma_{32}\sigma_{21} - \sigma_{22}'\sigma_{13}^2 - \sigma_{12}^2\sigma_{33}' - \sigma_{23}^2\sigma_{11}'] \\
 &\quad - \sigma_{23}^2\sigma_{11}'] = \lambda [-\sigma_{13}\sigma_{31} + \sigma_{12}\sigma_{21} + \sigma_{32}\sigma_{23} + \sigma_{11}'\sigma_{22}' + \sigma_{22}'\sigma_{33}' + \sigma_{33}'\sigma_{11}'] = 0 \\
 &\quad - \lambda [\text{II} - 2\sigma_0 I + 3\sigma_0^2]
 \end{aligned}$$

$$\begin{aligned}
 \text{where } \sigma_{11}' &= \sigma_{11} - \sigma_0 \\
 \sigma_{22}' &= \sigma_{22} - \sigma_0 \\
 \sigma_{33}' &= \sigma_{33} - \sigma_0 \\
 \sigma_{KK}' &= \sigma_{KK} - 3\sigma_0 = \sigma_{KK} - \sigma_{KK} = 0
 \end{aligned}$$

$$\begin{aligned}
 I_0 &= 0 \quad \text{II}_0 = \text{II} - 3\sigma_0^2 \\
 &= \text{II} - 3\left(\frac{1}{9}I\right)^2 = \text{II} - \frac{1}{3}I^2
 \end{aligned}$$

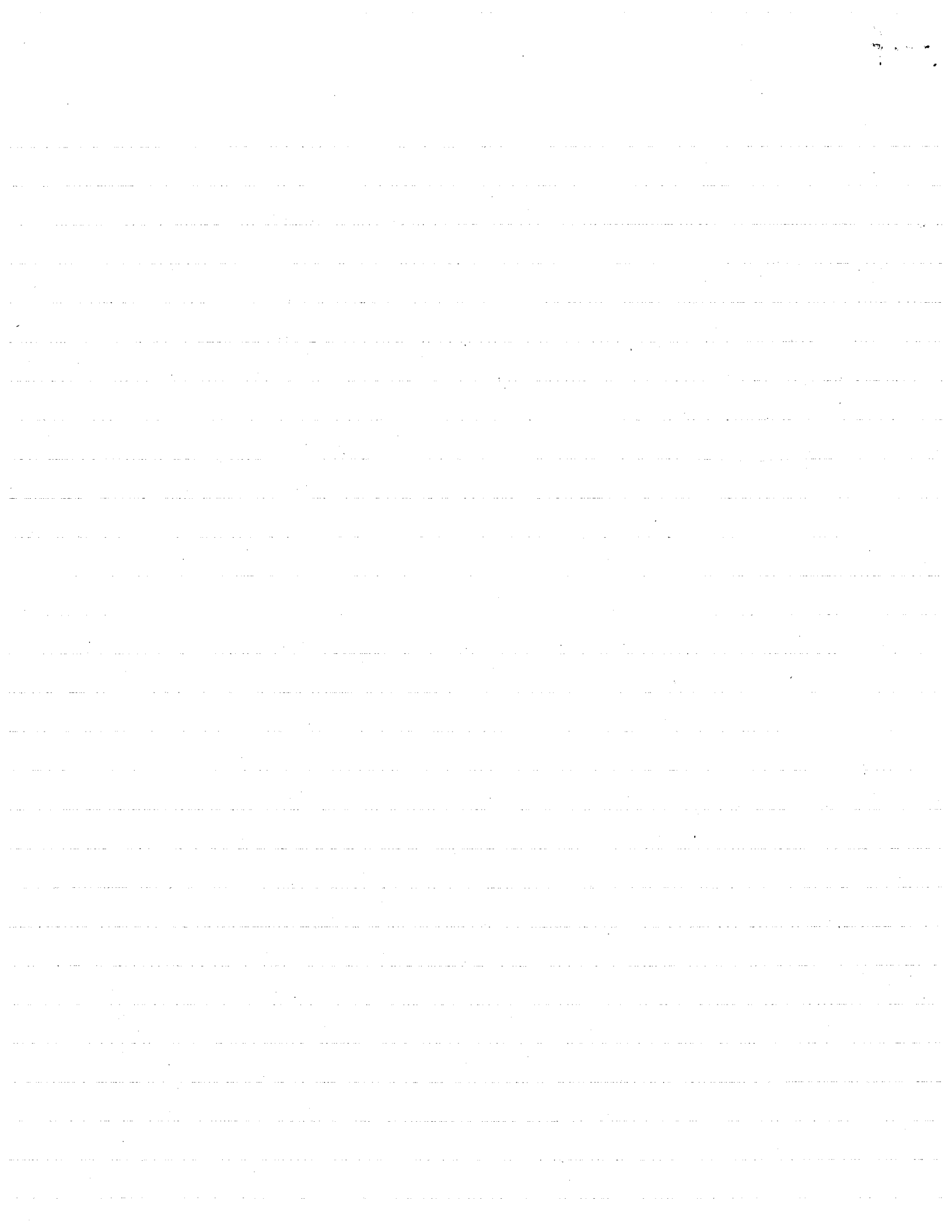
$$\therefore -\lambda^3 + I_0 \lambda^2 - \text{II}_0 \lambda + \text{III}_0 = 0$$

$$\begin{aligned}
 \sigma_{11}'\sigma_{22}' &= \sigma_{11}\sigma_{22} - \sigma_0(\sigma_{11} + \sigma_{22}) + \sigma_0^2 \\
 \sigma_{22}'\sigma_{33}' &= \sigma_{22}\sigma_{33} - \sigma_0(\sigma_{22} + \sigma_{33}) + \sigma_0^2 \\
 \sigma_{33}'\sigma_{11}' &= \sigma_{33}\sigma_{11} - \sigma_0(\sigma_{33} + \sigma_{11}) + \sigma_0^2 \\
 \sigma_{KK}'\sigma_{jj}' &= \sigma_{KK}\sigma_{jj} - 2\sigma_0(\sigma_{11} + \sigma_{22} + \sigma_{33}) + 3\sigma_0^2 \\
 &= -6\sigma_0^2 + 3\sigma_0^2 = -3\sigma_0^2
 \end{aligned}$$

$$\begin{aligned}
 \text{now } \sigma_{11}'\sigma_{22}'\sigma_{33}' &= [\sigma_{11}\sigma_{22} - \sigma_0(\sigma_{11} + \sigma_{22}) + \sigma_0^2][\sigma_{33} - \sigma_0] = \sigma_{11}\sigma_{22}\sigma_{33} - \sigma_0(\sigma_{11} + \sigma_{22})\sigma_{33} + \sigma_0^2\sigma_{33} - \sigma_0\sigma_{11}\sigma_{22} + \sigma_0^2(\sigma_{11} + \sigma_{22}) - \sigma_0^3 \\
 &= \sigma_{11}\sigma_{22}\sigma_{33} - \sigma_0(\sigma_{11}\sigma_{33} + \sigma_{11}\sigma_{22} + \sigma_{22}\sigma_{33}) + \sigma_0^2(\sigma_{11} + \sigma_{22} + \sigma_{33}) - \sigma_0^3 \\
 &\quad + 3\sigma_0^3 - \sigma_0^3
 \end{aligned}$$

$$\begin{aligned}
 \text{then } \text{III}_0 &= \left\{ \sigma_{11}\sigma_{22}\sigma_{33} + \sigma_{12}\sigma_{23}\sigma_{31} + \sigma_{13}\sigma_{32}\sigma_{21} - \sigma_{22}\sigma_{13}^2 - \sigma_{12}^2\sigma_{33} - \sigma_{23}^2\sigma_{11} \right\} \\
 &\quad - \sigma_0(\sigma_{11}\sigma_{33} + \sigma_{11}\sigma_{22} + \sigma_{22}\sigma_{33}) + \sigma_0(\sigma_{13}^2 + \sigma_{12}^2 + \sigma_{23}^2) + 2\sigma_0^3
 \end{aligned}$$

$$\text{III} - \sigma_0[\text{II}] + 2\sigma_0^3 = \text{III}_0 - \frac{1}{3}I \cdot \text{II} + \frac{2}{27}I^3 = \text{III} - \frac{1}{3}I(\text{II} - \frac{2}{9}I^2)$$



Problem Set #2

120/100

1. Given σ at a point for a cartesian system and the normal n at the given point. Find the magnitude of the traction acting at the point and the magnitude of the normal and tangential components on the plane whose normal is n .

For

$\sigma = \begin{pmatrix} 5 & 5 & 8 \\ 5 & 0 & -7.5 \\ 8 & -7.5 & -3 \end{pmatrix} \times 10^6$ then we can write $t_n = n \cdot \sigma$ where t_n is the traction on the plane whose normal is n . $\sigma = \sigma_{ij} e_i e_j$ & $t_i = \sigma_{ij} e_j$

$\therefore t_n = \frac{1}{2} t_1 + \frac{1}{2} t_2 + \frac{1}{\sqrt{2}} t_3$ where $n = \frac{1}{2} [e_1 + e_2 + \sqrt{2} e_3]$

or $t_n = (10.657 \times 10^6) e_1 + (-2.803 \times 10^6) e_2 + (-1.871 \times 10^6) e_3 = \alpha_i e_i$

Thus $|t_n| = (\alpha_i \cdot \alpha_i)^{1/2} = [(10.657)^2 + (-2.803)^2 + (-1.871)^2]^{1/2} \times 10^6$
 $= 11.177 \times 10^6 \text{ Pa} \approx 11.2 \times 10^6 \text{ Pa}$

Since $t_n = (n \cdot t_n) n + (\pi \cdot t_n) \pi$ where π is the unit vector tangent to the plane

now if we take $n \cdot t_n =$ magnitude of t_n in the normal direction. To find $\pi \cdot t_n =$ mag. of t_n in the tangential direction use the fact that $|t_n|^2 = (n \cdot t_n)^2 + (\pi \cdot t_n)^2$ and

$(\pi \cdot t_n) = \{|t_n|^2 - (n \cdot t_n)^2\}^{1/2}$

$\therefore n \cdot t_n = \frac{1}{2} (10.657 \times 10^6) + \frac{1}{2} (-2.803 \times 10^6) + \frac{1}{\sqrt{2}} (-1.871 \times 10^6)$
 $= 2.605 \times 10^6 \text{ Pa} \approx 2.61 \times 10^6 \text{ Pa}$

$\therefore \pi \cdot t_n = \{(11.2 \times 10^6)^2 - (2.61 \times 10^6)^2\}^{1/2} = 10.892 \times 10^6 \text{ Pa} \approx 10.9 \times 10^6 \text{ Pa}$



2.

Given $\begin{pmatrix} \sigma_{xx} & 0 & 0 \\ 0 & \sigma_{yy} & \sigma_{yz} \\ 0 & \sigma_{zy} & 0 \end{pmatrix}$ in the cartesian coordinate system for a given point on the boundary whose direction cosines are l, m, n . The magnitude of the surface traction is a and is parallel to x . Find $\sigma_{zy}, \sigma_{yy}, \sigma_{xx}$.

We note that at the surface the normal $n = l e_x + m e_y + n e_z$ can be dotted into σ to get the traction at the point in the n dir

$$\therefore t_n = n \cdot \sigma = l t_1 + m t_2 + n t_3 \quad \text{where } \sigma = t_i e_i$$

$$= l \sigma_{xx} e_x + m (\sigma_{yy} e_y + \sigma_{yz} e_z) + n \sigma_{zy} e_y$$

now since $t_n \parallel e_x \Rightarrow t_n = \alpha e_x + \beta e_y + \gamma e_z$ where $\beta = \gamma = 0$

but $\beta = m \sigma_{yy} + n \sigma_{zy} = 0$ and $m \sigma_{yz} = \gamma = 0$ or $\sigma_{yz} = 0$ if $m \neq 0$ (which will be true in general unless the normal through the boundary is $\perp$ to the y axis). But $\sigma_{yz} = \sigma_{zy}$ and $\Rightarrow \sigma_{yy} = 0$.

We therefore have $\boxed{\sigma_{yy} = \sigma_{yz} = \sigma_{zy} = 0}$. We are also told that $|t_n| = a$. But

$$|t_n| = a = l \sigma_{xx} \Rightarrow \boxed{\sigma_{xx} = a/l}$$

3. Verify I, II, III in terms of stress using indicial notation.

We are going to use the case where $\sum_{i=1}^3$ (i.e. the three coordinate axes)

From $\sigma \cdot n - \lambda \mathbb{I} \cdot n = 0$ we obtained the following determinant

$$\begin{vmatrix} \sigma_{11} - \lambda & \sigma_{12} & \sigma_{13} \\ \sigma_{21} & \sigma_{22} - \lambda & \sigma_{23} \\ \sigma_{31} & \sigma_{32} & \sigma_{33} - \lambda \end{vmatrix} = 0 ; \quad \text{when expanding we obtain: } (\sigma_{11} - \lambda)(\sigma_{33} - \lambda)(\sigma_{22} - \lambda) + \sigma_{12}\sigma_{23}\sigma_{31} \\ + \sigma_{13}\sigma_{32}\sigma_{21} - \sigma_{13}\sigma_{31}(\sigma_{22} - \lambda) - \sigma_{12}\sigma_{22}(\sigma_{33} - \lambda) - \sigma_{32}\sigma_{23}(\sigma_{11} - \lambda) = 0$$

when expanded and rearranged we obtain $-\lambda^3 + \text{I} \lambda^2 - \text{II} \lambda + \text{III} = 0$, where

$$\text{I} = (\sigma_{11} + \sigma_{22} + \sigma_{33}) \quad \text{or } \boxed{\text{I} = \sigma_{kk}}$$

$$\text{II} = -\sigma_{13}\sigma_{31} - \sigma_{12}\sigma_{21} - \sigma_{32}\sigma_{23} + \sigma_{11}\sigma_{22} + \sigma_{22}\sigma_{33} + \sigma_{11}\sigma_{33}$$

$$\text{III} = \sigma_{11}\sigma_{22}\sigma_{33} + \sigma_{12}\sigma_{23}\sigma_{31} + \sigma_{13}\sigma_{32}\sigma_{21} - \sigma_{13}\sigma_{22}\sigma_{31} - \sigma_{12}\sigma_{21}\sigma_{33} - \sigma_{11}\sigma_{32}\sigma_{23}$$

now $\text{II} = \frac{1}{2} e_{mik} e_{njl} \sigma_{ij} \sigma_{kl} \delta_{mn}$ and we will prove this by expanding it



$$\text{II} \stackrel{?}{=} \frac{1}{2} \epsilon_{mik} \epsilon_{nje} \sigma_{ij} \sigma_{kl} \delta_{mn} = \frac{1}{2} \epsilon_{mik} \epsilon_{nje} \sigma_{ij} \sigma_{kl} \quad \text{since } \delta_{mn} = \begin{cases} 0 & m \neq n \\ 1 & m = n \end{cases}$$

now note that $\epsilon_{mik}, \epsilon_{nje}$ only give terms when mik or njl are $(123, 231, 312, 321, 213, 132)$

Thus the only terms that come into being are:

$$\frac{1}{2} \left[\epsilon_{123} \epsilon_{123} \sigma_{22} \sigma_{33} + \epsilon_{123} \epsilon_{132} \sigma_{23} \sigma_{32} + \epsilon_{231} \epsilon_{231} \sigma_{33} \sigma_{11} + \epsilon_{231} \epsilon_{213} \sigma_{31} \sigma_{13} + \epsilon_{312} \epsilon_{312} \sigma_{11} \sigma_{22} + \epsilon_{312} \epsilon_{321} \sigma_{12} \sigma_{21} \right. \\ \left. + \epsilon_{321} \epsilon_{312} \sigma_{21} \sigma_{12} + \epsilon_{321} \epsilon_{321} \sigma_{22} \sigma_{11} + \epsilon_{213} \epsilon_{231} \sigma_{13} \sigma_{31} + \epsilon_{213} \epsilon_{213} \sigma_{11} \sigma_{33} + \epsilon_{132} \epsilon_{123} \sigma_{32} \sigma_{23} + \epsilon_{132} \epsilon_{132} \sigma_{33} \sigma_{22} \right]$$

Knowing that $\epsilon_{123, 231, 312} = 1$ and that $\epsilon_{321, 213, 132} = -1$ we obtain the result that

$$\frac{1}{2} \epsilon_{mik} \epsilon_{nje} \sigma_{kl} \sigma_{ij} \delta_{mn} = \frac{1}{2} \left[2 \sigma_{22} \sigma_{33} + 2 \sigma_{33} \sigma_{11} + 2 \sigma_{11} \sigma_{22} - 2 \sigma_{13} \sigma_{31} - 2 \sigma_{12} \sigma_{21} - 2 \sigma_{32} \sigma_{23} \right] \equiv \text{II}$$

$$\therefore \boxed{\text{II} = \frac{1}{2} \epsilon_{mik} \epsilon_{nje} \sigma_{kl} \sigma_{ij} \delta_{mn}}$$

Now $\text{III} = \frac{1}{6} \epsilon_{mik} \epsilon_{nje} \sigma_{ij} \sigma_{kl} \sigma_{mn}$ and we will prove this by expansion also

again we note that $\epsilon_{mik}, \epsilon_{nje}$ only give nonzero terms when mik, njl are $(123, 231, 312, 321, 213, 132)$

Thus we obtain for $\frac{1}{6} \epsilon_{mik} \epsilon_{nje} \sigma_{ij} \sigma_{kl} \sigma_{mn}$:

$$\frac{1}{6} \left[(\sigma_{11} \sigma_{22} \sigma_{33} + \sigma_{12} \sigma_{23} \sigma_{31} + \sigma_{13} \sigma_{21} \sigma_{32} - \sigma_{13} \sigma_{22} \sigma_{31} - \sigma_{12} \sigma_{21} \sigma_{33} - \sigma_{11} \sigma_{23} \sigma_{32}) + (\sigma_{21} \sigma_{32} \sigma_{13} + \sigma_{22} \sigma_{33} \sigma_{11} + \sigma_{23} \sigma_{31} \sigma_{12} \right. \\ \left. - \sigma_{23} \sigma_{32} \sigma_{11} - \sigma_{22} \sigma_{31} \sigma_{13} - \sigma_{21} \sigma_{33} \sigma_{12}) + (\sigma_{31} \sigma_{12} \sigma_{23} + \sigma_{32} \sigma_{13} \sigma_{21} + \sigma_{33} \sigma_{11} \sigma_{22} - \sigma_{33} \sigma_{12} \sigma_{21} - \sigma_{32} \sigma_{11} \sigma_{23} - \sigma_{31} \sigma_{13} \sigma_{22}) \right. \\ \left. - (\sigma_{31} \sigma_{22} \sigma_{13} + \sigma_{32} \sigma_{23} \sigma_{11} + \sigma_{33} \sigma_{21} \sigma_{12} - \sigma_{33} \sigma_{22} \sigma_{11} - \sigma_{32} \sigma_{21} \sigma_{13} - \sigma_{31} \sigma_{23} \sigma_{12}) - (\sigma_{21} \sigma_{12} \sigma_{33} + \sigma_{22} \sigma_{13} \sigma_{31} + \sigma_{23} \sigma_{11} \sigma_{32} \right. \\ \left. - \sigma_{23} \sigma_{12} \sigma_{31} - \sigma_{22} \sigma_{11} \sigma_{33} - \sigma_{21} \sigma_{13} \sigma_{32}) - (\sigma_{11} \sigma_{32} \sigma_{23} + \sigma_{12} \sigma_{33} \sigma_{21} + \sigma_{13} \sigma_{33} \sigma_{22} - \sigma_{13} \sigma_{32} \sigma_{21} - \sigma_{12} \sigma_{31} \sigma_{23} - \sigma_{11} \sigma_{33} \sigma_{22}) \right]$$

$$\therefore \frac{1}{6} \epsilon_{mik} \epsilon_{nje} \sigma_{ij} \sigma_{kl} \sigma_{mn} = \frac{1}{6} \left[6 \sigma_{11} \sigma_{22} \sigma_{33} + 6 \sigma_{12} \sigma_{23} \sigma_{31} + 6 \sigma_{13} \sigma_{32} \sigma_{21} - 6 \sigma_{13} \sigma_{22} \sigma_{31} - 6 \sigma_{12} \sigma_{21} \sigma_{33} - 6 \sigma_{11} \sigma_{23} \sigma_{32} \right] \equiv \text{III}$$

$$\therefore \boxed{\text{III} = \frac{1}{6} \epsilon_{mik} \epsilon_{nje} \sigma_{kl} \sigma_{ij} \sigma_{mn}}$$



(p20) ex credit

4. Given that the tractions on 3 separate planes are perpendicular to the plane and that the normals of these 3 planes are not coplanar.

Since $\mathbf{t}_n$ is given $\perp$ to its plane, then $\mathbf{t}_n$ is proportional to $\mathbf{n}$, the normal to the plane.

Therefore if $\mathbf{t}_{n_i} = \mathbf{n}_i \cdot \boldsymbol{\sigma} = \alpha_i \mathbf{n}_i \delta_{ij} \quad i=1,2,3$ then

$$\mathbf{n}_2 \times \mathbf{t}_{n_1} = \mathbf{n}_2 \times \mathbf{n}_1 \cdot \boldsymbol{\sigma} = \alpha_1 \mathbf{n}_2 \times \mathbf{n}_1 \quad \text{and} \quad \mathbf{n}_3 \cdot \mathbf{n}_2 \times \mathbf{t}_{n_1} = \mathbf{n}_3 \cdot \mathbf{n}_2 \times \mathbf{n}_1 \cdot \boldsymbol{\sigma} = \alpha_1 \mathbf{n}_3 \cdot \mathbf{n}_2 \times \mathbf{n}_1$$

but we can write

$$\mathbf{n}_3 \times \mathbf{t}_{n_2} = \mathbf{n}_3 \times \mathbf{n}_2 \cdot \boldsymbol{\sigma} = \alpha_2 \mathbf{n}_3 \times \mathbf{n}_2 \quad \text{and} \quad \mathbf{n}_1 \cdot \mathbf{n}_3 \times \mathbf{t}_{n_2} = \mathbf{n}_1 \cdot \mathbf{n}_3 \times \mathbf{n}_2 \cdot \boldsymbol{\sigma} = \alpha_2 \mathbf{n}_1 \cdot \mathbf{n}_3 \times \mathbf{n}_2$$

$$\text{but} \quad \mathbf{n}_3 \cdot \mathbf{n}_1 \times \mathbf{n}_2 = \mathbf{n}_1 \cdot \mathbf{n}_3 \times \mathbf{n}_2$$

$$\therefore \mathbf{n}_3 \cdot \mathbf{n}_2 \times \mathbf{t}_{n_1} - \mathbf{n}_1 \cdot \mathbf{n}_3 \times \mathbf{t}_{n_2} = (\mathbf{n}_3 \cdot \mathbf{n}_2 \times \mathbf{n}_1 - \mathbf{n}_1 \cdot \mathbf{n}_3 \times \mathbf{n}_2) \cdot \boldsymbol{\sigma} = (\alpha_1 - \alpha_2) \mathbf{n}_3 \cdot \mathbf{n}_1 \times \mathbf{n}_2 = 0 \cdot \boldsymbol{\sigma} = 0$$

however since $\mathbf{n}_1, \mathbf{n}_2, \mathbf{n}_3$ are not coplanar $\mathbf{n}_3 \cdot \mathbf{n}_1 \times \mathbf{n}_2 \neq 0^* \Rightarrow \alpha_1 = \alpha_2$

We can also show in a similar manner that $\alpha_2 = \alpha_3$ and $\alpha_1 = \alpha_3$ or $\alpha_1 = \alpha_2 = \alpha_3 = -p$

Since we did not choose the planes in any particular manner, the results shown are true for any 3 planes whose normals are not coplanar. Thus $\boldsymbol{\sigma} = -p \mathbf{I}$. This result is exactly that of hydrostatics. Now from the principle of linear momentum

* to show this we note that $\mathbf{n}_1 \times \mathbf{n}_2 = \underline{\underline{\zeta}}$ a vector $\perp$ to both $\mathbf{n}_1$ & $\mathbf{n}_2$. Thus if $\mathbf{n}_3$ were coplanar then $\underline{\underline{\zeta}}$ would also be $\perp$ to $\mathbf{n}_3$. Hence $\mathbf{n}_3 \cdot \mathbf{n}_1 \times \mathbf{n}_2 = \mathbf{n}_3 \cdot \underline{\underline{\zeta}} = |\mathbf{n}_3| |\underline{\underline{\zeta}}| \cos(\mathbf{n}_3, \underline{\underline{\zeta}})$ where $\cos(\mathbf{n}_3, \underline{\underline{\zeta}}) = \cos \frac{\pi}{2} = 0$; thus $\mathbf{n}_3 \cdot \mathbf{n}_1 \times \mathbf{n}_2 = 0$ for coplanarity.

