

DIVISION OF APPLIED MECHANICS
DEPARTMENT OF MECHANICAL ENGINEERING
STANFORD UNIVERSITY

ME 236 WAVES AND VIBRATIONS

Autumn 1979

SUGGESTED REFERENCE MATERIAL

1. J. D. Achenbach - *Wave Propagation in Elastic Solids* (North Holland, 1973)
2. B. A. Auld - *Acoustic Fields and Waves in Solids*, Volumes I and II (Wiley-Interscience, New York, 1973)
3. L. Brillouin - *Wave Propagation in Periodic Structures* (Dover, 1953)
4. L. Brillouin - *Wave Propagation and Group Velocity* (Academic Press, 1960) } work in crystal lattice.
5. W. M. Ewing, W. S. Jardetsky and F. Press - *Elastic Waves in Layered Media* (McGraw-Hill, 1957) Geophysics
6. Y. C. Fung - *Foundations of Solid Mechanics* (Prentice Hall, 1965) general mechanics + deformable bodies
- # 7. H. Kolsky - *Stress Waves in Solids* (Oxford, 1953) Oldest book
- # 8. A. H. E. Love - *Mathematical Theory of Elasticity* (Dover, 1944) one of topics in book
9. P. M. Morse - *Vibration and Sound* (McGraw-Hill, 1948) written from acoustician point of view primarily in air
10. Y.-H. Pao and C.-C. Mow - *Diffraction of Elastic Waves and Dynamic Stress Concentrations* (Crane, Russak, 1973) Special Applic.
11. Lord Rayleigh - *Theory of Sound*, Volumes I and II (Dover, 1945) (First Edition, 1877) Discrete Systems discussed.
12. J. S. Rinehart - *Stress Transients in Solids* (Hyperdynamics, Sante Fe, (197?)) Very large stresses in solids
13. L. A. Viktorov - *Rayleigh and Lamb Waves* (Plenum Press, New York, 1967) Translated - EE approach
surface waves / waves in thin layer.
14. R. J. Wasley - *Stress Wave Propagation in Solids* (M. Dekker, New York, 1973) Same class as 1.
15. G. B. Whitham - *Linear and Nonlinear Waves* (Wiley-Interscience, New York, 1974) Applied Mathematics point of view applied to fluid problem primarily
16. K. F. Graff - *Wave Motion in Elastic Solids* (Ohio State University Press, Columbus, 1975) almost same as one.
17. J. Miklowitz - *The Theory of Elastic Waves and Waveguides*, (North Holland, 1978) guides
18. *Modern Problems in Elastic Wave Propagation* - edited by J. Miklowitz and J.D. Achenbach (Wiley-Interscience, 1978) Proceedings of a conference

I 1. if $u = Ae^{i\phi(x,t)}$ then $\nabla_x \phi = k$ where $\nabla_x = \frac{\partial}{\partial x_i}$
 2. $\frac{\partial \phi}{\partial t} = -\omega$ and $\nabla_k \omega = C_g$ (group veloc.) $\nabla_k = \frac{\partial}{\partial k_i}$ } $\phi = k \cdot x - \omega t$
 3. $\frac{\omega}{|k|} = C_p$ (phase velocity) $C_k = \frac{|k|}{\omega}$

4. ENERGY propagates with C_g ; dispersion relation relates ω & k ; DISTURBANCE propagates with C_p
 II One dimensional motion $C^2 = E/\rho$ $\frac{\partial^2 u}{\partial x^2} - \frac{1}{C^2} \frac{\partial^2 u}{\partial t^2} = 0$ $u = f(x-ct) + g(x+ct)$ $x \pm ct$ are characteristic lines

along this line $E u_x = \sigma \therefore E \frac{\partial^3 u}{\partial x^3} - \frac{1}{C^2} E \frac{\partial^3 u}{\partial t^2 \partial x} = 0 \Rightarrow \frac{\partial^2 \sigma}{\partial x^2} - \frac{1}{C^2} \frac{\partial^2 \sigma}{\partial t^2} = 0$ $\sigma = -p c u$

DISPERSION occurs due to inhomogeneity, boundaries, inelasticity, nonlinearities.

III Period $T = \frac{2\pi}{\omega} = \frac{2\pi}{kC}$ where ω is circular freq; k is wave number; C is 1-D phase veloc.
 wavelength $\lambda = \frac{2\pi}{k}$

IV Eqs of motion w/o body force $\sigma_{ij,j} = \rho \ddot{u}_i$ $\nabla \cdot \sigma = \rho \ddot{u}$
 Strain - Displ $\epsilon_{ij} = \frac{1}{2}(u_{i,j} + u_{j,i})$ $\epsilon = \frac{1}{2}(\nabla u + u \nabla)$
 Strain - Stress Law $\sigma_{ij} = \lambda \delta_{ij} \epsilon_{kk} + 2\mu \epsilon_{ij}$ $\sigma = \lambda \text{II} \Delta + 2\mu \epsilon$
 Eqs of Motion - displ formulation $\mu u_{i,jj} + (\lambda + \mu) u_{j,ji} = \rho \ddot{u}_i$ $\mu \nabla^2 u + (\lambda + \mu) \nabla(\nabla \cdot u) = \rho \ddot{u}$

Wave Eqn on dilatation $\nabla \cdot u = \Delta$ $(\lambda + 2\mu) \nabla^2(\Delta) = \rho \frac{\partial^2}{\partial t^2}(\Delta)$ no shear
 rotation $\omega = \nabla \times u$ $\mu \nabla^2 \omega = \rho \frac{\partial^2}{\partial t^2} \omega$ no dilatation
 $\rightarrow u = \nabla \phi + \nabla \times H$ with $\nabla \cdot H = 0$ (Helmholtz Decomposition)
 $(\lambda + 2\mu) \nabla^2 \phi = \rho \ddot{\phi}$ $\mu \nabla^2 H = \rho \ddot{H}$
 thus

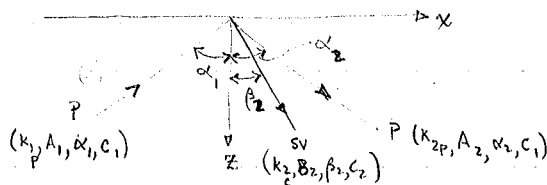
$W = -\frac{1}{4\pi} \iint \frac{u(\xi)}{|x - \xi|} dV_\xi$ $\phi = \nabla \cdot W$ $H = -\nabla \times W$ $|x - \xi| = [(x_i - \xi_i)(x_i - \xi_i)]^{1/2}$
 $dV_\xi = d\xi_1 d\xi_2 d\xi_3$

V PLANE waves in ∞ medium

$u = A d e^{i k \cdot (w \cdot r - ct)}$ if $d \parallel w$ then $c_1 = \left(\frac{\lambda + 2\mu}{\rho}\right)^{1/2}$ P waves
 if $d \perp w$ then $c_2 = \left(\frac{\mu}{\rho}\right)^{1/2}$ SH, SV "

if wave amplitude is not constant then it is an INHOMOGENEOUS wave i.e. the vector w is complex
 w - is a unit vector in direction of propagation d is a unit vector in the direction of motion

VI Reflections in an elastic half space at traction free surfaces



$k_1 = k_2 = k_p$ $\frac{k_{z1}}{k_{z2}} = \frac{c_1}{c_2} = \frac{\sin \alpha_2}{\sin \beta_2}$ and $k_2 \sin \alpha_2 = k$
 $\alpha_1 = \alpha_2$, $k = k_1 \sin \alpha_1$ is the wave number of wave propag. along $z=0$
 $c = \omega/k = c_1 / \sin \alpha_1$ phase velocity at surface

Love waves are dispersive since the eqn $f(\omega, k) = 0 \Rightarrow$

$$\tan \left\{ \left[\left(\frac{c}{c_2} \right)^2 - 1 \right]^{1/2} kH \right\} - \frac{\mu}{\mu_B} \frac{[1 - c^2/c_2^2]^{1/2} k}{[1 - c^2/c_2^2]^{1/2} k} = 0$$

X. Waves in an elastic layer in plane strain take $\phi = f e^{ik(x-ct)}$ $H_3 = h e^{ik_3(x-ct)}$

See pg 20-21

$u_x(u_y)$ is sym. in x_2 if it contains $\cos y$ ($\sin y$) Extens.
 $u_x(u_y)$ is antisym. in x_2 " " " $\sin y$ ($\cos y$) Flexural

these give rise to Rayleigh - Lamb Freq. Eqns.