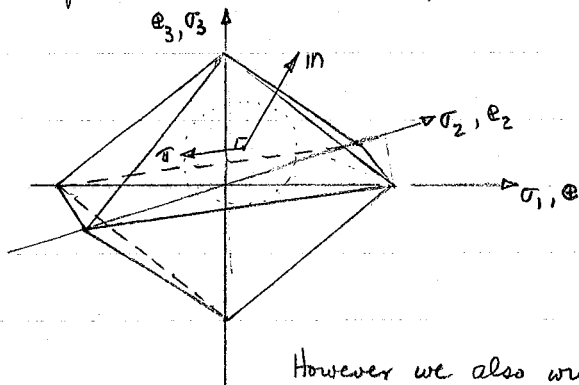
Using the principle of linear momentum that the rate of change of momentum in a given control volume = sum of all the forces acting on the volume. Since the rate of change of momentum is $\frac{\partial}{\partial t} \int_V \rho \mathbf{u}_i dV$, and the forces acting on the volume in general are pressure forces and body forces, then we can write, noting that ρ for materials we talk about is constant and for fixed volume

$$\int_V \rho \ddot{u}_i dV = \int_V - \frac{\partial p}{\partial x_i} dV + \int_V f_i dV \quad \text{or} \quad \int_V \left[\rho \ddot{u}_i + \frac{\partial p}{\partial x_i} - f_i \right] dV = 0$$

$$\text{or } \rho \ddot{u}_i = - \frac{\partial p}{\partial x_i} + f_i \quad \text{for arbitrary } dV.$$



5. Find the normal and shear stress on an octahedron. (A general outline of the results is given in Foundations of Solid Mechanics by Fung)



For an octahedron, the normal has equal directional cosines with the principal directions and thus $n = \frac{1}{\sqrt{3}} (e_1 + e_2 + e_3)$

Since we know that for this case

$$\sigma = \begin{pmatrix} \sigma_1 & 0 & 0 \\ 0 & \sigma_2 & 0 \\ 0 & 0 & \sigma_3 \end{pmatrix} \quad \text{then} \quad t_n = n \cdot \sigma = \frac{1}{\sqrt{3}} (\sigma_1 e_1 + \sigma_2 e_2 + \sigma_3 e_3)$$

which gives the stress tensor on the face of the octahedron

However we also write $t_n = a n + b \pi$ where $a = \sigma_{oct}$ and $b = \tau_{oct}$

$$\text{Thus } a = n \cdot t_n = \frac{1}{\sqrt{3}} (e_1 + e_2 + e_3) \cdot \frac{1}{\sqrt{3}} (\sigma_1 e_1 + \sigma_2 e_2 + \sigma_3 e_3) = \frac{1}{3} (\sigma_1 + \sigma_2 + \sigma_3)$$

$$\therefore \boxed{\sigma_{oct} = \frac{1}{3} (\sigma_1 + \sigma_2 + \sigma_3)}$$

we pick π to be a unit vector $\perp$ to n . Now as in problem 1 we can find b from

$$b = \sqrt{|t_n|^2 - a^2} \quad \text{where} \quad |t_n|^2 = t_n \cdot t_n = \frac{1}{3} (\sigma_1^2 + \sigma_2^2 + \sigma_3^2)$$

$$\text{Thus } b = \sqrt{\frac{1}{3} (\sigma_1^2 + \sigma_2^2 + \sigma_3^2) - \frac{1}{9} (\sigma_1 + \sigma_2 + \sigma_3)^2} \quad \text{After expansion and rearrangement of terms}$$

$$\boxed{\tau_{oct} = \frac{1}{3} \sqrt{(\sigma_1 - \sigma_2)^2 + (\sigma_2 - \sigma_3)^2 + (\sigma_3 - \sigma_1)^2}}$$

To obtain the other expressions we go back to problem #3 and realize that we can write the equation $-\lambda^3 + I\lambda^2 - II\lambda + III = 0$ as $-(\lambda - \sigma_1)(\lambda - \sigma_2)(\lambda - \sigma_3) = 0$

By expansion of this equation we get that $-\lambda^3 + I\lambda^2 - II\lambda + III = 0$ where

$$I_1 = \sigma_1 + \sigma_2 + \sigma_3; \quad II_2 = \sigma_1\sigma_2 + \sigma_1\sigma_3 + \sigma_2\sigma_3; \quad III_3 = \sigma_1\sigma_2\sigma_3. \quad \text{Now since } f(\lambda, I, II, III)$$

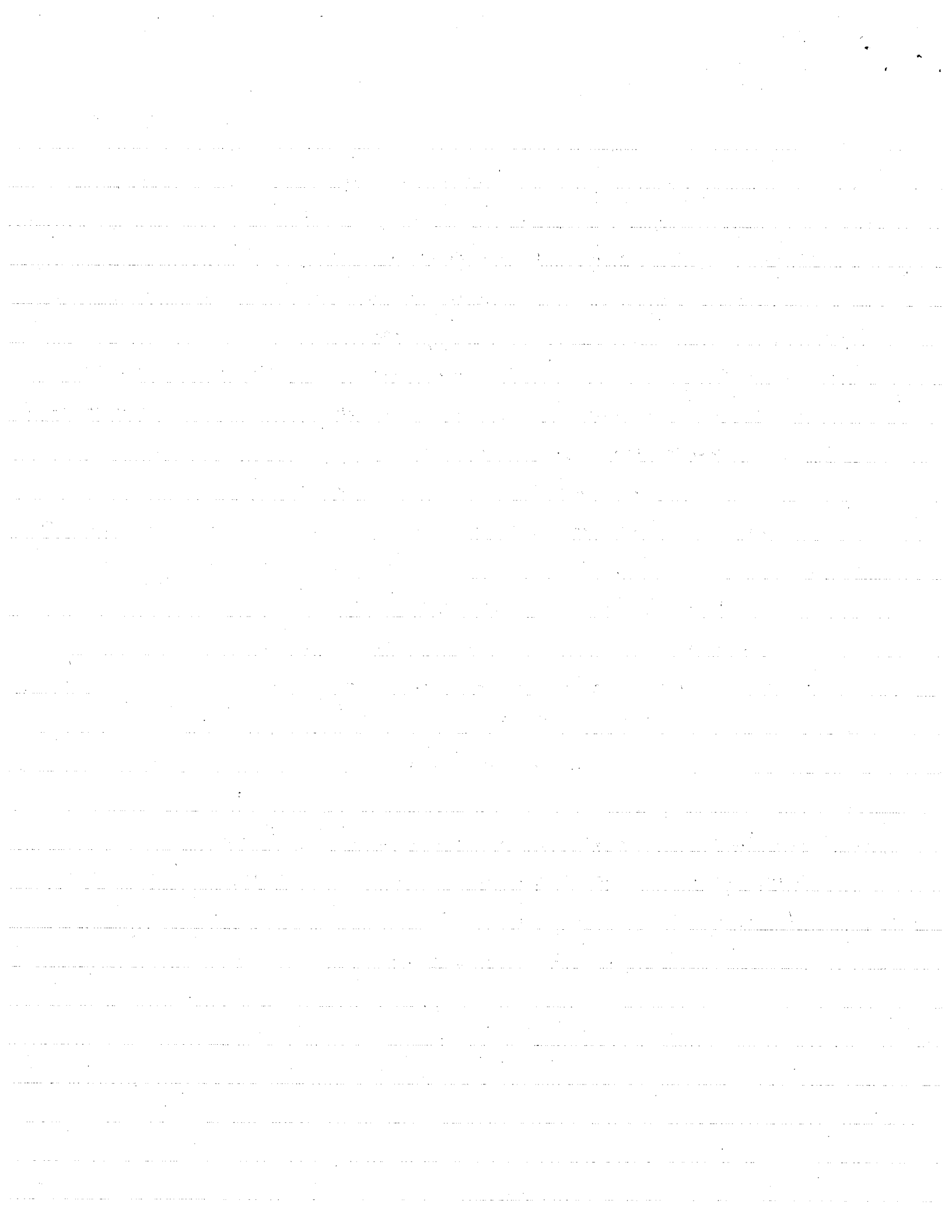
equals $g(\lambda, I_1, II_2, III_3)$ then $I = I_1, II = II_2, III = III_3$

$$\therefore \boxed{\text{since } \sigma_{oct} = \frac{1}{3} I_1 = \frac{1}{3} I = \frac{1}{3} (\sigma_{11} + \sigma_{22} + \sigma_{33})}$$

$$\text{Since } \tau_{oct} = \frac{1}{3} \sqrt{(\sigma_1 - \sigma_2)^2 + (\sigma_2 - \sigma_3)^2 + (\sigma_3 - \sigma_1)^2} \quad \text{then we will show that } 9\tau_{oct}^2 = 2I_1^2 - 6II_2$$

$$\text{but } 9\tau_{oct}^2 = 2\sigma_1^2 + 2\sigma_2^2 + 2\sigma_3^2 - 2(\sigma_1\sigma_2 + \sigma_1\sigma_3 + \sigma_2\sigma_3) = 2[\sigma_1^2 + \sigma_2^2 + \sigma_3^2 + 2(\sigma_1\sigma_2 + \sigma_2\sigma_3 + \sigma_1\sigma_3)] - 6(\sigma_1\sigma_3 + \sigma_1\sigma_2 + \sigma_2\sigma_3) = 2I_1^2 - 6II_2. \quad \text{Since } I_1 = I \text{ and } II_2 = II \text{ then } \boxed{9\tau_{oct}^2 = 2I^2 - 6II = 2I_1^2 - 6II_2}$$

$$\text{or } 9\tau_{oct}^2 = 2(\sigma_{11} + \sigma_{22} + \sigma_{33})^2 - 6(\sigma_{11}\sigma_{22} + \sigma_{22}\sigma_{33} + \sigma_{11}\sigma_{33} - \sigma_{12}^2 - \sigma_{23}^2 - \sigma_{13}^2). \quad \text{When we expand this, cancel and collect terms we get } 9\tau_{oct}^2 = (\sigma_{11} - \sigma_{22})^2 + (\sigma_{22} - \sigma_{33})^2 + (\sigma_{11} - \sigma_{33})^2 + 6(\sigma_{13}^2 + \sigma_{12}^2 + \sigma_{23}^2)$$



solving for τ_{oct} we get

$$\tau_{oct} = \frac{1}{3} \sqrt{(\sigma_{11} - \sigma_{22})^2 + (\sigma_{22} - \sigma_{33})^2 + (\sigma_{11} - \sigma_{33})^2 + 6(\sigma_{12}^2 + \sigma_{23}^2 + \sigma_{13}^2)}$$

6. Find the invariants of the stress deviator (again found in Fung)

If we decompose $\sigma = \sigma_0 \hat{\mathbb{I}} + \pi$ where $\sigma_0 = \frac{1}{3}(\sigma_1 + \sigma_2 + \sigma_3) = \frac{1}{3}I$, then $\pi = \sigma - \sigma_0 \hat{\mathbb{I}}$ where π is the stress deviator and $\hat{\mathbb{I}}$ is the idempfactor. In indicial notation $\tau_{ij} = \sigma_{ij} - \sigma_0 \delta_{ij}$

again we do what we did to find the principal stress - we find the principal deviations.

We will note that the principal stress deviation occurs when parallel to the vector normal to the plane where the stress deviator acts. Thus $\pi \cdot n = \lambda n$ or $\tau_{ij} n_j = \lambda n_i = \lambda \delta_{ij} n_j$

$$\text{Hence } (\tau_{ij} - \lambda \delta_{ij}) n_j = 0 \text{ or } (\pi - \lambda \hat{\mathbb{I}}) \cdot n = 0$$

For a solution to exist $\det(\pi - \lambda \hat{\mathbb{I}}) = 0$ which will be a cubic equation in λ of the form $-\lambda^3 + I_0 \lambda^2 - II_0 \lambda + III_0 = 0$ where, if we define $\sigma'_{ii} = \sigma_{ii} - \sigma_0$ (ie $\tau_{ii} = \sigma'_{ii} - \lambda$),

$$I_0 = (\sigma'_{11} + \sigma'_{22} + \sigma'_{33}) = \sigma_{11} + \sigma_{22} + \sigma_{33} - 3\sigma_0 = I - 3\left(\frac{1}{3}I\right) = 0 \quad \therefore \boxed{I_0 = 0};$$

$$III_0 = \sigma'_{11}\sigma'_{22}\sigma'_{33} + \sigma'_{12}\sigma'_{23}\sigma'_{31} + \sigma'_{13}\sigma'_{32}\sigma'_{21} - (\sigma_{13}^2 + \sigma_{12}^2 + \sigma_{23}^2) = (\sigma_{11}\sigma_{22} + \sigma_{22}\sigma_{33} + \sigma_{33}\sigma_{11} - [\sigma_{13}^2 + \sigma_{12}^2 + \sigma_{23}^2]) + 3\sigma_0^2 - 2\sigma_0(\sigma_{11} + \sigma_{22} + \sigma_{33}) \text{ when we expand. But the first bracket is } III \text{ and } \sigma_{11} + \sigma_{22} + \sigma_{33} = 3\sigma_0$$

$$\text{Thus } III_0 = III + 3\sigma_0^2 - 2\sigma_0(3\sigma_0) = III - 3\sigma_0^2 = III - 3\left(\frac{1}{9}I^2\right) = III - \frac{1}{3}I^2$$

$$\therefore \boxed{III_0 = III - \frac{1}{3}I^2};$$

$$\begin{aligned} II_0 &= \sigma'_{11}\sigma'_{22}\sigma'_{33} + \sigma'_{12}\sigma'_{23}\sigma'_{31} + \sigma'_{13}\sigma'_{32}\sigma'_{21} - \sigma_{22}^2\sigma'_{13} - \sigma_{12}^2\sigma'_{33} - \sigma_{23}^2\sigma'_{11} = [\sigma_{11}\sigma_{22}\sigma_{33}] - \sigma_0(\sigma_{11}\sigma_{33} + \sigma_{11}\sigma_{22} + \sigma_{22}\sigma_{33}) \\ &\quad + \sigma_0^2(\sigma_{11} + \sigma_{22} + \sigma_{33}) - \sigma_0^3 + [\sigma_{12}\sigma_{23}\sigma_{31} + \sigma_{13}\sigma_{32}\sigma_{21} - (\sigma_{22}\sigma_{13}^2 + \sigma_{12}^2\sigma_{33} + \sigma_{23}^2\sigma_{11})] + \sigma_0^2(\sigma_{13}^2 + \sigma_{12}^2 + \sigma_{23}^2) \\ &= III - \sigma_0 II + \sigma_0^2(3\sigma_0) - \sigma_0^3 = III - \sigma_0 II + 2\sigma_0^3 = III - \sigma_0(II - 2\sigma_0^2) = III - \frac{1}{3}I(II - \frac{2}{9}I^2) \end{aligned}$$

$$\therefore \boxed{II_0 = III - \frac{1}{3}I(II - \frac{2}{9}I^2)}$$



1. Components of ϕ with respect to Cartesian coordinates:

$$\phi = 10^6 \text{ Pa} \begin{bmatrix} 5 & 5 & 8 \\ 5 & 0 & -7.5 \\ 8 & -7.5 & -3 \end{bmatrix}$$

also given $m = \frac{1}{2} e_x + \frac{1}{2} e_y + \frac{\sqrt{2}}{2} e_z$

where m is the unit normal to the plane under study

a) $\Pi^n = m \cdot \phi$

In Dyadic form $\Pi^n = \left(\frac{1}{2} e_x + \frac{1}{2} e_y + \frac{\sqrt{2}}{2} e_z \right) \cdot \left(5 e_x e_x + 5 e_x e_y + 8 e_x e_z + 5 e_y e_x - 7.5 e_y e_z + 8 e_z e_x - 7.5 e_z e_y - 3 e_z e_z \right)$

$$= (10.66 e_x - 2.8 e_y - 1.87 e_z) 10^6 \text{ Pa}$$

In indicial form

$$t_i^n = \sigma_{ji} n_j$$

$$t_1^n = \sigma_{11} n_1 + \sigma_{21} n_2 + \sigma_{31} n_3$$

$$= 10^6 \text{ Pa} \left(\frac{5}{2} + \frac{5}{2} + \frac{8}{\sqrt{2}} \right) = 10.66 \times 10^6 \text{ Pa}$$

$$t_2^n = \sigma_{j2} n_j = 10^6 \text{ Pa} \left(\frac{5}{2} + 0 - \frac{7.5}{\sqrt{2}} \right) = -2.8 \times 10^6 \text{ Pa}$$

$$t_3^n = \sigma_{j3} n_j = 10^6 \text{ Pa} \left(\frac{8}{2} - \frac{7.5}{2} - \frac{3}{\sqrt{2}} \right) = -1.87 \times 10^6 \text{ Pa}$$

In Matrix Form

$$\Pi^n = m \cdot \phi = [n]^T [\phi] = \begin{bmatrix} \frac{1}{2} & \frac{1}{2} & \frac{\sqrt{2}}{2} \end{bmatrix} \begin{bmatrix} 5 & 5 & 8 \\ 5 & 0 & -7.5 \\ 8 & -7.5 & -3 \end{bmatrix} 10^6 \text{ Pa}$$

$$= \begin{bmatrix} 10.66 \\ -2.8 \\ -1.87 \end{bmatrix} \times 10^6 \text{ Pa}$$

$$|\Pi^n| = \sqrt{\Pi^n \cdot \Pi^n} = \left(\sqrt{10.66^2 + 2.8^2 + 1.87^2} \right) \times 10^6 \text{ Pa} = \underline{\underline{11.2 \times 10^6 \text{ Pa}}}$$

b) $t_n^n = \sigma_{nn} = \Pi^n \cdot m$

In Dyadic Form $\Pi^n \cdot m = (t_1^n e_1 + t_2^n e_2 + t_3^n e_3) \cdot \left(\frac{1}{2} e_1 + \frac{1}{2} e_2 + \frac{1}{\sqrt{2}} e_3 \right)$

$$= 10^6 \text{ Pa} \left(5.33 - 1.4 - \frac{1.87}{\sqrt{2}} \right) = 2.61 \times 10^6 \text{ Pa}$$

In Indicial form

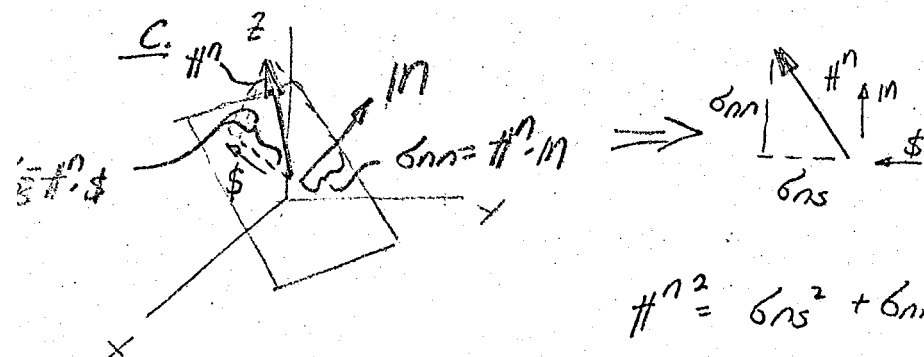
$$\Pi^n \cdot m = t_i^n n_i = \underbrace{t_1^n}_{\frac{1}{2}} \underbrace{n_1}_{\frac{1}{2}} + \underbrace{t_2^n}_{-2.8} \underbrace{n_2}_{\frac{1}{2}} + \underbrace{t_3^n}_{-1.87} \underbrace{n_3}_{\frac{1}{\sqrt{2}}} = 2.61 \times 10^6 \text{ Pa}$$

in matrix form

$$\#^n \cdot M = [\#^n]^T [M]$$

(2)

$$= 10^6 \text{Pa} [10.66 \quad -2.8 \quad -1.87] \begin{bmatrix} 1/2 \\ 1/2 \\ \sqrt{2}/2 \end{bmatrix} = 2.61 \times 10^6 \text{Pa}$$



$$\#^n^2 = \sigma_{ns}^2 + \sigma_{nn}^2 \quad \sigma_{ns} = \sqrt{(\#^n)^2 - \sigma_{nn}^2} = \underline{10.9 \times 10^6 \text{Pa}}$$

If direction were also desired, note vector sum

$$\sigma_{nn} M + \sigma_{ns} \$ = \#^n \quad \text{or} \quad (\#^n \cdot M) M + (\#^n \cdot \$) \$ = \#^n$$

$$10^6 \text{Pa} (2.61) \left(\frac{1}{2} e_1 + \frac{1}{2} e_2 + \frac{\sqrt{2}}{2} e_3 \right) + 10.9 \times 10^6 \text{Pa} (S_1 e_1 + S_2 e_2 + S_3 e_3) = 10^6 \text{Pa} (10.66 e_1 - 2.8 e_2 - 1.87 e_3)$$

$$S_1 = \frac{1}{10.9} (10.66 - \frac{2.61}{2}) = +.858$$

$$S_2 = \frac{1}{10.9} (-2.8 - \frac{2.61}{2}) = -.376$$

$$S_3 = \frac{1}{10.9} (-1.87 - \frac{2.61}{\sqrt{2}}) = -.341$$

$$\$ = +.858 e_1 - .376 e_2 - .341 e_3$$

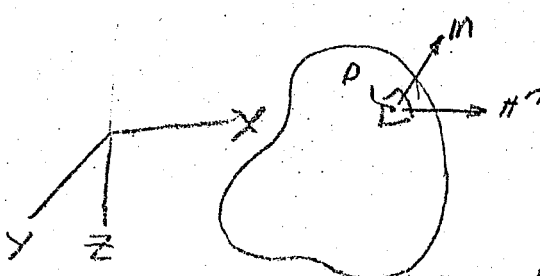
2.

Given $m = l e_x + m e_y + n e_z$,

assume $\{m, n\}$
do not = 0

$$\#^n = a e_x$$

$$\text{and } \Phi = \begin{bmatrix} \sigma_{xx} & 0 & 0 \\ 0 & \sigma_{yy} & \sigma_{yz} \\ 0 & \sigma_{zy} & 0 \end{bmatrix}$$



Dyadic Form: $\#^n = m \cdot \Phi = (l e_x + m e_y + n e_z) \cdot (\sigma_{xx} e_x e_x + \sigma_{yy} e_y e_y + \sigma_{yz} e_y e_z + \sigma_{zy} e_z e_y)$

$$= l \sigma_{xx} e_x + (m \sigma_{yy} + n \sigma_{yz}) e_z + m \sigma_{yz} e_y = a e_x$$

$$\therefore l \sigma_{xx} = a, \quad \sigma_{xx} = a/l; \quad m \sigma_{yz} = 0 \Rightarrow \sigma_{yz} = 0$$

$$m \sigma_{yy} + n \sigma_{yz} = 0 \Rightarrow \sigma_{yy} = 0$$

Indexial Form:

$$t_i^n = \sigma_{ij} n_j = l \sigma_{ii} = a$$

$$t_i^n = \sigma_{ij} n_j \quad (\text{noting } x=x_1, y=x_2, z=x_3)$$

$$t_3^n = \sigma_{ij} n_j = m \sigma_{23} = 0$$

$$t_2^n = \sigma_{ij} n_j = m \sigma_{22} + n \sigma_{23}$$

in matrix form, $M = \begin{bmatrix} l \\ m \\ n \end{bmatrix}$

$H^n = [a \ 0 \ 0]$ and

$\Phi = \begin{bmatrix} \sigma_{xx} & 0 & 0 \\ 0 & \sigma_{yy} & \sigma_{yz} \\ 0 & \sigma_{yz} & 0 \end{bmatrix}$

$H^n = M \cdot \Phi = M^T \Phi = \begin{bmatrix} l & m & n \end{bmatrix} \begin{bmatrix} \sigma_{xx} & 0 & 0 \\ 0 & \sigma_{yy} & \sigma_{yz} \\ 0 & \sigma_{yz} & 0 \end{bmatrix} = \begin{bmatrix} l\sigma_{xx} & m\sigma_{yy} & n\sigma_{yz} \end{bmatrix}$

$= [a \ 0 \ 0]$

$\Rightarrow \sigma_{xx} = l \quad \sigma_{yy} = \sigma_{yz} = 0$

3.) $\det \begin{vmatrix} \sigma_{xx} - \lambda & \sigma_{xy} & \sigma_{xz} \\ \sigma_{yx} & \sigma_{yy} - \lambda & \sigma_{yz} \\ \sigma_{zx} & \sigma_{zy} & \sigma_{zz} - \lambda \end{vmatrix} = 0$

$= (\sigma_{xx} - \lambda)[(\sigma_{yy} - \lambda)(\sigma_{zz} - \lambda) - \sigma_{yz}\sigma_{zy}] + \sigma_{xz}[\sigma_{yx}\sigma_{zy} - \sigma_{zx}(\sigma_{yy} - \lambda)] - \sigma_{xy}[\sigma_{yx}(\sigma_{zz} - \lambda) - \sigma_{zx}\sigma_{yz}]$ II

multiplying out, $\lambda^3 + \lambda^2 \underbrace{(\sigma_{xx} + \sigma_{yy} + \sigma_{zz})}_I + \lambda \underbrace{(\sigma_{xx}\sigma_{yy} + \sigma_{yy}\sigma_{zz} + \sigma_{xx}\sigma_{zz} - \sigma_{xy}^2 - \sigma_{yz}^2 - \sigma_{zx}^2)}_{III} + \underbrace{(\sigma_{xx}\sigma_{yy}\sigma_{zz} + 2\sigma_{xy}\sigma_{yz}\sigma_{zx} - \sigma_{xx}\sigma_{yz}^2 - \sigma_{yy}\sigma_{zx}^2 - \sigma_{xx}^2\sigma_{yz})}_{IV} = 0$

4) Given: a) for arbitrary unit vectors a, b, c , normal to planes A, B, C , respectively,

or $\Phi: \Phi = \sigma, \Phi, \dots$ $\mu, \kappa = \sigma, \mu, \dots$ $\mu, \kappa = \sigma, \mu, \dots$

or in indicial form

$$A \cdot B = a_k \phi_k = \delta_{ij} \phi_i \cdot \phi_j = a_k \delta_{ij} \delta_{ki} \phi_j$$

$$= a_k \delta_{kj} \phi_j = \delta_j a_j \phi_j$$

$$\Rightarrow a_k \delta_{kj} = a_j \Rightarrow$$

$$\text{similarly, } b_k \delta_{kj} = b_2 b_j \Rightarrow (\delta_{kj} - \delta_{kj} \delta_1) a_k = 0 \quad (1)$$

$$c_k \delta_{kj} = \delta_3 c_j \Rightarrow (\delta_{kj} - \delta_{kj} \delta_2) b_k = 0 \quad (2)$$

$$\Rightarrow (\delta_{kj} - \delta_{kj} \delta_3) c_k = 0 \quad (3)$$

multiply (1) by b_j , (2) by a_j , and subtract

$$\Rightarrow (\delta_{kj} - \delta_{kj} \delta_1) a_k b_j - (\delta_{kj} - \delta_{kj} \delta_2) b_k a_j = 0$$

$$\Rightarrow (\delta_2 - \delta_1) \delta_{jk} a_k b_j = 0 \quad (\text{using } \delta_{jk} = \delta_{kj})$$

$$\Rightarrow (\delta_2 - \delta_1) a_j b_j = 0$$

A, B are arbitrary $\therefore A \cdot B = a_j b_j$ is not necessarily zero

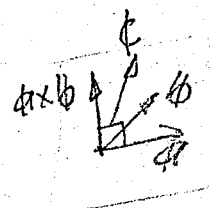
$$\therefore \delta_2 = \delta_1$$

$$\text{similarly } \delta_3 = \delta_2 \Rightarrow \delta_1 = \delta_2 = \delta_3 = \delta \quad (4)$$

(b) ϕ is not in plane formed by $A \neq B$

$$\therefore (A \times B) \cdot C \neq 0$$

$$\Rightarrow \begin{vmatrix} a_1 & a_2 & a_3 \\ b_1 & b_2 & b_3 \\ c_1 & c_2 & c_3 \end{vmatrix} \neq 0 \quad (5)$$



subst from (4) back into (1), (2), (3)

$$\Rightarrow (\delta_{kj} - \delta_{kj} \delta) a_k = 0 \quad (6)$$

$$(\delta_{kj} - \delta_{kj} \delta) b_k = 0 \quad (7)$$

$$(\delta_{kj} - \delta_{kj} \delta) c_k = 0 \quad (8)$$

(5)

For $j=1, 16, 7, 12$ become

$$(\sigma_{11}-6) a_1 + \sigma_{12} a_2 + \sigma_{13} a_3 = 0$$

$$(\sigma_{11}-6) b_1 + \sigma_{12} b_2 + \sigma_{13} b_3 = 0$$

$$(\sigma_{11}-6) c_1 + \sigma_{12} c_2 + \sigma_{13} c_3 = 0$$

$$\text{or } \begin{bmatrix} a_1 & a_2 & a_3 \\ b_1 & b_2 & b_3 \\ c_1 & c_2 & c_3 \end{bmatrix} \begin{bmatrix} \sigma_{11}-6 \\ \sigma_{12} \\ \sigma_{13} \end{bmatrix} = \begin{bmatrix} 0 \\ 0 \\ 0 \end{bmatrix} \quad (7)$$

3 homogeneous eqns. Nontrivial solution only if

$$\det \begin{vmatrix} a_1 & a_2 & a_3 \\ b_1 & b_2 & b_3 \\ c_1 & c_2 & c_3 \end{vmatrix} = 0 \quad \text{but this is not true by (5)}$$

$\therefore$ only solution to (7) is the "trivial" solution

$$\sigma_{11}-6=0 \Rightarrow \sigma_{11}=6 \quad \sigma_{12}=0 \quad \sigma_{13}=0$$

$$\text{similarly for } j=2 \text{ \& } j=3 \Rightarrow \sigma_{22}=\sigma_{33}=6, \sigma_{23}=0$$

Equation of motion

$$\text{In general } \sigma_{ij,i} + \rho f_j = \rho \ddot{u}_j \quad (8)$$

$$\text{here } \sigma_{ij} = \sigma \delta_{ij}$$

$$\text{into (8)} \Rightarrow (\sigma \delta_{ij})_{,i} + \rho f_j = \rho \ddot{u}_j$$

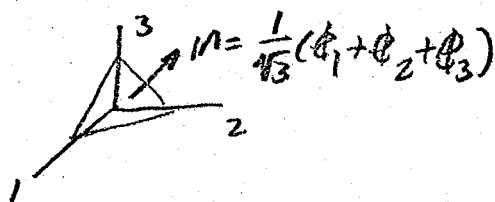
$$\sigma_{,i} \delta_{ij} + \cancel{\sigma \delta_{ij,i}} + \rho f_j = \rho \ddot{u}_j$$

$$\boxed{\sigma_{,i} + \rho f_j = \rho \ddot{u}_j} \quad \text{which are the Euler Equations}$$

5. The equation for a unit vector with like orientation with respect to the three principal directions is

$$m = \pm \frac{1}{\sqrt{3}} (\phi_1 + \phi_2 + \phi_3)$$

Choosing the plus sign places this vector in the first "octant"



$$\#^n = m \cdot \phi$$

$$= \frac{1}{\sqrt{3}} (\phi_1 + \phi_2 + \phi_3) \cdot (b_1 \phi_1 \phi_1 + b_2 \phi_2 \phi_2 + b_3 \phi_3 \phi_3)$$

$$= \frac{1}{\sqrt{3}} (b_1 \phi_1 + b_2 \phi_2 + b_3 \phi_3)$$

$$b_{oct} = b_{mm} = m \cdot \phi \cdot m = \frac{1}{\sqrt{3}} (b_1 \phi_1 + b_2 \phi_2 + b_3 \phi_3) \cdot \frac{1}{\sqrt{3}} (\phi_1 + \phi_2 + \phi_3)$$

$$= \frac{1}{3} (b_1 + b_2 + b_3) \quad (a)$$

noting that $I = b_{11} + b_{22} + b_{33} = b_{KK}$ is invariant under rotation of coordinates,

(a) is equivalent to $b_{oct} = \frac{1}{3} (b_{xx} + b_{yy} + b_{zz})$

$$T_{oct}^2 = b_{ms}^2 = \#^n{}^2 - (b_{oct})^2$$

$$= \frac{1}{3} (b_1^2 + b_2^2 + b_3^2) - \frac{1}{9} (b_1 + b_2 + b_3)^2$$

$$= \frac{1}{9} (3b_1^2 + 3b_2^2 + 3b_3^2 - b_1^2 - b_2^2 - b_3^2 - 2b_1b_2 - 2b_2b_3 - 2b_1b_3)$$

$$= \frac{1}{9} [(b_1 - b_2)^2 + (b_1 - b_3)^2 + (b_2 - b_3)^2]$$

$$T_{oct} = \frac{1}{3} [(b_1 - b_2)^2 + (b_1 - b_3)^2 + (b_2 - b_3)^2]^{1/2}; \text{ to express in terms of } x, y, z \text{ coord}$$

examine invariants: characteristic equation

$$\lambda^3 - \dots$$

finding roots of this is equivalent to factoring it:

$$(6-6_1)(6-6_2)(6-6_3)=0 \quad (2)$$

expanding, $6^3 - 6^2(6_1+6_2+6_3) + 6(6_1 6_2 + 6_1 6_3 + 6_2 6_3) - 6_1 6_2 6_3 = 0$

comparing (3) & (1),

$$I_1 = 6_1 + 6_2 + 6_3, \quad I_2 = 6_2 6_1 + 6_2 6_3 + 6_1 6_3, \quad I_3 = 6_1 6_2 6_3$$

using (4), it can be verified that

$$(6_1 - 6_3)^2 + (6_2 - 6_3)^2 + (6_1 - 6_2)^2 = 2I^2 - 6II$$

now substituting

$$I = 6_x + 6_y + 6_z \quad \& \quad II = 6_x 6_y + 6_y 6_z + 6_x 6_z - 6_x y^2 - 6_y z^2 - 6_x z^2$$

into $T_{oct} = \sqrt{2I^2 - 6II}$

yields $T_{oct} = \sqrt{(6_{xx} - 6_{yy})^2 + (6_{yy} - 6_{zz})^2 + (6_{zz} - 6_{xx})^2 + 6(6_x^2 + 6_y^2 + 6_z^2)}$

6)

by definition, $\phi_{spherical} = \begin{bmatrix} 6 & & \\ & 6 & \\ & & 6 \end{bmatrix} = \sigma \delta_{ij} \phi_i \phi_j$

where $\sigma = \frac{\sigma_{KK}}{3}$

$$[\sigma] = [\sigma_{dev}] + [\sigma_{sper}] = \begin{bmatrix} 6_{11} - 6 & 6_{12} & 6_{13} \\ 6_{21} & 6_{22} - 6 & 6_{23} \\ 6_{31} & 6_{32} & 6_{33} - 6 \end{bmatrix} + \begin{bmatrix} 6 & & \\ & 6 & \\ & & 6 \end{bmatrix}$$

or $\sigma_{ij}^{dev} = \sigma_{ij} - \frac{\sigma_{KK}}{3} \delta_{ij}$

invariants of σ_{ij}^{dev} are determined by solving

eigenvalue problem

$$\det \begin{vmatrix} \sigma_{11}^{dev} - \lambda & \sigma_{12}^{dev} & \sigma_{13}^{dev} \\ \sigma_{21}^{dev} & \sigma_{22}^{dev} - \lambda & \sigma_{23}^{dev} \\ \sigma_{31}^{dev} & \sigma_{32}^{dev} & \sigma_{33}^{dev} - \lambda \end{vmatrix} = 0$$

∴ expanding this leads to a cubic in λ =

(8)

$$\lambda^3 - I_0 \lambda^2 + \Pi_0 \lambda - \text{III}_0 = 0$$

where $I_0 = 0$

$$\Pi_0 = \Pi - \frac{1}{3} I^2$$

$$\text{III}_0 = \text{III} - \frac{1}{3} I \left(\Pi - \frac{2}{9} I^2 \right)$$

5

DEPARTMENT OF APPLIED MECHANICS
STANFORD UNIVERSITY

238A Theory of Elasticity

Autumn 1978

27 Nov 78

Problem Set No. 3

1. Consider the displacement vector

$$\begin{aligned} U &= (dx + 6y + 5z) \hat{e}_x \\ &+ (ax + ey + 3z) \hat{e}_y \\ &+ (bx + cy + fz) \hat{e}_z \end{aligned}$$

and determine the constants a, b, c, d, e and f such that U represents a rigid body rotation only.

2. Assume that the components of strain ϵ_{ij} are expressed in terms of a single function $\phi(x, y)$ as follows:

$$\epsilon_{xx} = \frac{\partial^2 \phi}{\partial y^2}$$

$$\epsilon_{zz} = 0$$

$$\epsilon_{yy} = \frac{\partial^2 \phi}{\partial x^2}$$

$$\epsilon_{yz} = 0$$

$$\epsilon_{xy} = -\frac{\partial^2 \phi}{\partial x \partial y}$$

$$\epsilon_{zx} = 0$$

Can the function ϕ be considered arbitrary or does it have to satisfy some conditions to insure single-valuedness of displacement?

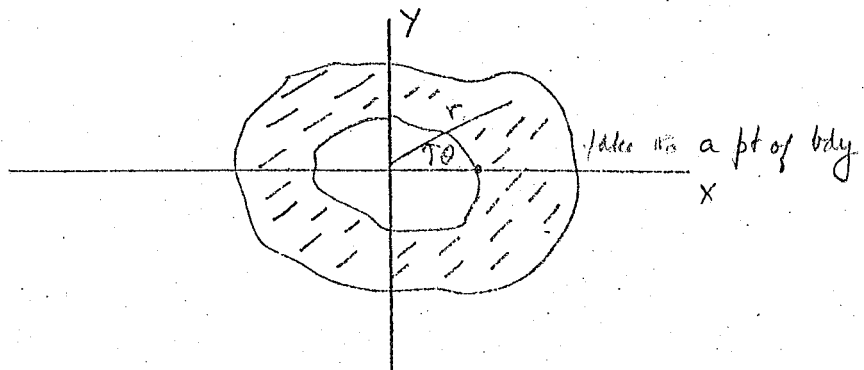
3. Consider the doubly connected plane region and assume the components of strain to be

$$\epsilon_{yz} = \frac{kx}{x^2 + y^2} = \frac{k \cos \phi}{r}$$

$$\text{All other } \epsilon_{ij} = 0$$

$$\epsilon_{zx} = \frac{-ky}{x^2 + y^2} = -\frac{k \sin \phi}{r}$$

Determine the conditions for single-valued displacements and determine k for single-valued displacements, for the special case of the inner boundary being a circle of radius a .



$$1. U = \overbrace{(dx+by+5z)}^{u_x} e_x + \overbrace{(ax+ey+3z)}^{u_y} e_y + \overbrace{(bx+cy+fz)}^{u_z} e_z$$

$$\epsilon_{ij} = \frac{1}{2}(u_{i,j} + u_{j,i})$$

$$\epsilon_{xx} = \frac{1}{2}(u_{x,x} + u_{x,x})$$

$$\omega_{ij} = \frac{1}{2}(u_{j,i} - u_{i,j})$$

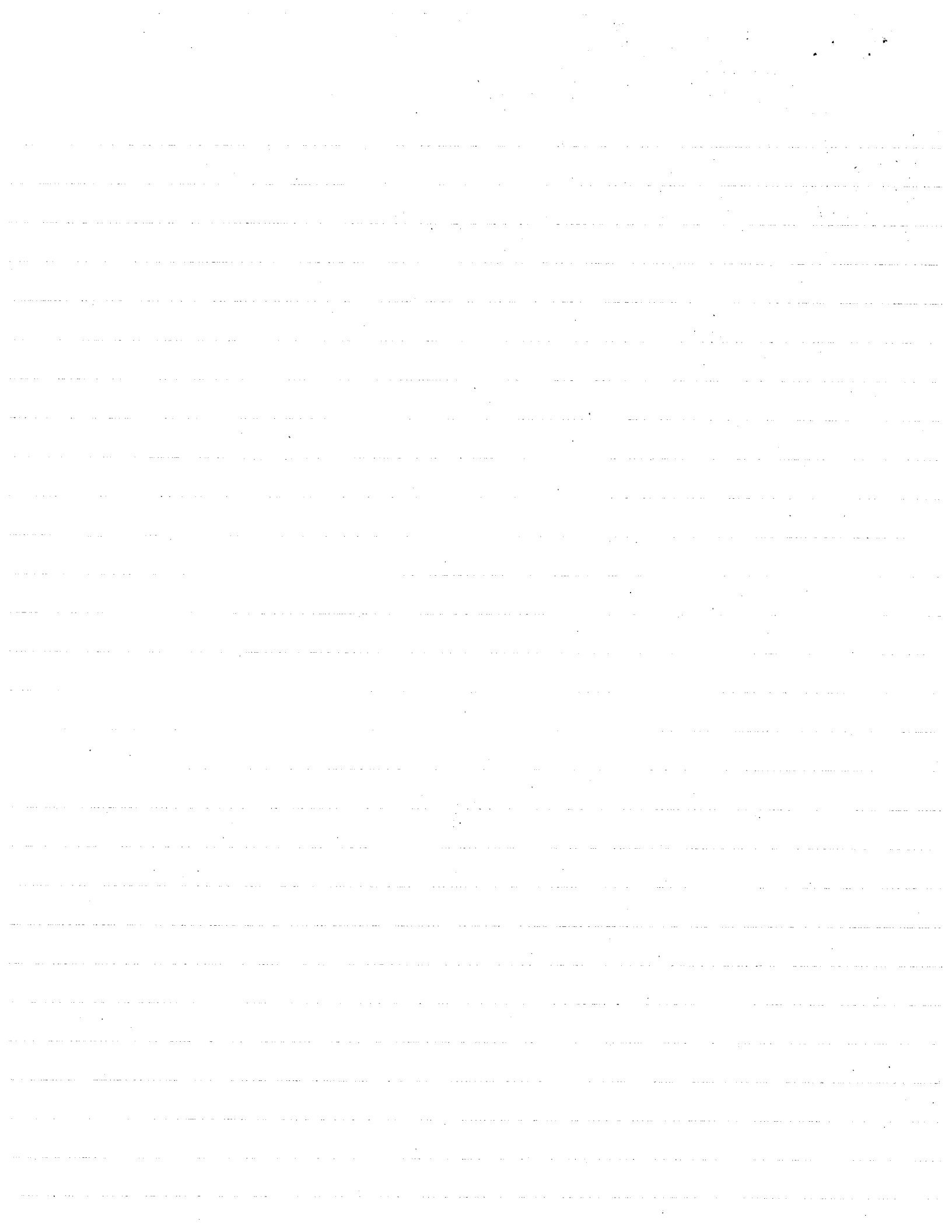
$$\epsilon_{ij} = \begin{pmatrix} d & \frac{b+a}{2} & \frac{5+b}{2} \\ \frac{a+b}{2} & e & \frac{3+c}{2} \\ \frac{b+5}{2} & \frac{c+3}{2} & f \end{pmatrix}$$

$$\omega_{ij} = \begin{pmatrix} 0 & \frac{a-b}{2} & \frac{b-5}{2} \\ \frac{b-a}{2} & 0 & \frac{c-3}{2} \\ \frac{5-b}{2} & \frac{3-c}{2} & 0 \end{pmatrix}$$

for rigid body rotation only the extensional strains $\epsilon_{ij} = 0$

$$\epsilon_{ij} = 0 \Rightarrow d=e=f=0, a=-b, b=-5, c=-3$$

$$\therefore \begin{pmatrix} 0 & -b & -5 \\ b & 0 & -3 \\ 5 & 3 & 0 \end{pmatrix} = \omega_{ij}$$



2. Must satisfy compatibility conditions namely that $\nabla \times \phi \times \nabla = 0$

$$\psi_{xx} = 2 \cdot 0 - \phi_{xxzz} - 0 = 0 \text{ since } \phi_{xxzz} = \phi_{zzxx} = 0$$

$$\psi_{yy} = 2 \cdot 0 - \phi_{yyzz} - 0 = 0$$

$$\psi_{zz} = 2(-\phi_{xxyy}) - \phi_{yyyy} - \phi_{xxxx} = \Delta^2 \phi = 0 \text{ for compat.}$$

$$\psi_{yz} = \phi_{yyyz} - \frac{\partial}{\partial x} (0 + 0 - \phi_{xyy}) = 0 \text{ since } \phi_z = 0$$

$$\psi_{xz} = \phi_{xxxz} - \frac{\partial}{\partial y} (0 - \phi_{xyy} + 0) = 0 \text{ since } \phi_z = 0$$

$$\psi_{xy} = 0 - \frac{\partial}{\partial z} (\phi_{xyy} + 0 + 0) = 0 \text{ since } \phi_z = 0$$