See pg 22 for table on extensional / Flexural motions

Propagation of disturbance created by some phenomena.

We may look at what happens when entire structure is impressed with a dynamic load or locally how the stress dynamic changes with the propagation of the waves.

Objective: to familiarize oneself where wave prog can occur not so much on how to solve the problems.

Wave propagation in continuous systems

Let $w(x, t)$ be the dependent variables. x = independent variables (space)

Let $L[w(x, t)] = 0$ where L is the linear operator characterizing the syste.

The solution will be of the form $w = A e^{i\phi(x, t)}$ where A is the amplitude and ϕ being the phase.

since $\phi = \phi(x, t)$ we can define $\nabla_x \phi = k$ = wave number

also $\frac{\partial \phi}{\partial t} = -\omega$ the frequency

Thus $L[w] = 0 \iff f(\omega, k) = 0$ this is the dispersion relation which plays a central relation in wave propagation.

We can now form $\nabla_k \omega = c_g$ which is the group velocity. It is ^{of} importance.

since energy of syste. will be propagated with the group velocity

- we can define $\frac{\omega}{|k|} m_k = c_p$ which is the phase velocity. $m_k = \frac{|k|}{|k|^2}$
 $\frac{\omega m_k}{|k|^2} = \frac{\omega |k|}{k^2} = c_p$

Example: one dim proble - if: $w(x, t) = U \cos \phi(x, t)$

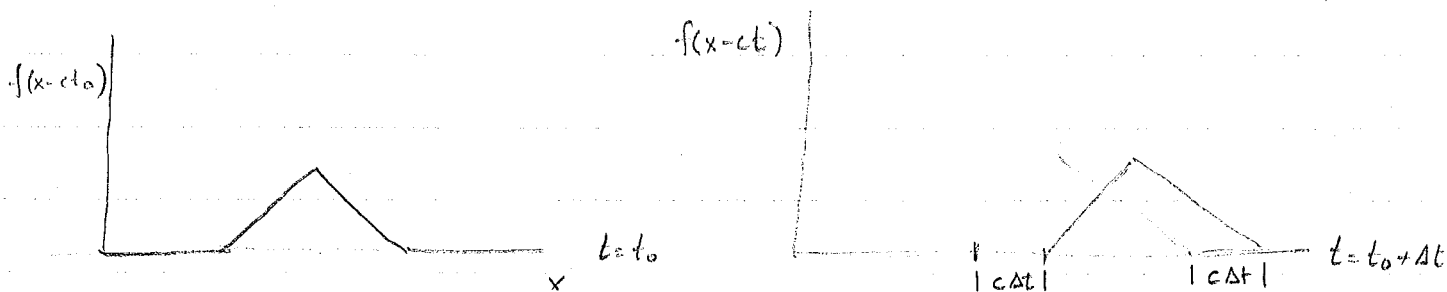
$$k = \frac{\partial \phi}{\partial x} \quad \omega = -\frac{\partial \phi}{\partial t} \quad f(\omega, k) = 0 \Rightarrow \omega = g(k) \quad \text{some times}$$

$$\omega = g(k) \Rightarrow -\frac{\partial \phi}{\partial t} = g\left(\frac{\partial \phi}{\partial x}\right)$$

$$d\phi = \frac{\partial \phi}{\partial x} dx + \frac{\partial \phi}{\partial t} dt \quad : \text{ if } \frac{\partial \phi}{\partial x}, \frac{\partial \phi}{\partial t} \text{ are const. then } \phi = \frac{\partial \phi}{\partial x} x + \frac{\partial \phi}{\partial t} t$$

$$\text{then } \boxed{\phi = kx - \omega t} \quad \text{or } \frac{\phi}{k} = \tilde{\phi} = x - \frac{\omega}{k} t = x - c_p t \quad c \text{ is phase velocity}$$

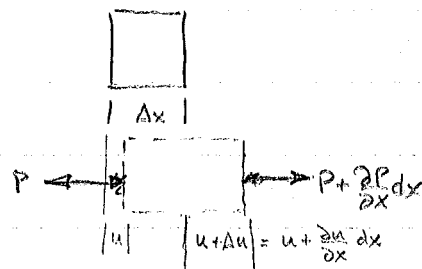
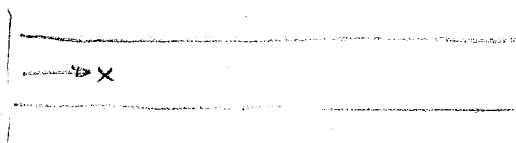
if we have $f(x - ct)$ which is simple wave fn. Now @ $t = t_0$



we can use the results at $t=t_0 \Rightarrow$ wave moves to the right with velocity c

Notation: phase velocity - no subscript; group velocity - $()_g$. c dictates the speed at which disturbance propagates.

Example: Uniform bar; cross-section remain plane & move in x direction only
one-dimensional motion



Eqn of mot: $\sum F = ma$

$$-\frac{\partial P}{\partial x} \Delta x = \rho A \frac{\partial^2 u}{\partial t^2} \Delta x; \text{ but } P = \sigma A = E \epsilon A = E \frac{\partial u}{\partial x} A$$

$$\text{then } E \frac{\partial^2 u}{\partial x^2} = \rho \frac{\partial^2 u}{\partial t^2} \text{ or } \frac{\partial^2 \sigma}{\partial x^2} = \rho/E \frac{\partial^2 \sigma}{\partial t^2} \text{ define } E/\rho = c^2 \text{ or } \left[c^2 \frac{\partial^2 u}{\partial x^2} - \frac{\partial^2 u}{\partial t^2} = 0 \right]$$

(take $\frac{\partial}{\partial x}$ of $E u_{xx} = \rho u_{tt}$ and use $E u_x = \sigma$ to get)

1-D wave eqn.

$$\text{let } v_1 = x - ct \quad v_2 = x + ct$$

$$\frac{\partial u}{\partial x} = \frac{\partial u}{\partial v_1} + \frac{\partial u}{\partial v_2}$$

$$\frac{\partial u}{\partial t} = -c \frac{\partial u}{\partial v_1} + c \frac{\partial u}{\partial v_2}$$

$$\frac{\partial^2 u}{\partial x^2} = \frac{\partial^2 u}{\partial v_1^2} + 2 \frac{\partial^2 u}{\partial v_1 \partial v_2} + \frac{\partial^2 u}{\partial v_2^2}$$

$$\frac{\partial^2 u}{\partial t^2} = c^2 \frac{\partial^2 u}{\partial v_1^2} - 2c^2 \frac{\partial^2 u}{\partial v_1 \partial v_2} + c^2 \frac{\partial^2 u}{\partial v_2^2}$$

$$c^2 \frac{\partial^2 u}{\partial x^2} - \frac{\partial^2 u}{\partial t^2} = 4c^2 \frac{\partial^2 u}{\partial v_1 \partial v_2} = 0 \quad \text{or} \quad \left[\frac{\partial^2 u}{\partial v_1 \partial v_2} = 0 \right]$$

$$\frac{\partial}{\partial v_1} \left(\frac{\partial u}{\partial v_2} \right) = 0$$

$$\frac{\partial u}{\partial v_2} = \psi(v_2)$$

$$u = \int \psi(v_2) dv_2 + f(v_1)$$

$$g(v_2) = f(v_1) + g(v_2)$$

then

$$\boxed{u = f(x-ct) + g(x+ct)}$$

wave to right wave to left

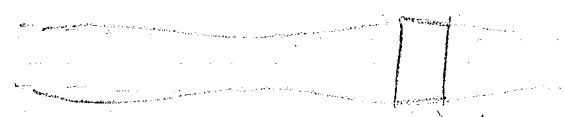
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Now assume $f(x-ct) = A \cos k(x-ct) = A \cos(kx - \omega t)$
 or in more compact form $f(x-ct) = A e^{i(kx - \omega t)}$

Put into wave eqn then $\Rightarrow c^2 = E/\rho$: here c is constant & not a fn of k ,
 as is the case in some systems. When c is a fn of k this is dispersion.

Dispersion might be due to nonlinear effects, inhomogeneity or boundaries or inelasticity

When deriving the wave eqn, we took an elementary theory - we expect that due to boundaries $c = c(k)$ but we don't get it.