3. for single valuedness of displacements in a simply connected domain $\Delta^2 \phi = \nabla^4 \phi = 0$

$$\phi \times \nabla = \epsilon_{il,k} \epsilon_{lkj} \phi_i \phi_j \quad \text{only for } \epsilon_{123}^{\begin{smallmatrix} +1 \\ -1 \end{smallmatrix}}, \epsilon_{231}^{\begin{smallmatrix} +1 \\ -1 \end{smallmatrix}}, \epsilon_{312}^{\begin{smallmatrix} +1 \\ -1 \end{smallmatrix}} \text{ and } \epsilon_{321}^{\begin{smallmatrix} +1 \\ -1 \end{smallmatrix}}, \epsilon_{213}^{\begin{smallmatrix} +1 \\ -1 \end{smallmatrix}}, \epsilon_{132}^{\begin{smallmatrix} +1 \\ -1 \end{smallmatrix}}$$

$$= \epsilon_{11,2} \phi_1 \phi_3 + \epsilon_{12,3} \phi_1 \phi_1 + \epsilon_{13,1} \phi_1 \phi_2 - \epsilon_{13,2} \phi_1 \phi_1 - \epsilon_{12,1} \phi_1 \phi_3 - \epsilon_{11,3} \phi_1 \phi_2$$

only $\epsilon_{23}, \epsilon_{31}$ are not zero

$$\psi_{xx} = 0 \text{ since } \epsilon_{yz} = f(x,y) \text{ only} \quad \frac{\partial^2 \epsilon_{yz}}{\partial y \partial z} = 0$$

$$\psi_{yy} = 0 \text{ since } \epsilon_{xz} = g(x,y) \text{ only} \quad \frac{\partial^2 \epsilon_{xz}}{\partial x \partial z} = 0$$

$$\psi_{zz} = 0$$

$$\psi_{yz} = \frac{\partial^2}{\partial x^2} \epsilon_{yz} - \frac{\partial^2 \epsilon_{zx}}{\partial x \partial y} = 0$$

$$\psi_{zx} = \frac{\partial^2}{\partial y^2} \epsilon_{zx} - \frac{\partial^2 \epsilon_{yz}}{\partial x \partial y} = 0$$

$$\psi_{xy} = 0 \text{ same as (1) (2)}$$

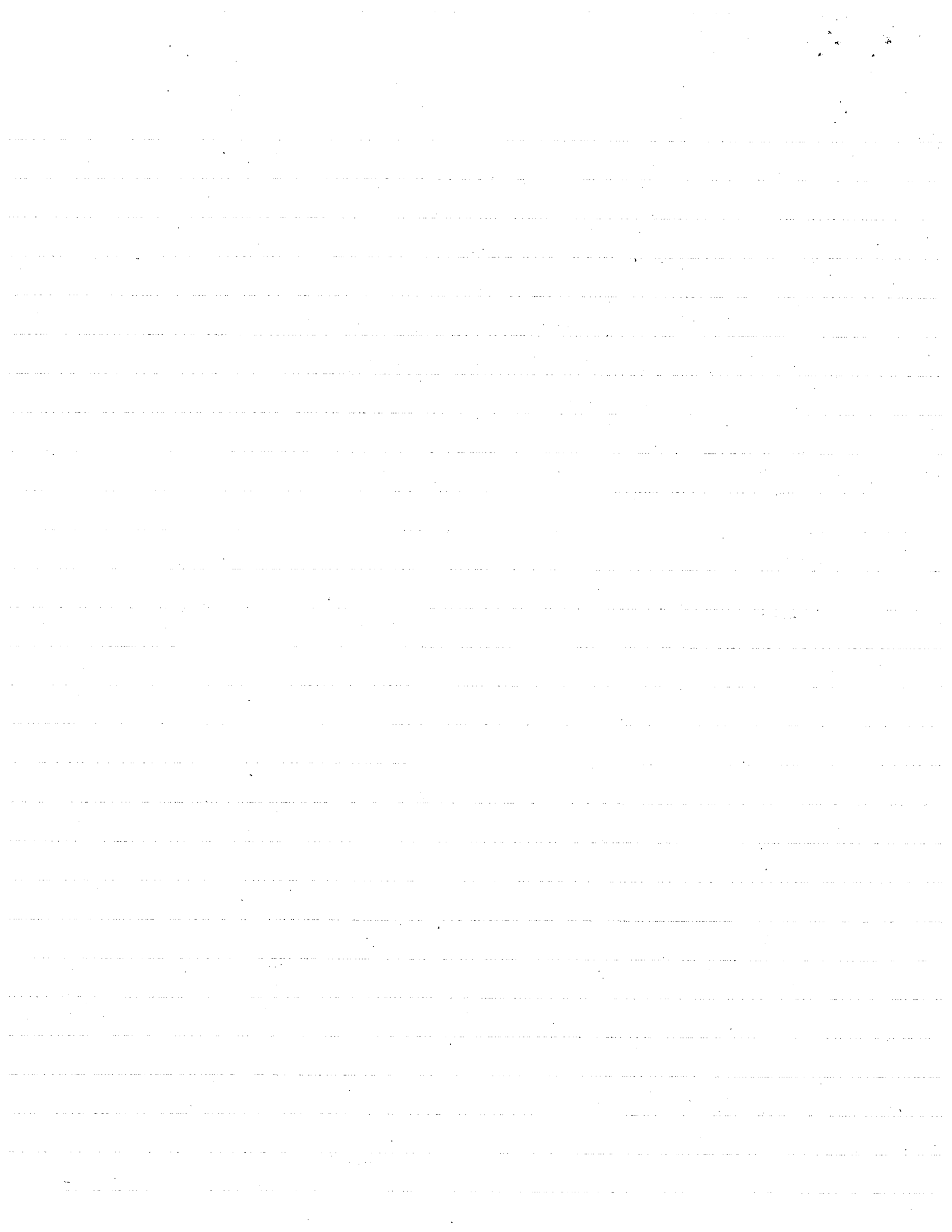
$$\epsilon_{yz,x} = \frac{(x^2+y^2)k - kx \cdot 2x}{(x^2+y^2)^2} = \frac{k(y^2-x^2)}{(x^2+y^2)^2} = \frac{k}{r^4} [r^2 \sin^2 \phi - r^2 \cos^2 \phi] = \frac{k}{r^2} (1 - 2\cos^2 \phi)$$

$$\epsilon_{yz,xx} = \frac{(x^2+y^2)^{-2} (-2kx) - 2k(y^2-x^2)(x^2+y^2)^{-3} \cdot 2x}{(x^2+y^2)^4} = (x^2+y^2)^{-4} (-2kx) \{ (x^2+y^2) + 2(y^2-x^2) \}$$

$$= \frac{(3y^2-x^2)(-2kx)}{(x^2+y^2)^3}$$

$$\epsilon_{zx,y} = \frac{(x^2+y^2)(-k) + 2ky^2}{(x^2+y^2)^2} = \frac{k(y^2-x^2)}{(x^2+y^2)^2}$$

$$\epsilon_{zx,x} = \frac{(x^2+y^2)^{-1} \cdot 0 + ky(2x)}{(x^2+y^2)^2} = \frac{2kxy}{(x^2+y^2)^2} = -\frac{k \cos 2\phi}{r^2}$$



$$\epsilon_{zx,xy} = \frac{(x^2+y^2)^2}{(x^2+y^2)^4} 2kx = 2kyx \cdot 2(x^2+y^2) \cdot 2y = \frac{(x^2+y^2) \cdot 2kx [(x^2+y^2) - 4y^2]}{(x^2+y^2)^4}$$

$$\therefore \epsilon_{yz,xx} - \epsilon_{zx,xy} = \frac{(3y^2-x^2)(-2kx)}{(x^2+y^2)^3} - \frac{(-2kx)(-x^2+3y^2)}{(x^2+y^2)^3} = 0$$

since $-\epsilon_{zx}$ can be obtained by replacing the x by y in ϵ_{yz} then

$-\epsilon_{zx,yy}$ can be obtained by replacing the x by y in $\epsilon_{yz,xx}$.

similarly $-\epsilon_{yz}$ can be obtained by replacing the y by x is $+\epsilon_{zx}$ then

$-\epsilon_{yz,xy}$ can be obtained by replacing the y by x is $+\epsilon_{zx,xy}$.

Hence $\epsilon_{yz,xy} - (-\epsilon_{zx,yy}) = \epsilon_{zx,yy} - \epsilon_{yz,xx} = 0$

$$\begin{aligned} \text{div } \phi \times \nabla &= (dx_i e_j) \cdot [(\epsilon_{i1,2} e_i e_3 - \epsilon_{i2,1} e_i e_3) + (\epsilon_{i2,3} e_i e_1 - \epsilon_{i3,2} e_i e_1) + (\epsilon_{i3,1} e_i e_2 - \epsilon_{i1,3} e_i e_2)] \\ &= dx_j \cdot (\epsilon_{i1,2} - \epsilon_{i2,1}) \delta_{ij} e_3 + dx_j \cdot (\epsilon_{i2,3} - \epsilon_{i3,2}) \delta_{ij} e_1 + dx_j \cdot (\epsilon_{i3,1} - \epsilon_{i1,3}) \delta_{ij} e_2 \end{aligned}$$

$$\Rightarrow \textcircled{1} \oint dx_i \cdot (\epsilon_{i1,2} - \epsilon_{i2,1}) = 0 \Rightarrow \int dx_1 (\epsilon_{11,2} - \epsilon_{12,1}) + dx_2 (\epsilon_{21,2} - \epsilon_{22,1}) + dx_3 (\epsilon_{31,2} - \epsilon_{32,1}) = 0$$

$$\textcircled{2} \oint dx_i \cdot (\epsilon_{i2,3} - \epsilon_{i3,2}) = 0 \Rightarrow \int dx_1 (\epsilon_{12,3} - \epsilon_{13,2}) + dx_2 (\epsilon_{22,3} - \epsilon_{23,2}) + dx_3 (\epsilon_{32,3} - \epsilon_{33,2}) = 0$$

$$\textcircled{3} \oint dx_i \cdot (\epsilon_{i3,1} - \epsilon_{i1,3}) = 0 \Rightarrow \int dx_1 (\epsilon_{13,1} - \epsilon_{11,3}) + dx_2 (\epsilon_{23,1} - \epsilon_{21,3}) + dx_3 (\epsilon_{33,1} - \epsilon_{31,3}) = 0$$

Now if we are integrating in the plane $dx_3 = 0 \Rightarrow \textcircled{1}$ is satisfied

$$\text{now } dx_1 = -r \sin \phi d\phi \quad dx_2 = r \cos \phi d\phi$$

$$\textcircled{2} \Rightarrow \int -\epsilon_{13,2} dx_1 - \epsilon_{23,2} dx_2 = 0 = \int -\epsilon_{13,2} (-\sin \phi \cdot r d\phi) - \epsilon_{23,2} (r \cos \phi d\phi) = 0$$

$$\textcircled{3} \Rightarrow \int \epsilon_{13,1} dx_1 + \epsilon_{23,1} dx_2 = 0 = \int +\epsilon_{13,1} (-\sin \phi r d\phi) + \epsilon_{23,1} (r \cos \phi d\phi) = 0$$

$$\textcircled{3} \int \epsilon_{23,1} (r \cos \phi d\phi) = \int \frac{-k}{r^2} (\cos 2\phi) (r \cos \phi d\phi) = \frac{k}{a} \int \cos 2\phi \cos \phi d\phi = 0$$

$$\textcircled{2} \int \epsilon_{13,2} (r \sin \phi d\phi) = \int \frac{-k}{a} \int_0^{2\pi} \cos 2\phi \sin \phi d\phi = 0$$



$$\oint_{C_1} [\phi + (r - r_0) \times (\nabla \times \phi)] \cdot dr = 0 \quad \text{let } r_0 = 0$$

$$p = \nabla \times \phi = \epsilon_{ljk} \epsilon_{kli} \phi_i \phi_j$$

$$\epsilon_{23} = \epsilon_{yz} = \frac{kx}{x^2 + y^2} = \frac{k \cos \phi}{r} \quad \epsilon_i = 0 \text{ all other}$$

$$r \times p = r_i p_j \epsilon_{min} \phi_n$$

$$\epsilon_{31} = \epsilon_{zx} = \frac{-ky}{x^2 + y^2} = \frac{-k \sin \phi}{r}$$

$$\epsilon_{kli} \neq 0 \text{ for } kli = 123, 231, 312 \text{ \& } 321, 213, 132$$

$$\therefore \epsilon_{2j,1} \phi_j + \epsilon_{3j,2} \phi_j + \epsilon_{1j,3} \phi_j - \epsilon_{2j,3} \phi_j - \epsilon_{1j,2} \phi_j - \epsilon_{3j,1} \phi_j \quad \text{this yields 18 terms.}$$

$$\text{if } j=1,2,3$$

$$\begin{aligned} & \epsilon_{31,2} \phi_1 - \epsilon_{31,1} \phi_2 + \epsilon_{32,2} \phi_1 - \epsilon_{32,1} \phi_2 + \epsilon_{32,3} \phi_3 - \epsilon_{31,3} \phi_2 - \epsilon_{32,3} \phi_1 - \epsilon_{31,2} \phi_3 \\ & \epsilon_{31,2} \phi_1 - \epsilon_{31,1} \phi_2 + \epsilon_{32,2} \phi_1 - \epsilon_{32,1} \phi_2 + \epsilon_{32,3} \phi_3 - \epsilon_{31,3} \phi_2 - \epsilon_{32,3} \phi_1 - \epsilon_{31,2} \phi_3 \end{aligned}$$

Since $\frac{\partial}{\partial z} = 0$ this reduces to 6

$$r \times \nabla \times \phi = x_i \phi_i \times (\nabla \times \phi) = -x_2 \epsilon_{31,2} \phi_3 \phi_1 - x_1 \epsilon_{31,1} \phi_3 \phi_1 - x_2 \epsilon_{32,2} \phi_3 \phi_2 - x_1 \epsilon_{32,1} \phi_3 \phi_2 - x_1 \epsilon_{32,1} \phi_2 \phi_3 - x_2 \epsilon_{31,2} \phi_2 \phi_3$$

$$\oint_{C_1} (\phi + r \times \nabla \times \phi) (dx_j \phi_j) =$$

$$\oint \left[(\epsilon_{31} - x_2 \epsilon_{31,2} - x_1 \epsilon_{31,1}) \phi_3 \phi_1 + (\epsilon_{32} - x_2 \epsilon_{32,2} - x_1 \epsilon_{32,1}) \phi_3 \phi_2 + (\epsilon_{32} + x_1 \epsilon_{32,1} + x_1 \epsilon_{31,2}) \phi_2 \phi_3 + (\epsilon_{31} + x_2 \epsilon_{32,1} - x_2 \epsilon_{31,2}) \phi_1 \phi_3 \right] \cdot dx_i \phi_i = 0 \quad \text{since } x_3 = 0$$

or

$$-r \sin \phi d\phi$$

$$r \cos \phi d\phi$$

$$\textcircled{1} \phi_3 \oint (\epsilon_{31} - x_2 \epsilon_{31,2} - x_1 \epsilon_{31,1}) dx_1 + (\epsilon_{32} - x_2 \epsilon_{32,2} - x_1 \epsilon_{32,1}) dx_2 = 0$$

$$\textcircled{2} \phi_2 \oint (\epsilon_{32} - x_1 \epsilon_{32,1} + x_1 \epsilon_{31,2}) dx_3 = 0 \quad \text{identically}$$

$$\textcircled{3} \phi_1 \oint (\epsilon_{31} + x_2 \epsilon_{32,1} - x_2 \epsilon_{31,2}) dx_3 = 0 \quad \text{identically}$$

$$\oint \epsilon_{31} dx_1 + \oint \epsilon_{32} dx_2 - \left[\oint x_2 \epsilon_{31,2} dx_1 + \oint x_1 \epsilon_{32,1} dx_2 + \oint x_1 \epsilon_{31,1} dx_1 + \oint x_2 \epsilon_{32,2} dx_2 \right]$$

$$= 2 \oint \epsilon_{31} dx_1 + 2 \oint \epsilon_{32} dx_2 - (x_1 \epsilon_{31} + x_2 \epsilon_{32}) \Big|_0^{2\pi} - \left[\oint [(r \sin \phi) \left(\frac{k}{r^2} \cos 2\phi \right) (-r \sin \phi d\phi) + (r \cos \phi) \left(\frac{-k}{r^2} \sin 2\phi \right) (r \cos \phi d\phi)] \right]$$

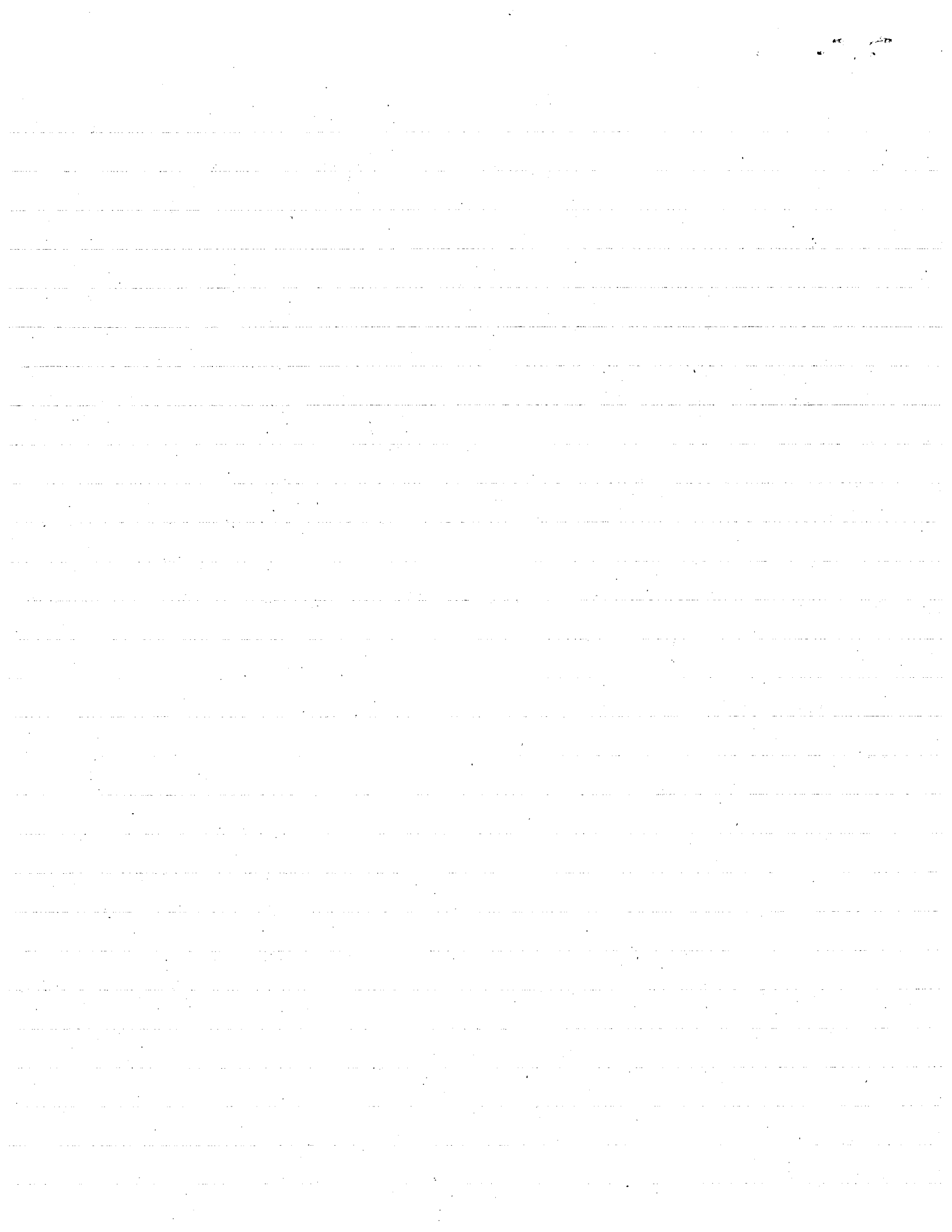
$$\epsilon_{31,2} = \epsilon_{zx,y} = -\frac{k}{r^2} \cos 2\phi$$

$$\epsilon_{23,1} = \epsilon_{yz,x} = -\frac{k}{r^2} \cos 2\phi$$

$$[-k \oint \cos 2\phi d\phi] = -\frac{k}{2} \sin 2\phi \Big|_0^{2\pi} = 0 \quad 2\pi$$

$$= 2 \oint \epsilon_{31} \cdot r \sin \phi d\phi + 2 \oint \epsilon_{32} \cdot r \cos \phi d\phi - \left(-\frac{k}{2} \sin 2\phi + \frac{k}{2} \sin 2\phi \right)$$

$$= +2k \oint \sin^2 \phi d\phi + 2k \oint \cos^2 \phi d\phi = 2k (2\pi) = 4\pi k \quad \rightarrow k=0$$



$$\int X_1 d\epsilon_{31} = \int X_1 \epsilon_{31,1} dx_1 + \int X_1 \epsilon_{31,2} dx_2$$

$$X_1 \epsilon_{31} = \int \epsilon_{31} dx_1$$

$$\text{then } - \int X_1 \epsilon_{31,1} dx_1 = \int X_1 \epsilon_{31,2} dx_2 - \int X_1 d\epsilon_{31}$$

$$- \int X_1 \epsilon_{31,1} dx_1 = \int X_1 \epsilon_{31,2} dx_2 - X_1 \epsilon_{31} + \int \epsilon_{31} dx_1$$

$$- \int X_2 \epsilon_{32,2} dx_2 = \int X_2 \epsilon_{32,1} dx_1 - X_2 \epsilon_{32} + \int \epsilon_{32} dx_2$$

$$\left(\int 2\epsilon_{31} dx_1 - X_2 \epsilon_{31,2} dx_2 \right) + \left(\int X_1 \epsilon_{31,2} dx_2 - X_1 \epsilon_{31} \right) \Big|_0^{2\pi} + \left(\int 2\epsilon_{32} dx_2 - X_1 \epsilon_{32,1} dx_1 \right) - X_2 \epsilon_{32} \Big|_0^{2\pi} + \int X_2 \epsilon_{32,1} dx_1$$

$$\left(\int 2\epsilon_{31} dx_1 + 2\epsilon_{32} dx_2 \right) - \left(\int X_2 \epsilon_{31,2} dx_1 + X_1 \epsilon_{32,1} dx_2 \right) - \left(X_1 \epsilon_{31} + X_2 \epsilon_{32} \right) \Big|_0^{2\pi}$$

$$(r \cos \phi) \left(-\frac{k}{r^2} \cos 2\phi \right) (dr \sin \phi + r \cos \phi d\phi) + (r \sin \phi) \left(-\frac{k}{r^2} \cos 2\phi \right) (dr \cos \phi - r \sin \phi d\phi)$$

$$\int (X_1 \epsilon_{31,2} dx_2 + X_2 \epsilon_{32,1} dx_1)$$

$$\int -\frac{k}{r} \cos 2\phi \sin 2\phi dr + \int -k \cos 2\phi \cos^2 \phi d\phi + k \cos 2\phi \sin^2 \phi d\phi$$

$$-k \int_0^{2\pi} \cos 2\phi \cos 2\phi d\phi = -k\pi$$

$$5\pi k - \pi k = 4\pi k$$

$$\oint -\epsilon_{13,2} dx_1 - \epsilon_{23,2} dx_2$$

$$d\epsilon_{23} = \epsilon_{23,1} dx_1 + \epsilon_{23,2} dx_2$$

$$-\int d\epsilon_{23} + \int \epsilon_{23,1} dx_1 = -\int \epsilon_{23,2} dx_2$$

$$\int -\epsilon_{13,2} dx_1 + \epsilon_{23,1} dx_1 = \epsilon_{23} \Big|_0^{2\pi}$$

$$\int (-\epsilon_{31,2} + \epsilon_{32,1}) dx_1 = \epsilon_{23} \Big|_0^{2\pi}$$

$$\oint \varepsilon_{13,1} dx_1 = \oint d\varepsilon_{13} - \oint \varepsilon_{13,2} dx_2$$

$$d\varepsilon_{13} = \varepsilon_{13,1} dx_1 + \varepsilon_{13,2} dx_2$$

$$d\varepsilon_{13} - \varepsilon_{13,2} dx_2$$

$$x_2^B (-\varepsilon_{31,2} + \varepsilon_{32,1}) dx_1 + x_1 (-\varepsilon_{32,1} + \varepsilon_{31,2}) dx_2$$

Prob. Set. No. 3 Solution

11-14-78

1. $u_x = dx + by + 5z$ $u_y = ax + cy + 3z$ $u_z = bx + cy + fz$

For Rigid body ^{rotation} displacement only, all strain components = 0

$$\epsilon_{xx} = \frac{\partial u_x}{\partial x} = 0 \Rightarrow d = 0$$

$$\epsilon_{yy} = \frac{\partial u_y}{\partial y} = 0 \Rightarrow c = 0$$

$$\epsilon_{zz} = \frac{\partial u_z}{\partial z} = 0 \Rightarrow f = 0$$

$$\epsilon_{xy} = \frac{1}{2} \left(\frac{\partial u_x}{\partial y} + \frac{\partial u_y}{\partial x} \right) = \frac{1}{2} (b + a) = 0 \Rightarrow a = -b$$

$$\epsilon_{xz} = \frac{1}{2} \left(\frac{\partial u_x}{\partial z} + \frac{\partial u_z}{\partial x} \right) = \frac{1}{2} (5 + b) = 0 \Rightarrow b = -5$$

$$\epsilon_{yz} = \frac{1}{2} \left(\frac{\partial u_y}{\partial z} + \frac{\partial u_z}{\partial y} \right) = \frac{1}{2} (3 + c) = 0 \Rightarrow c = -3$$

$$u = (6y + 5z) \phi_x + (-6x + 3z) \phi_y + (-5x - 3y) \phi_z$$

There are no constant terms in this displacement so it represents rotation only, no translation

components of rotation:

(Rotation about X) $\omega_{yz} = \left(\frac{\partial u_y}{\partial z} - \frac{\partial u_z}{\partial y} \right) = 6$

(Rotation about Y) $\omega_{zx} = \left(\frac{\partial u_z}{\partial x} - \frac{\partial u_x}{\partial z} \right) = -10$

(Rotation about Z) $\omega_{xy} = \left(\frac{\partial u_x}{\partial y} - \frac{\partial u_y}{\partial x} \right) = 12$

Motion is sum of 3 simple rotations about X_1, X_2, X_3

2. Assume simply connected region. Check compatibility

5 out of 6 equations trivially satisfied

6th: $\psi_{zz} = 0 = \frac{2 \partial^2 \epsilon_{xy}}{\partial x \partial y} - \frac{\partial^2 \epsilon_{xx}}{\partial y^2} - \frac{\partial^2 \epsilon_{yy}}{\partial x^2}$

$$\epsilon_{xx} = \frac{\partial^2 \phi}{\partial y^2} \quad \frac{\partial \epsilon_{xx}}{\partial y} = \frac{\partial^3 \phi}{\partial y^3} \quad \frac{\partial^2 \epsilon_{xx}}{\partial y^2} = \frac{\partial^4 \phi}{\partial y^4}$$

$$\epsilon_{yy} = \frac{\partial^2 \phi}{\partial x^2} \quad \frac{\partial \epsilon_{yy}}{\partial x} = \frac{\partial^3 \phi}{\partial x^3} \quad \frac{\partial^2 \epsilon_{yy}}{\partial x^2} = \frac{\partial^4 \phi}{\partial x^4}$$

Prob. Set No. 3 Solution

$$\epsilon_{xy} = -\frac{\partial^2 \phi}{\partial x \partial y} \quad \frac{\partial \epsilon_{xy}}{\partial x} = -\frac{\partial^3 \phi}{\partial x^2 \partial y} \quad \frac{\partial^2 \epsilon_{xy}}{\partial x \partial y} = -\frac{\partial^4 \phi}{\partial x^2 \partial y^2}$$

$$0 = \frac{\partial^4 \phi}{\partial x^4} + 2 \frac{\partial^4 \phi}{\partial x^2 \partial y^2} + \frac{\partial^4 \phi}{\partial y^4} \quad (1)$$

using the 2 dimensional Laplacian operator

$$\nabla^2 = \nabla \cdot \nabla = \left(\frac{\partial^2}{\partial x^2} + \frac{\partial^2}{\partial y^2} \right)$$

1) can be written $\nabla^4 \phi = \nabla^2 \nabla^2 \phi = 0$ which is the biharmonic equation:

$$\nabla^2 \nabla^2 \phi = \left(\frac{\partial^2}{\partial x^2} + \frac{\partial^2}{\partial y^2} \right) \left(\frac{\partial^2 \phi}{\partial x^2} + \frac{\partial^2 \phi}{\partial y^2} \right) = \frac{\partial^4 \phi}{\partial x^4} + 2 \frac{\partial^4 \phi}{\partial x^2 \partial y^2} + \frac{\partial^4 \phi}{\partial y^4} = 0$$

$$3. \quad a) \quad \psi_{yz} = \frac{\partial^2 \epsilon_{yz}}{\partial y \partial z} - \frac{\partial}{\partial x} \left(-\frac{\partial \epsilon_{yz}}{\partial x} + \frac{\partial \epsilon_{zx}}{\partial y} + \frac{\partial \epsilon_{xy}}{\partial z} \right) = 0$$

$$\frac{\partial \epsilon_{yz}}{\partial x} = \frac{K}{x^3 y z} - \frac{2Kx^2}{(x^2 + y^2)^2}$$

$$\frac{\partial \epsilon_{zx}}{\partial y} = \frac{-K}{x^2 y^2} + \frac{2Ky^2}{(x^2 + y^2)^2}$$

$$-\frac{\epsilon_{yz}}{\partial x} + \frac{\partial \epsilon_{zx}}{\partial y} = \frac{2Kx^2}{(x^2 + y^2)^2} - \frac{K}{x^3 y z} + \frac{2Ky^2}{(x^2 + y^2)^2} - \frac{K}{x^3 y z}$$

$$= \frac{2K}{x^2 y z} - \frac{2K}{x^2 y z} = 0 \quad \therefore \text{no information about } K$$

similar $\psi_{zx} = 0$ is identically satisfied with no restriction on K

$\psi_{xx} = 0$, etc. are trivially satisfied

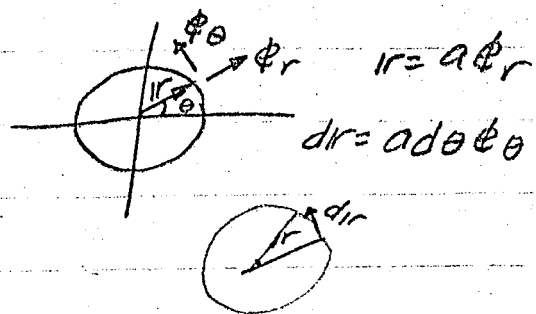
Check compatibility for multiply-connected region

$$b) \quad \oint_{C_i} d\mathbf{r} \cdot (\mathbf{E} \times \nabla) = 0$$

here C_i is a circle of radius a :

$$\epsilon_{zy} = \frac{K \cos \theta}{r} = \frac{Kx}{x^2 + y^2}$$

$$\epsilon_{zx} = \frac{-K \sin \theta}{r} = \frac{-Ky}{x^2 + y^2}$$



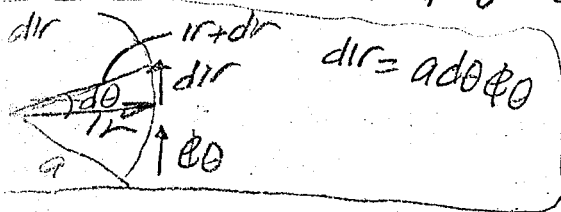
$$\begin{aligned} \mathbf{E} \times \nabla &= (E_z \mathbf{e}_z \phi_y + E_y \mathbf{e}_y \phi_z + E_z \mathbf{e}_z \phi_x + E_x \mathbf{e}_x \phi_z) \times (\phi_x \frac{\partial}{\partial x} + \phi_y \frac{\partial}{\partial y} + \phi_z \frac{\partial}{\partial z}) \\ &= \phi_x (E_z \mathbf{e}_z \phi_z - E_y \mathbf{e}_y \phi_y - E_x \mathbf{e}_x \phi_x) \\ &\quad + \phi_y (-E_z \mathbf{e}_z \phi_z + E_y \mathbf{e}_y \phi_y + E_x \mathbf{e}_x \phi_x) \\ &\quad + \phi_z (E_z \mathbf{e}_z \phi_y - E_y \mathbf{e}_y \phi_x) \end{aligned}$$

$$E_y \mathbf{e}_y \phi_z = \frac{-2Kxy}{(x^2+y^2)^2} = \frac{-2K \sin \theta \cos \theta}{r^2} \quad E_x \mathbf{e}_x \phi_z = \frac{-K}{x^2+y^2} + \frac{2Ky^2}{(x^2+y^2)^2} = \frac{-K}{r^2} + \frac{2K \sin^2 \theta}{r^2}$$

$$E_y \mathbf{e}_y \phi_x = \frac{K}{x^2+y^2} - \frac{2Kx^2}{(x^2+y^2)^2} = \frac{-K}{r^2} + \frac{2K \cos^2 \theta}{r^2} \quad E_x \mathbf{e}_x \phi_y = \frac{2Kxy}{(x^2+y^2)^2} = \frac{2K \sin \theta \cos \theta}{r^2}$$

subst into $\mathbf{E} \times \nabla$ gives

$$\begin{aligned} \mathbf{E} \times \nabla &= \phi_x \left(\frac{2K \sin \theta \cos \theta}{r^2} \phi_y + \left(\frac{K}{r^2} - \frac{2K \sin^2 \theta}{r^2} \right) \phi_x \right) \\ &\quad + \phi_y \left(\left(\frac{K}{r^2} - \frac{2K \cos^2 \theta}{r^2} \right) \phi_y + \frac{2K \sin \theta \cos \theta}{r^2} \phi_x \right) \end{aligned}$$



(ϕ_z term cancels)

$$\mathbf{dir} \cdot \mathbf{e}_z = 0 \quad \mathbf{e}_x \cdot \mathbf{dir} = -a \sin \theta d\theta \quad \mathbf{e}_y \cdot \mathbf{dir} = a \cos \theta d\theta$$

$$\mathbf{dir} \cdot (\mathbf{E} \times \nabla) = \left[\phi_x \left(\frac{K \sin \theta}{r^2} \right) + \phi_y \left(-\frac{K \cos \theta}{r^2} \right) \right] a d\theta$$

notes: $\mathbf{r}_0$ is the position vector of the point at which you start - finish, the line integral $\oint_C$ for a circle centered at origin, the magnitude of $\oint_C$ must be a

$$\begin{aligned} \text{on } C_i, r=a &\Rightarrow \int_0^{2\pi} \left(\phi_x \frac{K \sin \theta}{a} - \phi_y \frac{K \cos \theta}{a} \right) d\theta = 0 \\ &= -\frac{K \cos \theta}{a} \Big|_0^{2\pi} \phi_x - \frac{K \sin \theta}{a} \Big|_0^{2\pi} \phi_y = 0 \quad \text{with no restriction on } K \end{aligned}$$

$$c) \oint_C [\mathbf{E} + (r - r_0) \times \nabla \times \mathbf{E}] \cdot \mathbf{dir} = 0$$

$$\begin{aligned} \mathbf{E} \cdot \mathbf{dir} &= (E_y \mathbf{e}_y \phi_z + E_z \mathbf{e}_z \phi_y + E_z \mathbf{e}_z \phi_x + E_x \mathbf{e}_x \phi_z) \cdot a d\theta (-\phi_x \sin \theta + \phi_y \cos \theta) \\ &= \phi_z \frac{K a d\theta}{r} = \phi_z K d\theta \quad \text{on } C_i \text{ (where } r=a) \end{aligned}$$

$$\text{pick } r_0 = a \mathbf{e}_x; \quad r = a \mathbf{e}_r = a \cos \theta \mathbf{e}_x + a \sin \theta \mathbf{e}_y$$

$$r - r_0 = a(\cos \theta - 1) \mathbf{e}_x + a \sin \theta \mathbf{e}_y$$

$$\nabla \times \mathbf{E} = -\mathbf{E}^T \times \nabla = -\mathbf{E} \times \nabla = \text{(see above)} \quad \begin{bmatrix} \frac{K}{r^2} - \frac{2K \sin^2 \theta}{r^2} & \frac{2K \sin \theta \cos \theta}{r^2} & 0 \\ \frac{2K \sin \theta \cos \theta}{r^2} & \frac{K}{r^2} - \frac{2K \cos^2 \theta}{r^2} & 0 \\ 0 & 0 & 0 \end{bmatrix}$$

$$\begin{aligned} (r - r_0) \times (\nabla \times \mathbf{E}) &= \phi_z \left[\phi_x (a(\cos \theta - 1) \frac{2K \sin \theta \cos \theta}{r^2}) + a \sin \theta \left(\frac{K}{r^2} - \frac{2K \sin^2 \theta}{r^2} \right) \right. \\ &\quad \left. + \phi_y (a(\cos \theta - 1) \left(\frac{K}{r^2} - \frac{2K \cos^2 \theta}{r^2} \right) + a \sin \theta \left(\frac{2K \sin \theta \cos \theta}{r^2} \right)) \right] \end{aligned}$$

$$\begin{aligned} (r - r_0) \times (\nabla \times \mathbf{E}) \cdot \mathbf{dir} &= \frac{\phi_z K a d\theta}{a} \left[\frac{2a(\cos \theta - 1) \sin^2 \theta \cos \theta}{r^2} + a \sin^2 \theta (1 - 2 \sin^2 \theta) + a(\cos \theta - 1)(\cos \theta - 2 \cos^2 \theta) + 2a \sin^2 \theta \cos^2 \theta \right] \\ &= \phi_z K a d\theta [1 - \cos \theta] \end{aligned}$$

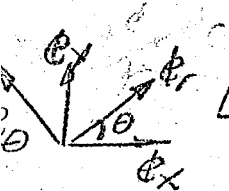
$$\oint_C \mathbf{E} \cdot d\mathbf{r} = \int_0^{2\pi} \phi_z K d\theta = 2\pi K \phi_z$$

$$\oint_C [\mathbf{E} + (r - r_0) \times (\nabla \times \mathbf{E})] \cdot d\mathbf{r} = \phi_z (2\pi K + 2\pi K) = 4\pi K \Rightarrow \underline{K=0}$$

$\therefore$ The given strain field will not be compatible except for the trivial case $K=0$ (i.e. no strain)

Alternate approach : use polar coordinates

transform strain to polar : direction cosines -



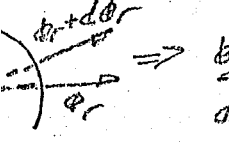
$$\begin{bmatrix} \phi_r \\ \phi_\theta \\ \phi_z \end{bmatrix} = \underbrace{\begin{bmatrix} \cos\theta & \sin\theta & 0 \\ -\sin\theta & \cos\theta & 0 \\ 0 & 0 & 1 \end{bmatrix}}_L \begin{bmatrix} \phi_x \\ \phi_y \\ \phi_z \end{bmatrix} \quad \begin{bmatrix} \phi_{\text{polar}} \end{bmatrix} = \begin{bmatrix} \phi \end{bmatrix} = [L][\mathbf{E}][L^T]$$

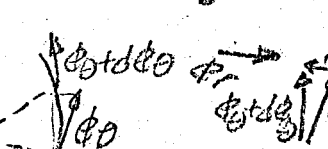
$$= \begin{bmatrix} 0 & 0 & (\cos\theta \phi_x z + \sin\theta \phi_y z) \\ 0 & 0 & (-\sin\theta \phi_x z + \cos\theta \phi_y z) \\ (\cos\theta \phi_x z - \sin\theta \phi_y z) & (-\sin\theta \phi_x z + \cos\theta \phi_y z) & 0 \end{bmatrix} = \begin{bmatrix} 0 & 0 & 0 \\ 0 & 0 & K/r \\ 0 & K/r & 0 \end{bmatrix}$$

Polar $\nabla = (\phi_r \frac{\partial}{\partial r} + \phi_\theta \frac{1}{r} \frac{\partial}{\partial \theta} + \phi_z \frac{\partial}{\partial z})$

$$\nabla \times \mathbf{E} = (\phi_r \frac{\partial}{\partial r} + \phi_\theta \frac{1}{r} \frac{\partial}{\partial \theta} + \phi_z \frac{\partial}{\partial z}) \times (\frac{K}{r} \phi_\theta \phi_z + \frac{K}{r} \phi_z \phi_\theta)$$

note all unit vectors do not vary with r or z thus $\frac{\partial}{\partial r} \phi$ & $\frac{\partial}{\partial z} \phi$ of unit vectors = 0
also $\frac{\partial \phi_z}{\partial \theta} = 0$. However $\frac{\partial \phi_r}{\partial \theta} = \phi_\theta$ and $\frac{\partial \phi_\theta}{\partial r} = -\phi_r$:



$$\frac{\partial \mathbf{r}}{\partial \theta} = \frac{\partial}{\partial \theta} (r \hat{r}) = r \frac{\partial \hat{r}}{\partial \theta} = r \hat{\theta} \Rightarrow \frac{\partial \hat{r}}{\partial \theta} = \hat{\theta}$$


$$\frac{\partial \hat{\theta}}{\partial \theta} = \frac{\partial}{\partial \theta} (\hat{r}) = -\hat{r} = -\phi_r$$

$$\begin{aligned} \nabla \times \mathbf{E} &= \phi_r \left[\frac{1}{r} \frac{\partial}{\partial \theta} \left(\frac{K}{r} \phi_\theta \right) - \frac{\partial}{\partial z} \left(\frac{K}{r} \phi_z \right) \right] + \phi_\theta \left[-\frac{\partial}{\partial r} \left(\frac{K}{r} \phi_\theta \right) \right] + \phi_z \left[\frac{\partial}{\partial r} \left(\frac{K}{r} \phi_z \right) \right] \\ &= -\frac{K}{r^2} \phi_r \phi_r + \frac{K}{r^2} \phi_\theta \phi_\theta - \frac{K}{r^2} \phi_z \phi_z \end{aligned}$$

$r = a \phi_r$; pick $r_0 = 0 \phi_x$ $d\mathbf{r} = a d\theta \phi_\theta$

$r \times (\nabla \times \mathbf{E}) = -\frac{K a}{r^2} (-\phi_z \phi_\theta - \phi_\theta \phi_z)$ $r \times (\nabla \times \mathbf{E}) \cdot d\mathbf{r} = \frac{K a^2}{r^2} d\theta \phi_z$

$-r_0 \times (\nabla \times \mathbf{E}) = \frac{K a}{r^2} [(\phi_x \times \phi_r) \phi_r - (\phi_x \times \phi_\theta) \phi_\theta + (\phi_x \times \phi_z) \phi_z]$

$-r_0 \times (\nabla \times \mathbf{E}) \cdot d\mathbf{r} = -\frac{K a^2}{r^2} d\theta (\phi_x \times \phi_\theta) = -\frac{K a^2}{r^2} d\theta \sin(\theta + \frac{\pi}{2}) \phi_z$

$$= -\frac{K a^2 \cos\theta}{r^2} \phi_z$$

$\mathbf{E} \cdot d\mathbf{r} = \frac{K a}{r} \phi_z$

$\oint_C [\mathbf{E} + (r - r_0) \times (\nabla \times \mathbf{E})] \cdot d\mathbf{r} = \phi_z \int_0^{2\pi} \left(\frac{K a}{r} + \frac{K a^2}{r^2} - \frac{K a^2}{r^2} \cos\theta \right) d\theta$; on $C, r = a$

$\Rightarrow \phi_z \int_0^{2\pi} (K + K - K \cos\theta) d\theta = K \phi_z \int_0^{2\pi} (2 - \cos\theta) d\theta$
 $= 4\pi K = 0$

Problem Set # 3

1. Consider $u = (dx + by + 5z) e_x + (ax + ey + 3z) e_y + (bx + cy + fz) e_z = u_i e_i$

Find a, b, c, d, e, f for u to represent rigid body rotation

For small displacements then $\phi + \omega = \nabla u$ or $\omega = \frac{1}{2} (\nabla u - u \nabla)$, $\phi = \frac{1}{2} (\nabla u + u \nabla)$

and $\epsilon_{ij} = \frac{1}{2} (u_{j,i} + u_{i,j})$, $\omega_{ij} = \frac{1}{2} (u_{j,i} - u_{i,j})$. Thus

$$\epsilon_{ij} = \begin{pmatrix} d & \frac{b+a}{2} & \frac{5+b}{2} \\ \frac{a+b}{2} & e & \frac{3+c}{2} \\ \frac{b+5}{2} & \frac{c+3}{2} & f \end{pmatrix} \quad \omega_{ij} = \begin{pmatrix} 0 & \frac{a-b}{2} & \frac{b-5}{2} \\ \frac{b-a}{2} & 0 & \frac{c-3}{2} \\ \frac{5-b}{2} & \frac{3-c}{2} & 0 \end{pmatrix}$$

For rigid body rotation $\epsilon_{ij} = 0 \forall i, j$ hence $d, e, f = 0$ $a = -6, b = -5, c = -3$

hence

$$\omega_{ij} = \begin{pmatrix} 0 & -6 & -5 \\ 6 & 0 & -3 \\ 5 & 3 & 0 \end{pmatrix} \quad \text{and} \quad u = (by + 5z) e_x + (-6x + 3z) e_y + (-5x - 3y) e_z$$

2. given that ϕ is given by $\begin{pmatrix} \phi_{,yy} & -\phi_{,xy} & 0 \\ -\phi_{,xy} & \phi_{,xx} & 0 \\ 0 & 0 & 0 \end{pmatrix}$ can ϕ be considered arbitrary or does it have to satisfy some conditions

in order to insure single valuedness of displacements

- Assumption: we will be working in a simply connected region

To insure single valued displacements ϕ must check satisfaction of the compatibility equations $\nabla \times \phi \times \nabla = 0$

$\therefore$ Since $\phi = \phi(x, y)$ only, it will satisfy $\psi_{xx}, \psi_{yy}, \psi_{yz}, \psi_{zx}, \psi_{xy} = 0$ identically because they will involve taking derivatives of ϕ with respect to z .

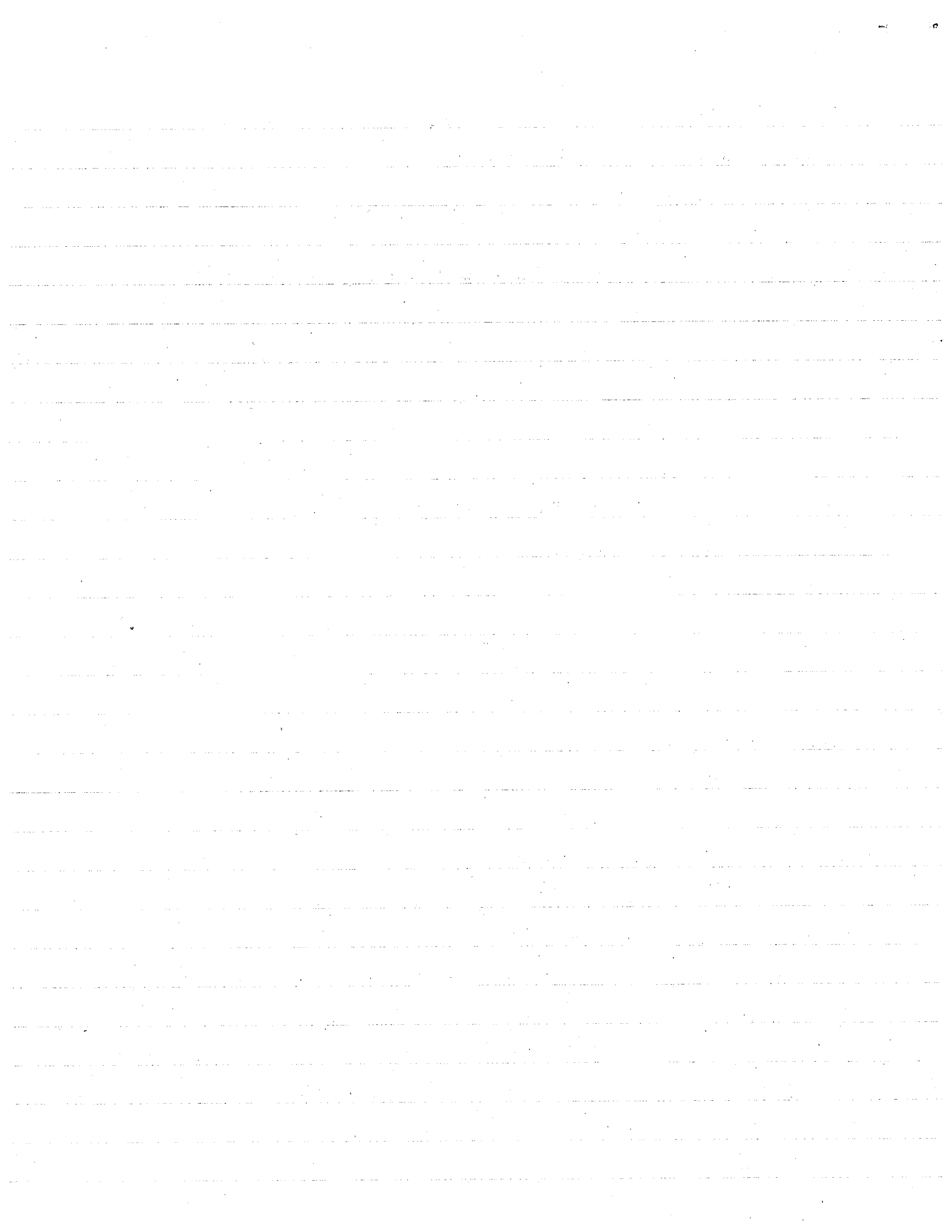
The only non vanishing element of $\nabla \times \phi \times \nabla = \psi$ is

$$\psi_{zz} = 2(-\phi_{,xxyy}) - \phi_{,yyyy} - \phi_{,xxxx} = -\Delta^2 \phi = \nabla^4 \phi \quad (\text{note } \Delta \phi = \frac{\partial^2}{\partial x^2} + \frac{\partial^2}{\partial y^2})$$

For single valued displacements ϕ must satisfy $\Delta^2 \phi = \nabla^4 \phi = 0$ pointwise.

- We note that this will be the only condition of ϕ in a simply connected region
for multiply connected regions we must also check $\oint_{C_i} \text{div} \cdot \phi \times \nabla = 0$ and $\oint_{C_i} [\phi + (r - r_0) \times (\nabla \times \phi)] \cdot dr = 0$

Since no other information about the region is given we will not check these integrals.



3. Given a doubly connected region with Φ given by $\begin{pmatrix} 0 & 0 & A \\ 0 & 0 & B \\ A & B & 0 \end{pmatrix}$ where $A = \frac{kx}{x^2+y^2} = \frac{k \cos \phi}{r}$

and $B = \frac{-ky}{x^2+y^2} = -\frac{k \sin \phi}{r}$. Determine the conditions for single-valued displacements and determine the value of k for the special case of the inner boundary being a circle of radius a .