There is shear which we don't take into account

Particle velocity

$$u(x,t) = A e^{i k(x-ct)} \quad \frac{\partial u}{\partial x} = A i k e^{i k(x-ct)}$$

$$\sigma = E \frac{\partial u}{\partial x} = E A i k e^{i k(x-ct)}$$

$$\frac{\partial u}{\partial t} = \dot{u} = -A i k c e^{i k(x-ct)} \Rightarrow -\frac{E}{c} \frac{\partial u}{\partial t} = \sigma = -\frac{E}{c} \rho \dot{u} = -\rho \dot{u}$$

$\sigma = -\rho c^2 \frac{\partial u}{\partial x}$ for a harmonic fn.

Period $T = \frac{2\pi}{\omega} = \frac{2\pi}{kc}$

Wave length $\lambda = \frac{2\pi}{k}$ wave number represents the no of wave length in period of 2π

Starting with an extended solid w/ isotropic, homogeneous, elastic properties

Recall that the stress eqns of motion w/o body force

$$\sigma_{ij,j} = \rho \ddot{u}_i \quad \nabla \cdot \sigma = \rho \ddot{u}$$

Strain - Disp relations

$$\epsilon_{ij} = \frac{1}{2} (u_{i,j} + u_{j,i}) \quad \mathcal{E} = \frac{1}{2} (\nabla u + u \nabla)$$

Hooke's Law

$$\sigma_{ij} = \lambda \delta_{ij} \epsilon_{kk} + 2\mu \epsilon_{ij}$$

$$\sigma = \lambda \Delta \mathbb{I} + 2\mu \epsilon$$

Take Hooke's law & put in there the strain-displ relations. Take result & put into eqns of motion. Thus:

$$\mu u_{i,jj} + (\lambda + \mu) u_{j,ji} = \rho \ddot{u}_i$$

$$\boxed{\mu \nabla^2 \mathbf{u} + (\mu + \lambda) \nabla (\nabla \cdot \mathbf{u}) = \rho \ddot{\mathbf{u}}} \quad (*)$$

Reduction to wave eqns.

(1) Take $\nabla \cdot$ of the (*) eqn.

$$\mu \nabla^2 (\nabla \cdot \mathbf{u}) + (\mu + \lambda) \nabla^2 (\nabla \cdot \mathbf{u}) = \rho \frac{\partial^2}{\partial t^2} (\nabla \cdot \mathbf{u})$$

thus $\nabla^2 [\mu \nabla \cdot \mathbf{u} + (\lambda + \mu) \nabla \cdot \mathbf{u}] = \rho \frac{\partial^2}{\partial t^2} (\nabla \cdot \mathbf{u}) \quad \Delta = \nabla \cdot \mathbf{u}$

or $\boxed{[(\lambda + 2\mu) \nabla^2 (\Delta)] = \rho \frac{\partial^2}{\partial t^2} \Delta}$ 3-D wave eqn on the dilatation.
→ does not involve shear

(2) Take $\nabla \times$ the (*) eqn.

$$\mu \nabla^2 (\nabla \times \mathbf{u}) + (\mu + \lambda) \nabla \times \nabla (\nabla \cdot \mathbf{u}) = \rho \frac{\partial^2}{\partial t^2} (\nabla \times \mathbf{u})$$

this is 3-D wave eqn on the rotation $\omega = \nabla \times \mathbf{u} : \boxed{\mu \nabla^2 \omega = \rho \frac{\partial^2}{\partial t^2} \omega}$

dilatation is uncoupled from rotation. → does not involve volume change.

Reduction by Helmholtz resolution w/ gauge condition

$$\mathbf{u} = \nabla \phi + \nabla \times \mathbf{H} \quad \text{also take } \nabla \cdot \mathbf{H} = 0 \text{ to take indeterminacy on } \phi, \mathbf{H} \text{ are called potentials of } \mathbf{u}$$

$\nabla \phi$ is the lamellar part of $\mathbf{u}$

$\nabla \times \mathbf{H}$ is the solenoidal part of $\mathbf{u}$

put $\mathbf{u}$ into (*) → $\rho \nabla \ddot{\phi} + \rho \nabla \times \ddot{\mathbf{H}} = \mu \nabla^2 (\nabla \phi) + (\lambda + \mu) \nabla^2 (\nabla \phi) + \mu \nabla^2 (\nabla \times \mathbf{H}) + (\lambda + \mu) \nabla (\nabla \cdot \nabla \times \mathbf{H}) = 0$ by gauge

$$\therefore \nabla [(\lambda + 2\mu) \nabla^2 \phi - \rho \ddot{\phi}] + \nabla \times [\mu \nabla^2 \mathbf{H} - \rho \ddot{\mathbf{H}}] = 0$$

$$\Rightarrow (\lambda + 2\mu) \nabla^2 \phi = \rho \ddot{\phi} \quad \& \quad \mu \nabla^2 \mathbf{H} = \rho \ddot{\mathbf{H}}$$