Solution by CESARO'S theorem we must check $\nabla \times \Phi \times \nabla = \Psi = 0$ pointwise and $\oint_{C_i} \text{dir.} [\Phi \times \nabla] = 0$,

$\oint_{C_i} [\Phi + (I - II_0) \times (\nabla \times \Phi)] \cdot \text{dir} = 0$ over any curve enclosing the inclusions in the region ✓

A $\nabla \times \Phi \times \nabla = \Psi$: we note that $\Psi_{xx} = \Psi_{yy} = \Psi_{zz} = \Psi_{xy} = 0$ identically, since both ϵ_{yz} , ϵ_{zx} are fns of x, y only and these values of Ψ_{ij} require taking derivatives with respect to z . But

$$\Psi_{yz} = \frac{\partial^2}{\partial x^2} \epsilon_{yz} - \frac{\partial^2}{\partial x \partial y} \epsilon_{zx} \quad \text{and} \quad \Psi_{zx} = \frac{\partial^2}{\partial y^2} \epsilon_{zx} - \frac{\partial^2}{\partial x \partial y} \epsilon_{yz}$$

$$\text{Now } \frac{\partial \epsilon_{yz}}{\partial x} = \frac{k(y^2 - x^2)}{(x^2 + y^2)^2} = -\frac{k}{r^2} \cos 2\phi ; \quad \frac{\partial^2 \epsilon_{yz}}{\partial x^2} = \frac{(3y^2 - x^2)(-2kx)}{(x^2 + y^2)^3} ; \quad \frac{\partial \epsilon_{zx}}{\partial x} = \frac{2kyx}{(x^2 + y^2)^2} ;$$

$$\frac{\partial^2 \epsilon_{zx}}{\partial x \partial y} = \frac{2kx(x^2 - 3y^2)}{(x^2 + y^2)^3}$$

Thus we can see that $\Psi_{yz} \equiv 0$.

We also note that $-\epsilon_{zx}$ can be obtained by replacing the $\begin{pmatrix} x \\ y \end{pmatrix}$ by $\begin{pmatrix} y \\ x \end{pmatrix}$ in ϵ_{yz} . Hence $-\epsilon_{zx,yy}$ can be obtained by replacing the $\begin{pmatrix} x \\ y \end{pmatrix}$ by $\begin{pmatrix} y \\ x \end{pmatrix}$ in $\epsilon_{yz,xx}$ (This is OK to do since differentiation is with respect to the other variable). Similarly $-\epsilon_{yz}$ can be obtained by replacing the $\begin{pmatrix} x \\ y \end{pmatrix}$ by $\begin{pmatrix} y \\ x \end{pmatrix}$ in ϵ_{zx} then $-\epsilon_{yz,xy}$ can be obtained by replacing the $\begin{pmatrix} x \\ y \end{pmatrix}$ by $\begin{pmatrix} y \\ x \end{pmatrix}$ in $\epsilon_{zx,xy}$.

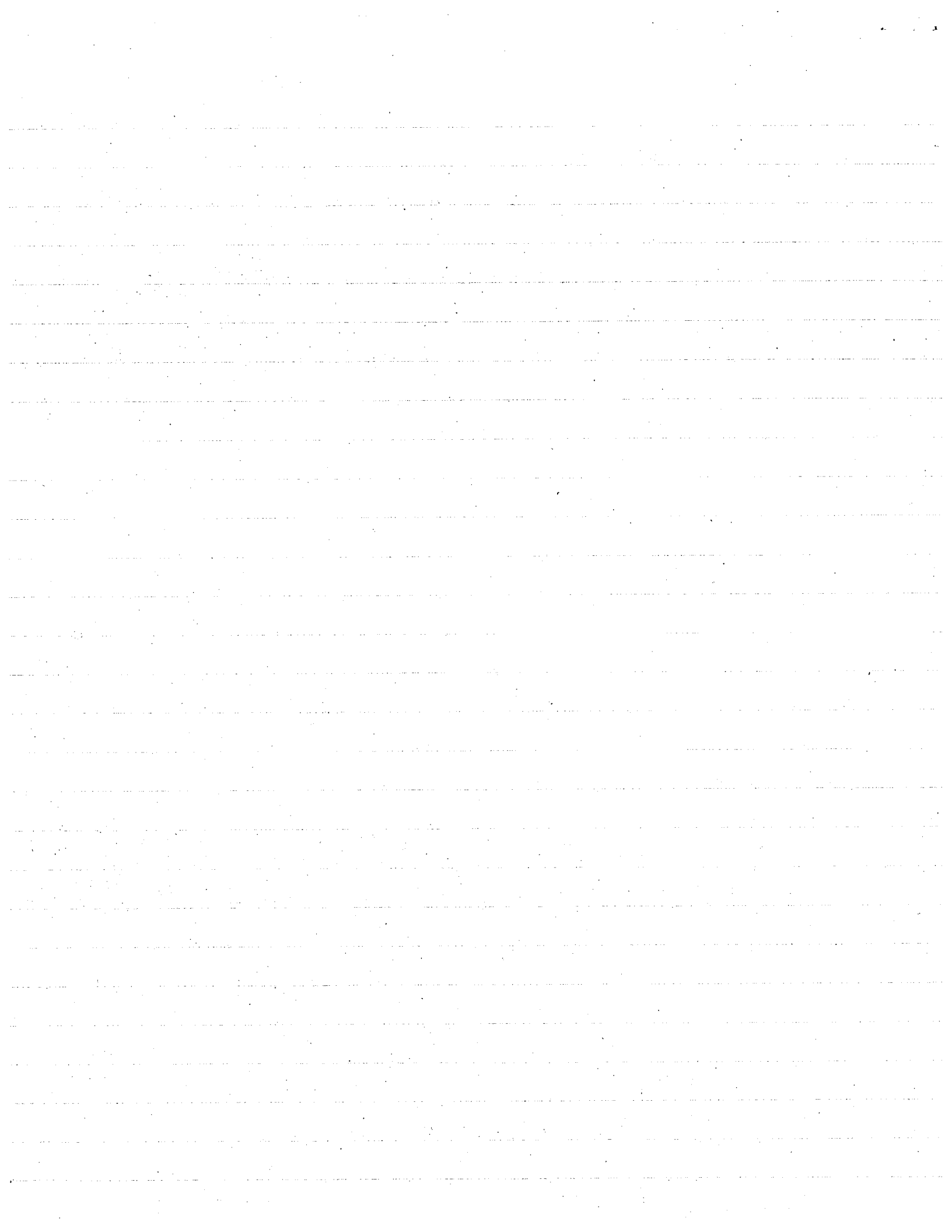
$$\text{Hence } \Psi_{zx} = \frac{\partial^2}{\partial y^2} \epsilon_{zx} - \frac{\partial^2}{\partial x \partial y} \epsilon_{yz} = \text{Replaced } (-\epsilon_{yz,xx} + \epsilon_{zx,xy}) = \text{Replaced } (-\Psi_{yz}) = 0$$

Thus Φ satisfies $\nabla \times \Phi \times \nabla \equiv 0$

B $\Phi \times \nabla = \epsilon_{il,k} \epsilon_{lkj} \Phi_i \Phi_j$ and $\text{dir.} \Phi \times \nabla = dx_m \Phi_m \cdot \epsilon_{il,k} \epsilon_{lkj} \Phi_i \Phi_j = dx_i \epsilon_{il,k} \epsilon_{lkj} \Phi_i \Phi_j$

this reduces to the following: $\oint \text{dir.} \Phi \times \nabla = 0 =$

$$\oint_C dx_i (\epsilon_{i1,2} - \epsilon_{i2,1}) \Phi_3 + \oint_C dx_i (\epsilon_{i2,3} - \epsilon_{i3,2}) \Phi_1 + \oint_C dx_i (\epsilon_{i3,1} - \epsilon_{i1,3}) \Phi_2 = 0$$



or we get 3 independent equations

$$1. \oint_C (\epsilon_{11,2} - \epsilon_{12,1}) dx_1 + \oint_C dx_2 (\epsilon_{21,2} - \epsilon_{22,1}) + \oint_C (\epsilon_{31,2} - \epsilon_{32,1}) dx_3 \equiv 0 \text{ since } dx_3 = 0 \text{ and all } \epsilon_{ij} \text{ in the other two integrals} = 0$$

$$2. \oint_C (\epsilon_{12,3} - \epsilon_{13,2}) dx_1 + \oint_C (\epsilon_{21,3} - \epsilon_{23,2}) dx_2 + \oint_C (\epsilon_{32,3} - \epsilon_{33,2}) dx_3 = \oint_C -\epsilon_{13,2} dx_1 - \epsilon_{23,2} dx_2 \text{ since } dx_3 = 0 \text{ and all other } \epsilon_{ij} = 0$$

$$3. \oint_C (\epsilon_{13,1} - \epsilon_{11,3}) dx_1 + \oint_C (\epsilon_{23,1} - \epsilon_{21,3}) dx_2 + \oint_C (\epsilon_{33,1} - \epsilon_{31,3}) dx_3 = \oint_C \epsilon_{13,1} dx_1 + \epsilon_{23,1} dx_2 \text{ since } dx_3 = 0 \text{ and all other } \epsilon_{ij} = 0$$

$$\text{For single valuedness } \oint_C -\epsilon_{13,2} dx_1 - \epsilon_{23,2} dx_2 = \oint_C \epsilon_{13,1} dx_1 - \epsilon_{23,1} dx_2 = 0$$

$$c. \oint_C [\mathbf{f} + (\mathbf{r} - \mathbf{r}_0) \times (\nabla \times \mathbf{f})] \cdot d\mathbf{r} = 0 \quad \times -4$$

First we will take $\mathbf{r}_0$ to be the origin and remember that

$$\nabla \times \mathbf{f} = \epsilon_{\ell j, k} e_{k\ell i} \Phi_i \Phi_j \quad \text{hence } \mathbf{r} \times (\nabla \times \mathbf{f}) = (x_m \mathbf{e}_m) \times (\epsilon_{\ell j, k} e_{k\ell i} \Phi_i \Phi_j) = x_m \epsilon_{\ell j, k} e_{k\ell i} e_{min} \Phi_n \Phi$$

hence we get for $[(\mathbf{r} - \mathbf{r}_0) \times \nabla \times \mathbf{f}] \cdot d\mathbf{r} = x_m \epsilon_{\ell j, k} e_{k\ell i} e_{min} \Phi_n dx_j$. Putting it together the integral reduces to

$$\oint_C [\mathbf{f} \times (\mathbf{r} - \mathbf{r}_0) \times (\nabla \times \mathbf{f})] \cdot d\mathbf{r} = \oint_C \left\{ (\epsilon_{31} - x_2 \epsilon_{31,2} - x_1 \epsilon_{31,1}) dx_1 + (\epsilon_{32} - x_2 \epsilon_{32,2} - x_1 \epsilon_{32,1}) dx_2 \right\} \mathbf{e}_3 + \left\{ (\epsilon_{32} - x_1 \epsilon_{32,1} + x_1 \epsilon_{31,2}) dx_3 \right\} \mathbf{e}_2 + \left\{ (\epsilon_{31} + x_2 \epsilon_{32,1} - x_2 \epsilon_{31,2}) dx_3 \right\} \mathbf{e}_1 = 0$$

or we get 3 independent equations

$$1. \oint_C [(\epsilon_{31} - x_2 \epsilon_{31,2} - x_1 \epsilon_{31,1}) dx_1 + (\epsilon_{32} - x_2 \epsilon_{32,2} - x_1 \epsilon_{32,1}) dx_2] = 0$$

$$2. \text{ and } 3. \oint_C (\epsilon_{32} - x_1 \epsilon_{32,1} + x_1 \epsilon_{31,2}) dx_3 = \oint_C (\epsilon_{31} + x_2 \epsilon_{32,1} - x_2 \epsilon_{31,2}) dx_3 \equiv 0 \text{ since } dx_3 = 0$$

now we can rewrite equation 1 as

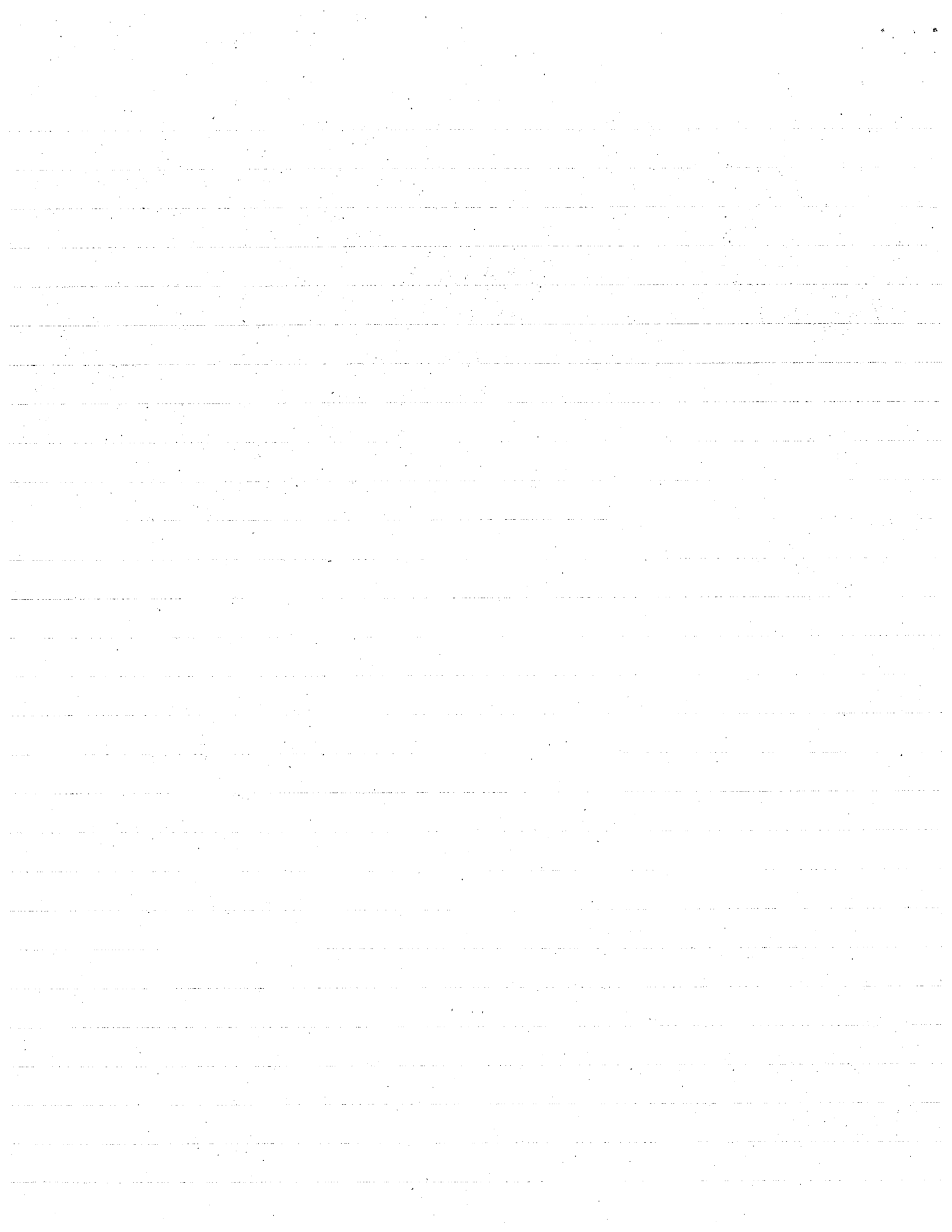
$$\oint_C (x_1 \epsilon_{31,2} dx_2 + x_2 \epsilon_{32,1} dx_1) + 2 \oint_C (\epsilon_{31} dx_1 + \epsilon_{32} dx_2) - (x_1 \epsilon_{31} + x_2 \epsilon_{32}) \Big|_0^{2\pi} - \oint_C (x_2 \epsilon_{31,2} dx_1 + x_1 \epsilon_{32,1} dx_2) = 0$$

and since $x_1(2\pi) = x_1(0)$ & $x_2(2\pi) = x_2(0)$ for single valuedness, we obtain

$$\oint_C [(x_1 \epsilon_{31,2} dx_2 + x_2 \epsilon_{32,1} dx_1) + 2(\epsilon_{31} dx_1 + \epsilon_{32} dx_2) - (x_2 \epsilon_{31,2} dx_1 + x_1 \epsilon_{32,1} dx_2)] = 0$$

- SUMMARY we will use $x_1 = r \cos \phi$, $x_2 = r \sin \phi$ with $r = r(\phi)$ in general so that $r(0) = r(2\pi)$

$$\nabla \times \mathbf{f} \times \nabla \equiv 0$$



From B. $\oint_C -\epsilon_{13,2} dx_1 - \epsilon_{23,2} dx_2 = \oint_C (-\epsilon_{31,2} + \epsilon_{23,1}) dx_1 \equiv \oint_C d\epsilon_{23} \equiv 0$ (I)

$\oint_C \epsilon_{13,1} dx_1 + \epsilon_{23,1} dx_2 = \oint_C (-\epsilon_{13,2} + \epsilon_{23,1}) dx_2 \equiv -\oint_C d\epsilon_{13} \equiv 0$ (II)

where for single valued displacements $\oint_C d\epsilon_{23} \equiv 0$, $\oint_C d\epsilon_{13} \equiv 0$. Note that the integrands are 0, so that it doesn't matter what the shape of the hole is (except that the 2nd derivs of ϵ_{ij} be continuous)

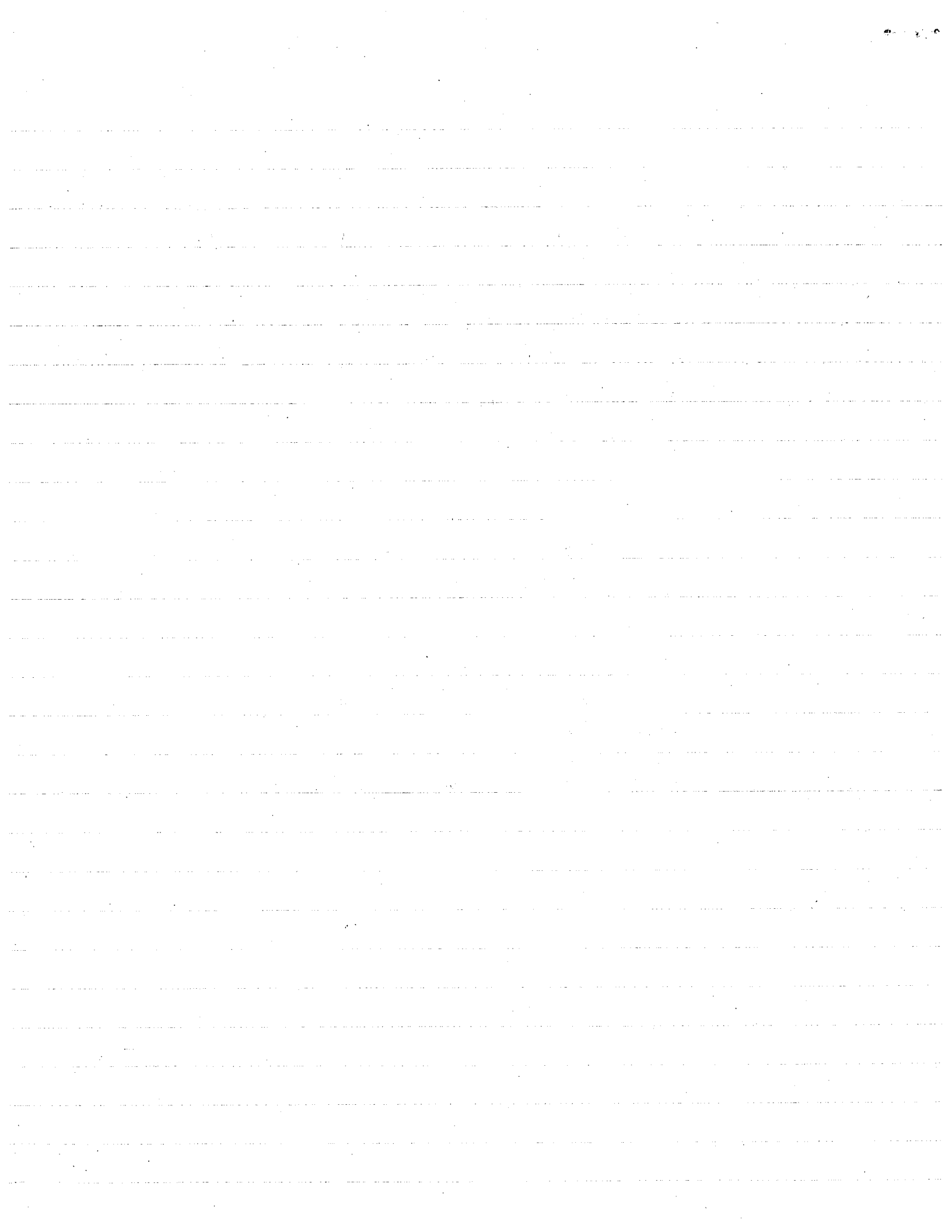
From C.

$\oint_C [2(\epsilon_{31} dx_1 + \epsilon_{32} dx_2) - (x_2 \epsilon_{31,2} dx_1 + x_1 \epsilon_{32,1} dx_2) + (x_1 \epsilon_{31,2} dx_2 + x_2 \epsilon_{32,1} dx_1)] = \int_0^{2\pi} 2K d\phi = 0$ (III)

or in general $4K\pi = 0$ note that this integral is not dependent on r and would be equal to $4K\pi$ no matter what shape the closed curve is (Curve has to be such that 2nd derivs of ϵ_{ij} are cont.)

D for the specific case $r=a$

We note that I, II tell you nothing about K . III tells you that $K \equiv 0$.
It is very interesting though that the results (both in general and specific terms) do not depend on the radius and hence the shape of the hole is of no consequence.



DIVISION OF APPLIED MECHANICS
DEPARTMENT OF MECHANICAL ENGINEERING

ME 238A Theory of Elasticity

Autumn 1978

Problem Set No. 4

1. Determine the transformation relations of components of strain if the yz -plane is rotated about the x -axis through an angle θ .
2. Consider a cubic crystal with three elastic constants c_{11} , c_{12} and c_{44} . Determine the relation between these constants if the stress-strain relations are to remain unchanged for a rotation of the xy -plane about the z -axis through an angle ψ .
3. a) Express Lamé's constant λ in terms of the shear modulus and the bulk modulus.
b) Express Poisson's ratio in terms of Young's modulus and Lamé's constant μ .

3. Write $\lambda = \lambda(G, K)$ where $e = \frac{I\sigma}{(3\lambda + 2\mu)} = -3p$ where $e = e_x + e_y + e_z$

$$K = \frac{3\lambda + 2\mu}{3}$$

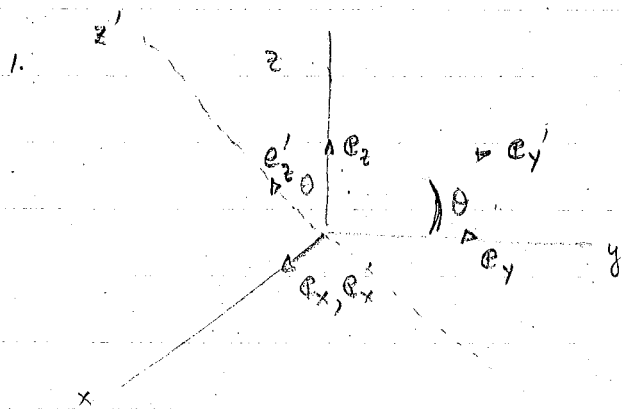
$$G = \frac{E}{2(1+\nu)} = \mu$$

$$3K = 3\lambda + 2G \Rightarrow \left| \frac{3K - 2G}{3} = \lambda \right|$$

$$\nu = \nu(E, \mu) \quad \text{since} \quad G = \mu = \frac{E}{2(1+\nu)}$$

$$\frac{E}{2\mu} = 1 + \nu$$

$$\therefore 1 + \nu = \frac{E}{2\mu} \quad \text{or} \quad \left| \nu = \frac{E}{2\mu} - 1 = \frac{E - 2\mu}{2\mu} \right|$$



find ϵ'_{ij}

$$e'_m \cdot e_i = l_{mi}$$

	e_1	e_2	e_3
e'_1	1	0	0
e'_2	0	$\cos \theta$	$\sin \theta$
e'_3	0	$-\sin \theta$	$\cos \theta$

$$l_{21} = l_{12} = l_{13} = l_{31} = 0$$

$$\epsilon_{mn} = \epsilon_{ij} l_{mi} l_{nj}$$

$$\checkmark \epsilon'_{11} = \epsilon_{11}$$

$$\checkmark \gamma'_{12} = \gamma_{12} \cos \theta + \gamma_{13} \sin \theta$$

$$\checkmark \gamma'_{13} = -\gamma_{12} \sin \theta + \gamma_{13} \cos \theta$$

$$\checkmark \gamma'_{21} = \gamma_{21} \cos \theta + \gamma_{31} \sin \theta$$

$$\checkmark \epsilon'_{22} = \epsilon_{22} \cos^2 \theta + \gamma_{23} \frac{\sin 2\theta}{2} + \epsilon_{33} \sin^2 \theta$$

$$\checkmark \gamma'_{23} = -\epsilon_{22} \sin 2\theta + \gamma_{23} (\cos^2 \theta - \sin^2 \theta) + \epsilon_{33} \sin 2\theta$$

$$\checkmark \gamma'_{31} = -\gamma_{21} \sin \theta + \gamma_{31} \cos \theta$$

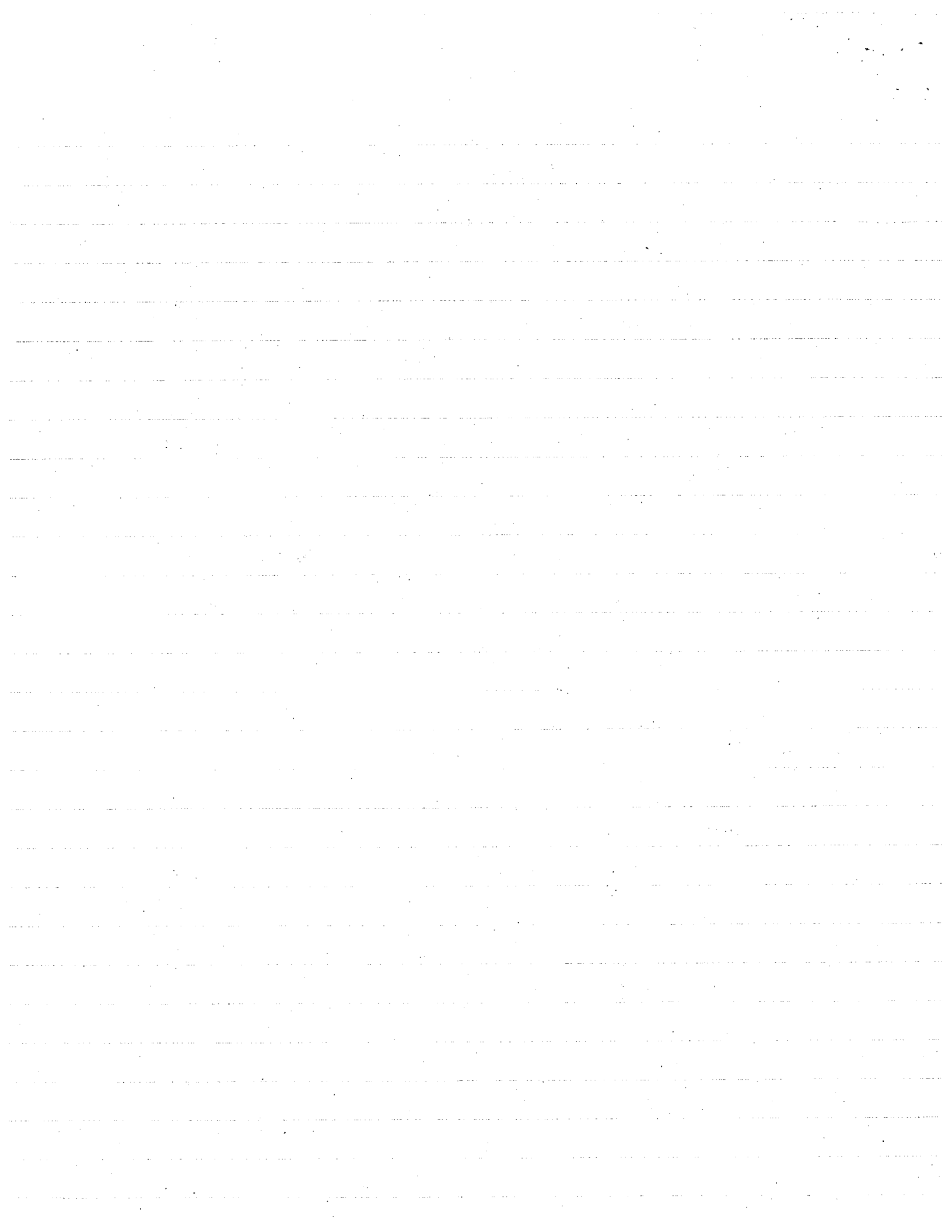
$$\checkmark \gamma'_{32} = -\epsilon_{22} \sin 2\theta + \gamma_{23} (\cos^2 \theta - \sin^2 \theta) + \epsilon_{33} \sin 2\theta$$

$$\checkmark \epsilon'_{33} = \epsilon_{22} \sin^2 \theta - \gamma_{23} \frac{\sin 2\theta}{2} + \epsilon_{33} \cos^2 \theta$$

$$\begin{aligned} x &\rightarrow z \\ z &\rightarrow y \\ y &\rightarrow x \end{aligned}$$

$$\therefore \epsilon'_{11} = \epsilon_{11}, \quad \epsilon'_{22} = \epsilon_{22} \cos^2 \theta + \epsilon_{33} \sin^2 \theta + \gamma_{23} \frac{\sin 2\theta}{2}, \quad \epsilon'_{33} = \epsilon_{22} \sin^2 \theta - \gamma_{23} \frac{\sin 2\theta}{2} + \epsilon_{33} \cos^2 \theta$$

$$\gamma'_{12} = \gamma'_{21} = \gamma_{12} \cos \theta + \gamma_{13} \sin \theta, \quad \gamma'_{13} = \gamma'_{31} = -\gamma_{12} \sin \theta + \gamma_{13} \cos \theta, \quad \gamma'_{23} = \gamma'_{32} = \sin 2\theta (\epsilon_{33} - \epsilon_{22}) + \gamma_{23} \cos 2\theta$$



where $\epsilon_1 = \epsilon_{11}$ $\epsilon_2 = \epsilon_{22}$ $\epsilon_3 = \epsilon_{33}$ $\epsilon_4 = 2\epsilon_{23}$ $\epsilon_5 = 2\epsilon_{13}$ $\epsilon_6 = 2\epsilon_{12}$

2. A cubic crystal

$$\begin{bmatrix} c_{11} & c_{12} & c_{12} & & & \\ c_{12} & c_{11} & c_{12} & & & \\ c_{12} & c_{12} & c_{11} & & & \\ & & & c_{44} & c_{44} & c_{44} \end{bmatrix}$$

we want

$$\begin{bmatrix} x' \\ y' \\ z' \end{bmatrix} = \begin{bmatrix} \cos \psi & \sin \psi & 0 \\ -\sin \psi & \cos \psi & 0 \\ 0 & 0 & 1 \end{bmatrix} \begin{bmatrix} x \\ y \\ z \end{bmatrix}$$

$$\sigma_{xx} \quad \sigma_1' = \sigma_1 \cos^2 \psi + \sigma_6 \sin 2\psi + \sigma_2 \sin^2 \psi$$

$$\sigma_{yy} \quad \sigma_2' = \sigma_3 \sin^2 \psi - \sigma_6 \sin 2\psi + \sigma_2 \cos^2 \psi$$

$$\sigma_{zz} \quad \sigma_3' = \sigma_3$$

$$\sigma_{yz} \quad \sigma_4' = -\sigma_5 \sin \psi + \sigma_4 \cos \psi$$

$$\sigma_{zx} \quad \sigma_5' = \sigma_5 \cos \psi + \sigma_4 \sin \psi$$

$$\sigma_{xy} \quad \sigma_6' = -\frac{\sigma_1}{2} \sin 2\psi + \sigma_6 \cos 2\psi + \frac{\sigma_2}{2} \sin 2\psi$$

$$= (\sigma_2 - \sigma_1) \cos 2\psi + \sigma_6 \sin 2\psi$$

$$\epsilon_1' = \epsilon_1 \cos^2 \psi + \epsilon_6 \frac{\sin 2\psi}{2} + \epsilon_2 \sin^2 \psi$$

$$\epsilon_2' = \epsilon_1 \sin^2 \psi - \epsilon_6 \frac{\sin 2\psi}{2} + \epsilon_2 \cos^2 \psi$$

$$\epsilon_3' = \epsilon_3$$

$$\epsilon_4' = -\epsilon_5 \sin \psi + \epsilon_4 \cos \psi$$

$$\epsilon_5' = \epsilon_5 \cos \psi + \epsilon_4 \sin \psi$$

$$\epsilon_6' = -\epsilon_1 \sin 2\psi + \epsilon_6 \cos 2\psi + \epsilon_2 \sin 2\psi$$

$$= (\epsilon_2 - \epsilon_1) \sin 2\psi + \epsilon_6 \cos 2\psi$$

$$\sigma_1' = c_{11} \epsilon_1' + c_{12} \epsilon_2' + c_{12} \epsilon_3' \quad \& \quad \sigma_1 = c_{11} \epsilon_1 + c_{12} \epsilon_2 + c_{12} \epsilon_3$$

$$\sigma_1 \cos^2 \psi + \sigma_6 \sin 2\psi + \sigma_2 \sin^2 \psi = c_{11} \epsilon_1 \cos^2 \psi + c_{11} \epsilon_6 \frac{\sin 2\psi}{2} + c_{11} \epsilon_2 \sin^2 \psi + c_{12} \epsilon_3 \sin^2 \psi - c_{12} \epsilon_6 \frac{\sin 2\psi}{2} + c_{12} \epsilon_2 \cos^2 \psi + c_{12} \epsilon_3$$

$$\sigma_2' = c_{12} \epsilon_1' + c_{11} \epsilon_2' + c_{12} \epsilon_3' \quad \& \quad \sigma_2 = c_{12} \epsilon_1 + c_{11} \epsilon_2 + c_{12} \epsilon_3$$

$$\sigma_3 \sin^2 \psi - \sigma_6 \sin 2\psi + \sigma_2 \cos^2 \psi = c_{12} (\epsilon_1 \cos^2 \psi + \epsilon_6 \frac{\sin 2\psi}{2} + \epsilon_2 \sin^2 \psi) + c_{11} (\epsilon_3 \sin^2 \psi - \epsilon_6 \frac{\sin 2\psi}{2} + \epsilon_2 \cos^2 \psi) + c_{12} \epsilon_3$$

$$\sigma_3' = c_{12} \epsilon_1' + c_{12} \epsilon_2' + c_{11} \epsilon_3' \quad \sigma_3 = c_{12} \epsilon_1 + c_{12} \epsilon_2 + c_{11} \epsilon_3$$

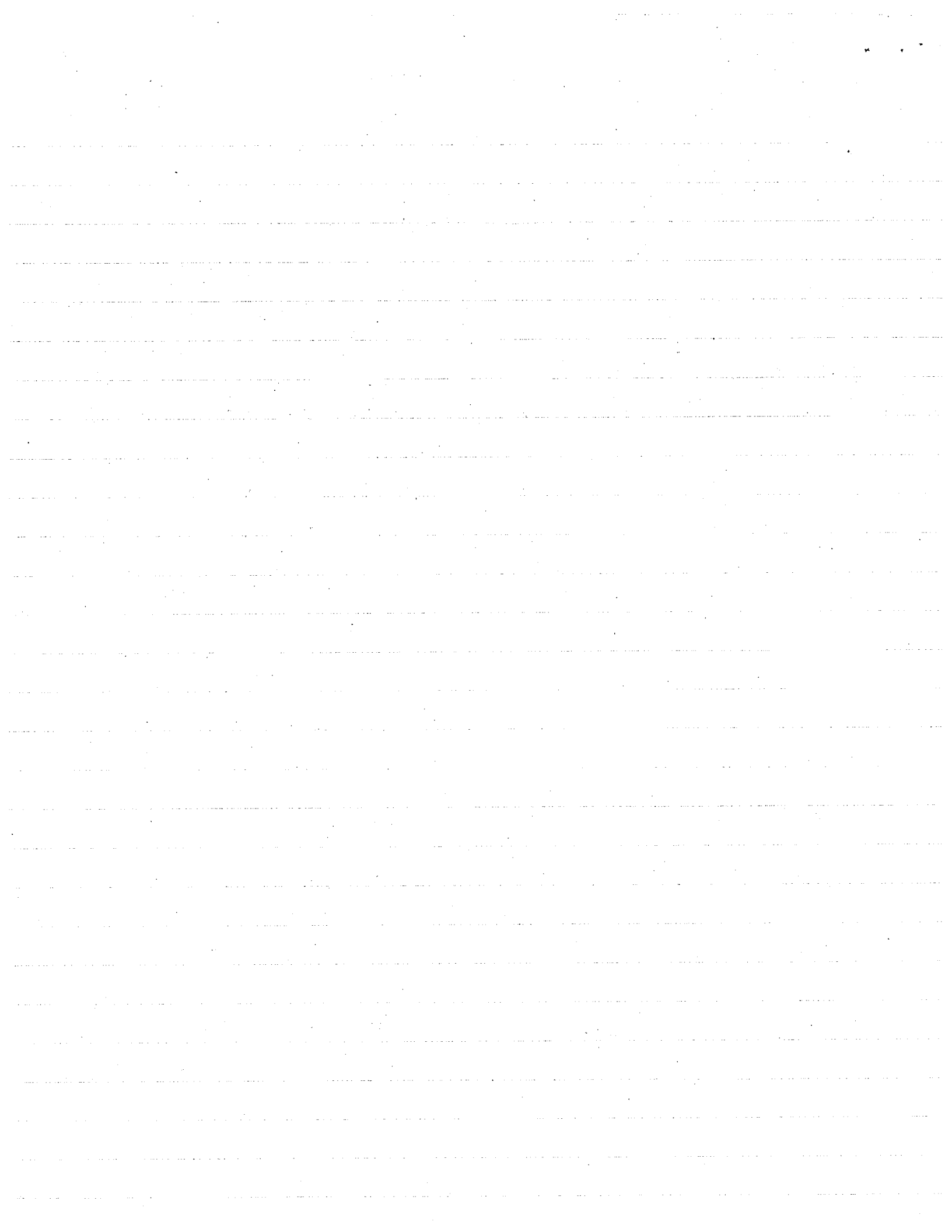
$$\sigma_3 = c_{12} (\epsilon_1 \cos^2 \psi + \epsilon_6 \frac{\sin 2\psi}{2} + \epsilon_2 \sin^2 \psi) + c_{12} (\epsilon_3 \sin^2 \psi - \epsilon_6 \frac{\sin 2\psi}{2} + \epsilon_2 \cos^2 \psi) + c_{11} \epsilon_3$$

$$\sigma_4' = c_{44} \epsilon_4' \quad \sigma_4 = c_{44} \epsilon_4$$

$$(-\sigma_5 \sin \psi + \sigma_4 \cos \psi) = c_{44} (-\epsilon_5 \sin \psi + \epsilon_4 \cos \psi) \Rightarrow -\sigma_5 \sin \psi = c_{44} (-\epsilon_5 \sin \psi)$$

$$\sigma_5' = c_{44} \epsilon_5' \quad \sigma_5 = c_{44} \epsilon_5$$

$$\sigma_5 \cos \psi + \sigma_4 \sin \psi = c_{44} (\epsilon_5 \cos \psi + \epsilon_4 \sin \psi) \Rightarrow \sigma_4 \sin \psi = c_{44} \epsilon_4 \sin \psi$$



$$\sigma'_6 = c_{44} \epsilon'_6$$

$$\sigma_6 = c_{44} \epsilon_6$$

$$-\frac{\sigma_1}{2} \sin 2\psi + \sigma_6 \cos 2\psi + \frac{\sigma_2}{2} \sin 2\psi = c_{44} (-\epsilon_1 \sin 2\psi + \epsilon_6 \cos 2\psi + \epsilon_2 \sin 2\psi)$$

$$\begin{aligned} \sigma_1 \cos^2 \psi &= c_{11} \epsilon_1 \cos^2 \psi + c_{12} \epsilon_2 \cos^2 \psi + c_{12} \epsilon_3 \cos^2 \psi \\ + \sigma_6 \sin 2\psi &= c_{44} \epsilon_6 \sin 2\psi \\ + \sigma_2 \sin^2 \psi &= c_{12} \epsilon_1 \sin^2 \psi + c_{11} \epsilon_2 \sin^2 \psi + c_{12} \epsilon_3 \sin^2 \psi \\ &= c_{11} \epsilon_1 \cos^2 \psi + c_{12} \epsilon_2 \cos^2 \psi + c_{12} \epsilon_3 \cos^2 \psi + c_{44} \epsilon_6 \sin 2\psi + c_{12} \epsilon_1 \sin^2 \psi + c_{11} \epsilon_2 \sin^2 \psi \\ &= c_{11} \epsilon_1 \cos^2 \psi + c_{12} \epsilon_2 \cos^2 \psi + c_{12} \epsilon_3 \cos^2 \psi + \frac{c_{11}}{2} \epsilon_6 \sin 2\psi + c_{12} \epsilon_1 \sin^2 \psi + c_{11} \epsilon_2 \sin^2 \psi \\ &\quad - \frac{c_{12}}{2} \epsilon_6 \sin 2\psi \end{aligned}$$

$$\Rightarrow \left[\cancel{c_{12} (\cos^2 \psi - 1) \epsilon_3} + (c_{44} - \frac{c_{11} + c_{12}}{2}) \epsilon_6 \sin 2\psi + \cancel{c_{12} \epsilon_1 \sin^2 \psi} + \cancel{c_{11} \epsilon_2 \sin^2 \psi} = 0 \right]$$

$$c_{44} - \frac{c_{11} + c_{12}}{2} = 0$$

$$c_{44} = \frac{c_{11} + c_{12}}{2}$$

$$\begin{aligned} \sigma_3 \sin^2 \psi &= c_{12} \epsilon_1 \sin^2 \psi + c_{12} \epsilon_2 \sin^2 \psi + c_{11} \epsilon_3 \sin^2 \psi \\ - \sigma_6 \sin 2\psi &= -c_{44} \epsilon_6 \sin 2\psi \\ + \sigma_2 \cos^2 \psi &= c_{12} \epsilon_1 \cos^2 \psi + c_{11} \epsilon_2 \cos^2 \psi + c_{12} \epsilon_3 \cos^2 \psi \\ &= c_{12} \epsilon_1 \sin^2 \psi + c_{12} \epsilon_2 \sin^2 \psi + c_{11} \epsilon_3 \sin^2 \psi - c_{44} \epsilon_6 \sin 2\psi + c_{12} \epsilon_1 \cos^2 \psi + c_{11} \epsilon_2 \cos^2 \psi + c_{12} \epsilon_3 \cos^2 \psi \\ &= c_{12} \epsilon_1 \cos^2 \psi + c_{12} \epsilon_2 \sin^2 \psi + c_{11} \epsilon_3 \sin^2 \psi + c_{12} \epsilon_6 \sin 2\psi + c_{11} \epsilon_2 \cos^2 \psi + c_{12} \epsilon_3 \cos^2 \psi \\ &\quad - c_{11} \epsilon_6 \sin 2\psi \end{aligned}$$

$$\Rightarrow \left[-c_{12} \epsilon_1 \cos 2\psi + \epsilon_6 (-c_{44} - c_{12} + c_{11}) \sin 2\psi + \epsilon_1 (c_{12} \cos^2 \psi) + c_{12} (\cos^2 \psi - 1) \epsilon_3 = 0 \right]$$

$$\sigma_3 = c_{12} \epsilon_1 + c_{12} \epsilon_2 + c_{11} \epsilon_3$$

$$\sigma_3 = c_{12} \epsilon_1 \cos^2 \psi + c_{12} \epsilon_2 \sin^2 \psi + c_{11} \epsilon_3 + c_{12} \epsilon_6 \sin 2\psi$$

$$c_{12} \epsilon_1 \sin^2 \psi + c_{12} \epsilon_2 \cos^2 \psi + \dots - c_{12} \epsilon_6 \sin 2\psi$$

$$\left[c_{12} (1 - \cos^2 \psi) \epsilon_1 + c_{12} (-\sin^2 \psi) \epsilon_3 = 0 \right]$$

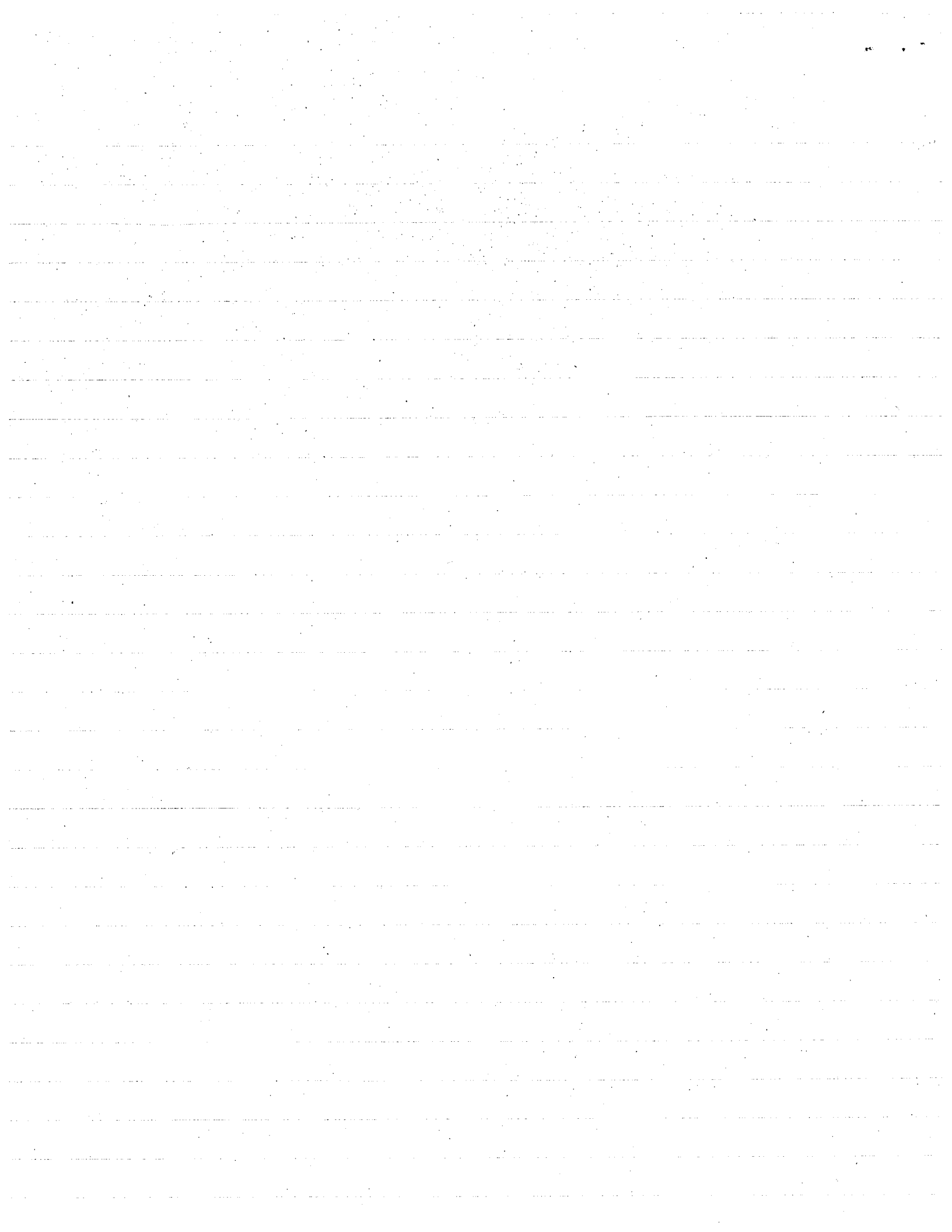
$0 = 0$ nothing from here $\cancel{\sin 2\psi \neq 0}$

$$-\sigma_5 \sin \psi + \sigma_4 \cos \psi = -\epsilon_5 c_{44} \sin \psi + \epsilon_4 c_{44} \cos \psi$$

$$= \epsilon_5 c_{44} \sin \psi + \epsilon_4 c_{44} \cos \psi$$

$$0 = 0 \quad \checkmark \checkmark$$

nothing here



$$\sigma_5 \cos \psi + \sigma_4 \sin \psi = \epsilon_5 c_{44} \cos \psi + \epsilon_4 c_{44} \sin \psi$$

$$c_{44} \epsilon_5 \cos \psi + \epsilon_4 c_{44} \sin \psi$$

$0 = 0$ ✓ nothing here.

$$-\frac{\sigma_1}{2} \sin 2\psi = -\frac{1}{2} \sin 2\psi (c_{11} \epsilon_1 + c_{12} \epsilon_2 + c_{12} \epsilon_3)$$

$$+\sigma_6 \cos 2\psi = c_{44} \epsilon_6 \cos 2\psi$$

$$+\frac{\sigma_2}{2} \sin 2\psi = \frac{1}{2} \sin 2\psi (c_{12} \epsilon_1 + c_{11} \epsilon_2 + \epsilon_3 c_{12})$$

$$= -c_{11} \epsilon_1 \left(+\frac{1}{2} \sin 2\psi\right) - c_{12} \epsilon_2 \left(+\frac{1}{2} \sin 2\psi\right) + c_{12} \epsilon_3 \left(-\frac{1}{2} \sin 2\psi\right) + c_{44} \epsilon_6 \cos 2\psi + \frac{1}{2} \sin 2\psi c_{12} \epsilon_1 + \frac{1}{2} \sin 2\psi c_{11} \epsilon_2 + \frac{1}{2} \sin 2\psi c_{12} \epsilon_3$$

$$-c_{44} \epsilon_1 \left(\sin 2\psi\right) + c_{44} \epsilon_2 \left(-\sin 2\psi\right) + c_{44} \epsilon_6 \cos 2\psi$$

$$\epsilon_1 \left(-c_{11} + c_{44} + c_{12}\right) \frac{\sin 2\psi}{2} + \epsilon_2 \left(-c_{12} - c_{11} - c_{44}\right) \frac{\sin 2\psi}{2} - \sin 2\psi c_{12} \epsilon_3 = 0$$

$$\left(-\frac{c_{11} + c_{12} + c_{44}}{2}\right) \epsilon_1 \sin 2\psi + \left(\frac{c_{11} - c_{12} + c_{44}}{2}\right) \epsilon_2 \sin 2\psi = 0 \quad \left(-\frac{c_{11} + c_{12}}{2} + c_{44}\right) (\epsilon_1 - \epsilon_2) \sin 2\psi = 0$$

$$\frac{c_{11} - c_{12}}{2} = c_{44}$$

$c_{12} \sin^2 \psi$	$c_{11} \sin^2 \psi$	$c_{12} (-\sin^2 \psi)$	0	0	$(c_{44} - c_{11} + c_{12}) \sin^2 \psi$
$c_{12} \sin^2 \psi$	0	$c_{12} (-\sin^2 \psi)$	0	0	$-(c_{44} - c_{11} + c_{12}) \sin^2 \psi$
$c_{12} \sin^2 \psi$	0	$c_{12} (-\sin^2 \psi)$	0	0	0
0	0	0	0	0	0
0	0	0	0	0	0
$(-c_{11} + c_{44} - c_{12}) \frac{\sin 2\psi}{2}$	$(c_{12} + c_{11} + c_{44}) \frac{\sin 2\psi}{2}$	$-c_{12} \sin 2\psi$	0	0	0

$$\begin{pmatrix} \epsilon_1 \\ \epsilon_2 \\ \epsilon_3 \\ \vdots \\ \epsilon_6 \end{pmatrix} = \begin{pmatrix} 0 \\ 0 \\ 0 \\ 0 \\ 0 \\ 0 \end{pmatrix}$$

$$\sigma_1 \sin^2 \psi = c_{11} \epsilon_1 \sin^2 \psi + c_{12} \epsilon_2 \sin^2 \psi + c_{12} \epsilon_3 \sin^2 \psi$$

$$-\sigma_6 \sin 2\psi = -c_{44} \epsilon_6 \sin 2\psi$$

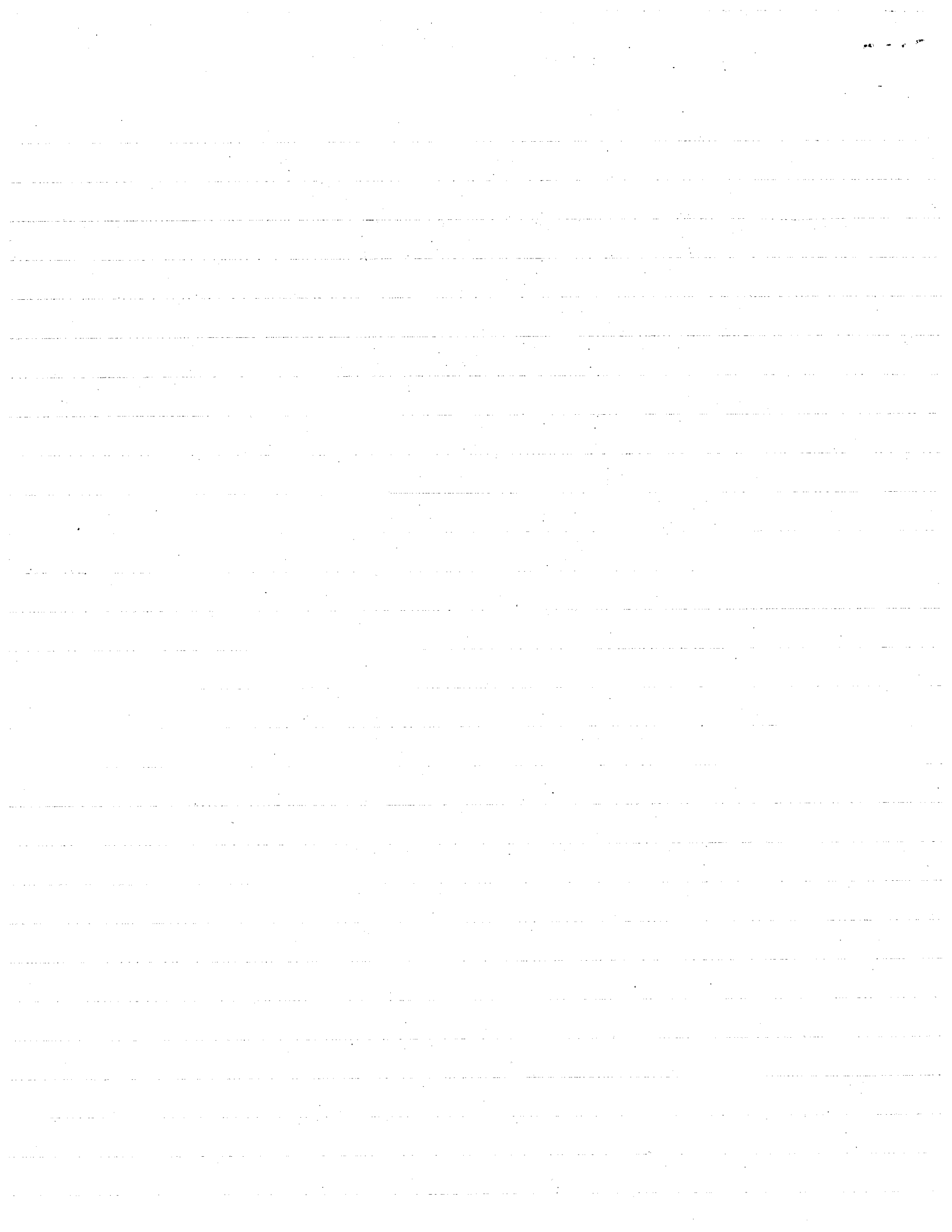
$$+\sigma_2 \cos^2 \psi = c_{12} \epsilon_1 \cos^2 \psi + c_{11} \epsilon_2 \cos^2 \psi + c_{12} \epsilon_3 \cos^2 \psi$$

$$= c_{11} \epsilon_1 \sin^2 \psi + c_{12} \epsilon_2 \sin^2 \psi + c_{12} \epsilon_3 \sin^2 \psi - c_{44} \epsilon_6 \sin 2\psi + c_{12} \epsilon_1 \cos^2 \psi + c_{11} \epsilon_2 \cos^2 \psi + c_{12} \epsilon_3 \cos^2 \psi$$

$$c_{11} \epsilon_1 \sin^2 \psi + c_{12} \epsilon_2 \sin^2 \psi + c_{12} \epsilon_3 \sin^2 \psi + c_{12} \epsilon_1 \cos^2 \psi + c_{11} \epsilon_2 \cos^2 \psi + c_{12} \epsilon_3 \cos^2 \psi - c_{44} \epsilon_6 \sin 2\psi$$

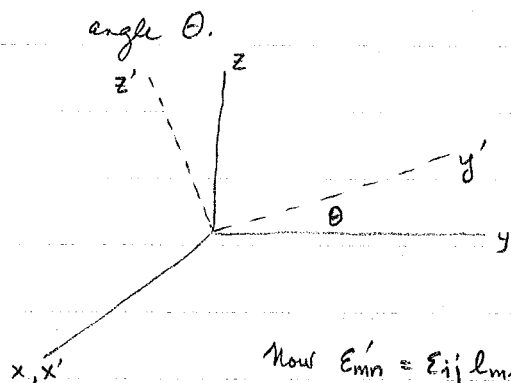
$$\left(-c_{44} - \frac{c_{12}}{2} + \frac{c_{11}}{2}\right) \epsilon_6 \sin 2\psi = 0$$

$$\text{or } \left[c_{44} - \frac{c_{11} + c_{12}}{2} = 0\right]$$



Problem #4

1. Determine the strains $\epsilon'_{ij} = \epsilon'_{ij}(\epsilon_{ij})$ if the yz plane is rotated about the x axis through an angle θ .



	e_x	e_y	e_z
e'_x	1	0	0
e'_y	0	$\cos\theta$	$\sin\theta$
e'_z	0	$-\sin\theta$	$\cos\theta$

and define $e'_m \cdot e_i = \delta_{mi}$

Now $\epsilon'_{mn} = \epsilon_{ij} \delta_{mi} \delta_{nj}$. By applying this using the above transformation matrix we obtain

$$\epsilon'_{xx} = \epsilon_{xx}$$

$$\epsilon'_{yy} = \epsilon_{yy} \cos^2\theta + \epsilon_{zz} \sin^2\theta + \epsilon_{yz} \sin 2\theta$$

$$\epsilon'_{zz} = \epsilon_{yy} \sin^2\theta - \epsilon_{yz} \sin 2\theta + \epsilon_{zz} \cos^2\theta$$

$$\epsilon'_{xy} = \epsilon'_{yx} = \epsilon_{xy} \cos\theta + \epsilon_{xz} \sin\theta$$

$$\epsilon'_{xz} = \epsilon'_{zx} = -\epsilon_{xy} \sin\theta + \epsilon_{xz} \cos\theta$$

$$\epsilon'_{yz} = \epsilon'_{zy} = \frac{\sin 2\theta}{2} (\epsilon_{zz} - \epsilon_{yy}) + \epsilon_{yz} \cos 2\theta$$

For 2D case drop $\epsilon_{xx}, \epsilon_{xy}, \epsilon_{xz}$

2. Consider a cubic crystal whose stiffness matrix looks as follows:

$$\Delta = \begin{bmatrix} c_{11} & c_{12} & c_{12} & 0 & 0 & 0 \\ c_{12} & c_{11} & c_{12} & 0 & 0 & 0 \\ c_{12} & c_{12} & c_{11} & 0 & 0 & 0 \\ 0 & 0 & 0 & c_{44} & 0 & 0 \\ 0 & 0 & 0 & 0 & c_{44} & 0 \\ 0 & 0 & 0 & 0 & 0 & c_{44} \end{bmatrix}$$

Find the relation between c_{11}, c_{12}, c_{44} if after a rotation of the xy plane about the z axis through an angle ψ leaves the stress-strain relations unchanged.

- A. First the rotation matrix looks as follows:

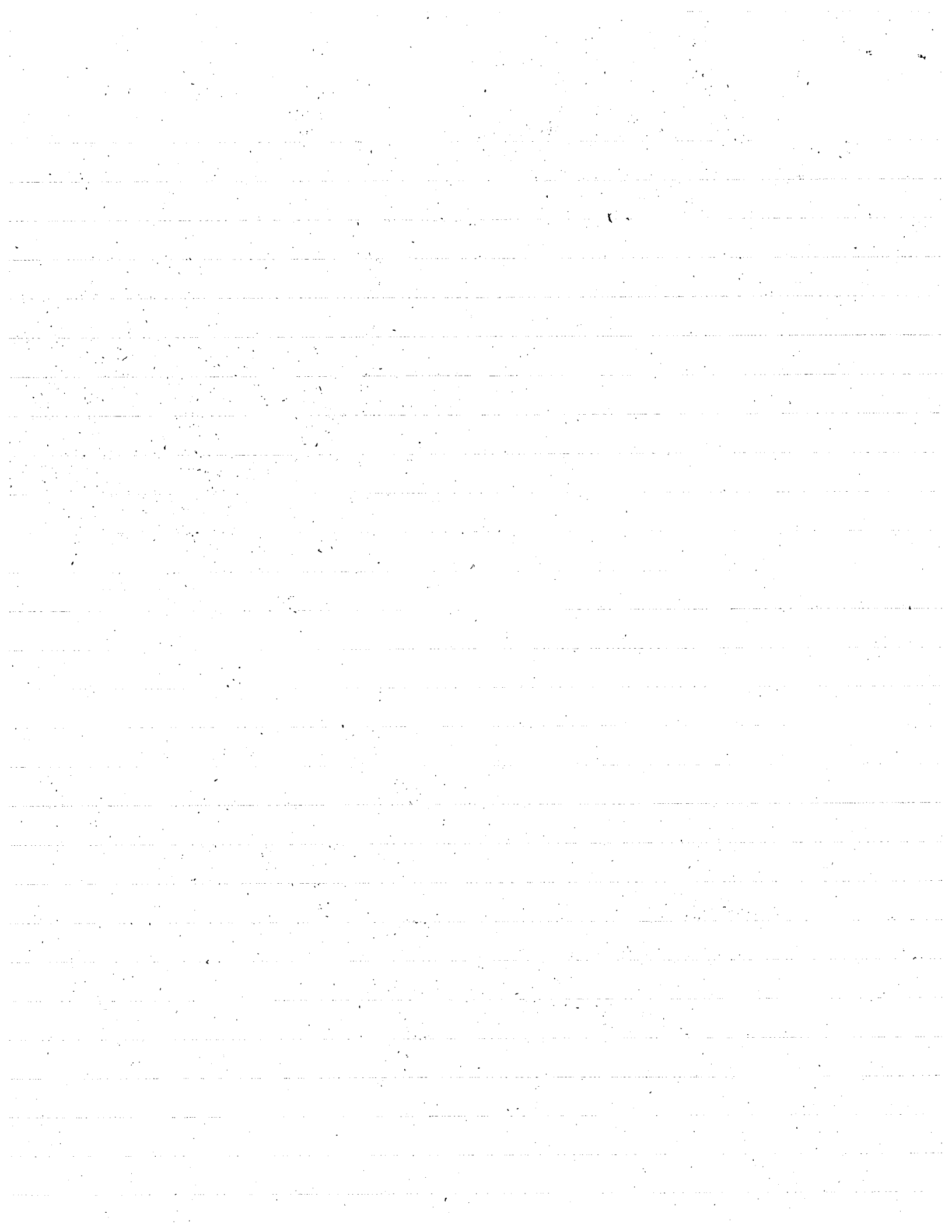
	e_1	e_2	e_3
e'_1	$\cos\theta$	$\sin\theta$	0
e'_2	$-\sin\theta$	$\cos\theta$	0
e'_3	0	0	1

we now remember the stresses and strains so that

$$\sigma_1 = \sigma_{11}, \sigma_2 = \sigma_{22}, \sigma_3 = \sigma_{33}, \sigma_4 = \sigma_{23}, \sigma_5 = \sigma_{13}, \sigma_6 = \sigma_{12}$$

$$\epsilon_1 = \epsilon_{11}, \epsilon_2 = \epsilon_{22}, \epsilon_3 = \epsilon_{33}, \epsilon_4 = 2\epsilon_{23}, \epsilon_5 = 2\epsilon_{13}, \epsilon_6 = 2\epsilon_{12}$$

- B. To find the relations between σ'_i and σ_i , ϵ'_i and ϵ_i we note we could use the equations found in problem 1 if in the double subscripted variables $x \rightarrow 3, y \rightarrow 1, z \rightarrow 2$.



therefore if we now place the result into the single subscripted variables

$$\begin{pmatrix} \sigma_1' \\ \epsilon_1' \end{pmatrix} = \begin{pmatrix} \sigma_1 \\ \epsilon_1 \end{pmatrix} \cos^2 \psi + \begin{pmatrix} \sigma_6 \\ \epsilon_6 \end{pmatrix} \sin 2\psi + \begin{pmatrix} \sigma_2 \\ \epsilon_2 \end{pmatrix} \sin^2 \psi \quad ; \quad \begin{pmatrix} \sigma_2' \\ \epsilon_2' \end{pmatrix} = \begin{pmatrix} \sigma_1 \\ \epsilon_1 \end{pmatrix} \sin^2 \psi - \begin{pmatrix} \sigma_6 \\ \epsilon_6 \end{pmatrix} \sin 2\psi + \begin{pmatrix} \sigma_2 \\ \epsilon_2 \end{pmatrix} \cos^2 \psi$$

$$\begin{pmatrix} \sigma_3' \\ \epsilon_3' \end{pmatrix} = \begin{pmatrix} \sigma_3 \\ \epsilon_3 \end{pmatrix} \quad ; \quad \begin{pmatrix} \sigma_4' \\ \epsilon_4' \end{pmatrix} = -\begin{pmatrix} \sigma_5 \\ \epsilon_5 \end{pmatrix} \sin \psi + \begin{pmatrix} \sigma_4 \\ \epsilon_4 \end{pmatrix} \cos \psi \quad ; \quad \begin{pmatrix} \sigma_5' \\ \epsilon_5' \end{pmatrix} = \begin{pmatrix} \sigma_5 \\ \epsilon_5 \end{pmatrix} \cos \psi + \begin{pmatrix} \sigma_4 \\ \epsilon_4 \end{pmatrix} \sin \psi$$

$$\begin{pmatrix} \sigma_6' \\ \epsilon_6' \end{pmatrix} = -\begin{pmatrix} \sigma_1 \\ \epsilon_1 \end{pmatrix} \sin 2\psi + \begin{pmatrix} \sigma_6 \\ \epsilon_6 \end{pmatrix} \cos 2\psi + \begin{pmatrix} \sigma_2 \\ \epsilon_2 \end{pmatrix} \sin 2\psi$$

c. Now we know that $\sigma_i' = \Delta \epsilon_i'$ and $\sigma_i = \Delta \epsilon_i$ for invariance under this transformation

By manipulation of $\sigma_i' = \delta_{ij} \epsilon_j'$ and $\sigma_i = \delta_{ij} \epsilon_j$ we can get the following for the 6 equations

$$(C_{44} - \frac{C_{11} + C_{12}}{2}) \epsilon_6 \sin 2\psi = 0$$

$$(-C_{44} - \frac{C_{12} + C_{11}}{2}) \epsilon_6 \sin 2\psi = 0$$

$$0 = 0$$

$$0 = 0$$

$$0 = 0$$

$$(\frac{C_{11} - C_{12} - C_{44}}{2}) (\epsilon_2 - \epsilon_1) \sin 2\psi = 0$$

From all these the basic equation that pops out if $\psi \neq 0$, $\epsilon_6 \neq 0$, $\epsilon_1 \neq \epsilon_2$ is

$$\boxed{\frac{1}{2}(C_{11} - C_{12}) = C_{44}}$$

3. Express Lamé's constant λ as $\lambda = \lambda(G, K)$ where G is the shear modulus and K is the bulk modulus.

Express Poisson's ratio ν as $\nu = \nu(E, \mu)$ where μ is the 2nd Lamé constant

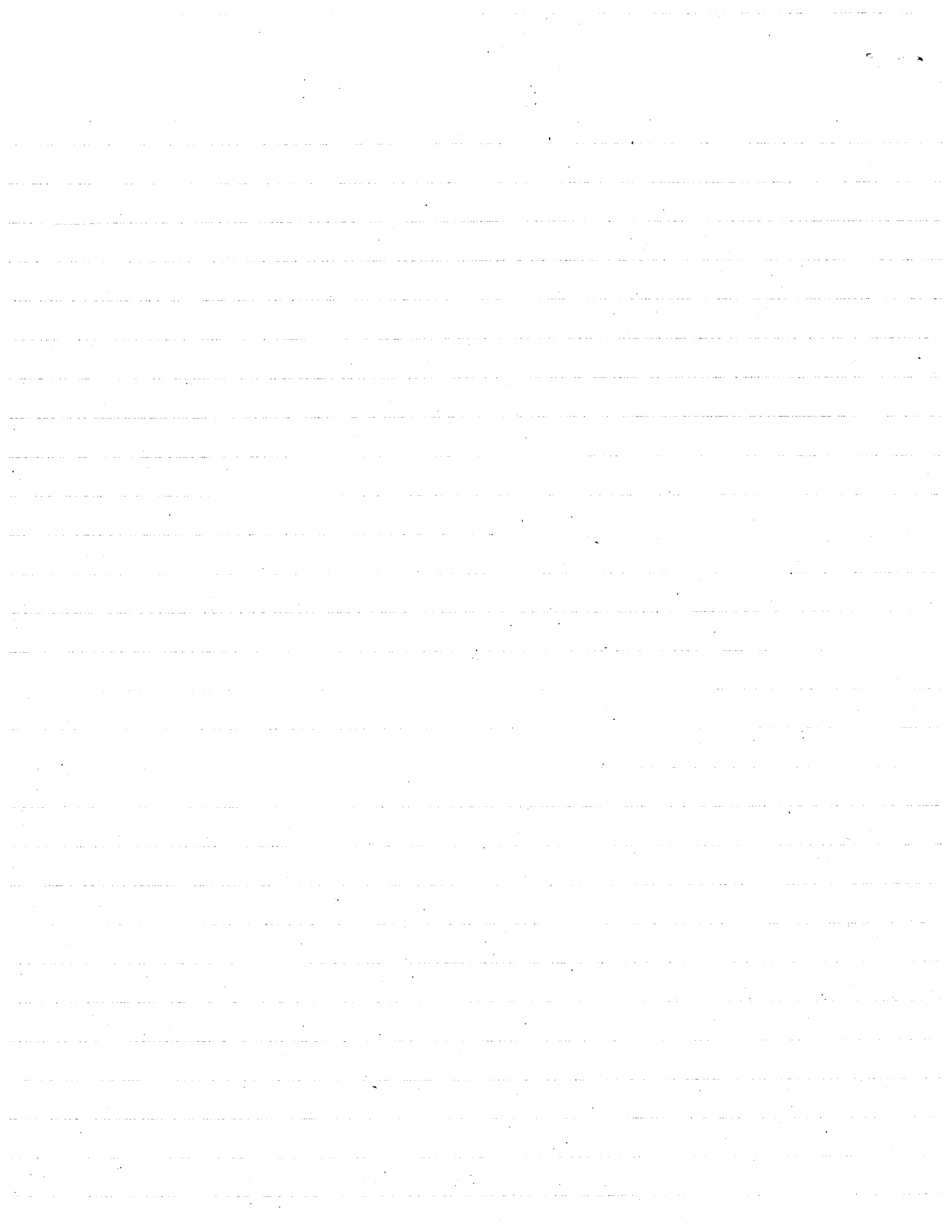
a. We know that $\sigma_{ij} = \lambda \epsilon_{kk} \delta_{ij} + 2\mu \epsilon_{ij}$ now $\sigma_{ii} = 3\lambda \epsilon_{kk} + 2\mu \epsilon_{kk} = (3\lambda + 2\mu) \epsilon_{kk}$

if $\sigma_{ii} = -3p$ (hydrostatic pressure) then $\epsilon_{kk} = \frac{-p}{K} = \frac{-p}{(3\lambda + 2\mu)/3}$

We also know that $\mu = G$ hence $K = (3\lambda + 2G)/3$ or

$$\boxed{\lambda = \frac{3K - 2G}{3}}$$

b. We know that $\epsilon_{ij} = \frac{1+\nu}{E} \sigma_{ij} - \frac{\nu}{E} I_0 \delta_{ij}$ where $I_0 = \sigma_{11} + \sigma_{22} + \sigma_{33}$



When $i \neq j$ $\epsilon_{ij} = \frac{1+\nu}{E} \sigma_{ij}$ or $2\epsilon_{ij} = \gamma_{ij} = \frac{2(1+\nu)}{E} \sigma_{ij}$ and $\frac{2(1+\nu)}{E} = \frac{1}{G} = \frac{1}{\mu}$

Hence $\frac{E}{2\mu} - 1 = \nu$ or

$$\boxed{\nu = \frac{E - 2\mu}{2\mu}}$$