Given u how does one find ϕ, H
define

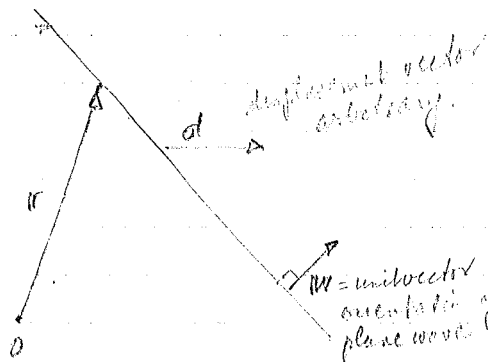
$$W(x) = \frac{1}{4\pi} \iint \frac{u(\xi)}{|x - \xi|} dV_\xi$$

$$|x - \xi| = [(x_1 - \xi_1)^2 + (x_2 - \xi_2)^2 + (x_3 - \xi_3)^2]^{1/2}$$

then $\phi = V \cdot W$ $H = -V \times W$

Plane wave solutions that are harmonic in an ∞ body.

Given plane wave



$$W = A d e^{ik(W \cdot r - ct)}$$

$$u_x = A d_x e^{ik(W_x x + W_y y + W_z z - ct)}$$

$$\frac{\partial u_x}{\partial x} = ik W_x A d_x e^{ik(W \cdot r - ct)}$$

$$\nabla \cdot u = ik A (W \cdot d) e^{ik(W \cdot r - ct)} = \frac{\partial u_i}{\partial x_i}$$

$$\nabla \times u = ik A (W \times d) e^{ik(W \cdot r - ct)}$$

$$(\nabla \cdot \nabla) u = \nabla^2 u = -k^2 A d e^{ik(W \cdot r - ct)} \quad \ddot{u} = -k^2 c^2 A d e^{ik(W \cdot r - ct)}$$

$$\nabla(\nabla \cdot u) = -k^2 A (W \cdot d) e^{ik(W \cdot r - ct)} \quad W = \nabla [ik A (W \cdot d) e^{ik(W \cdot r - ct)}] = ik W [\nabla \cdot u]$$

$$\nabla u = ik W [u]$$

put into law of motion $\Rightarrow$ vector eqn. is defined

ie
$$\mu d + (\lambda + \mu)(W \cdot d) W = \rho c^2 d \quad \left. \begin{array}{l} \text{either } W \parallel d \\ \text{or } W \cdot d = 0 \end{array} \right\} \text{imply that } \mu = \rho c^2$$

Look at these 2 cases

Case 1) $d \parallel W$

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 $\mu = \rho c^2 \quad (\lambda + \mu) = 0$ since $W \parallel d \Rightarrow W \cdot d = d \cdot d = 1$
 $\therefore c_1 = \left(\frac{\lambda + 2\mu}{\rho} \right)^{1/2} \Rightarrow \nabla \times u = 0$ irrotational wave

2)

$\mu = \rho c^2$ or $c_2 = (\mu/\rho)^{1/2} \Rightarrow \nabla \cdot u = 0$ solenoidal wave

suppose we want to find $\nabla \cdot u$: $\nabla \cdot u = W \cdot (\lambda \nabla \cdot u \mathbb{I} + 2\mu \nabla u)$
 $= \lambda (\nabla \cdot u) W + \mu W \cdot (\nabla u + u \nabla)$
 $W \cdot \nabla u = ik A d e^{ik(W \cdot r - ct)} = W \cdot ik W [u] = ik u$
 $W \cdot u \nabla = ik A (W \cdot d) W e^{ik(W \cdot r - ct)} = W \cdot \nabla u = W \cdot u [ik W] = ik A (W \cdot d) W e^{ik(W \cdot r - ct)}$
Thus
$$W \cdot u = ik A e^{ik(W \cdot r - ct)} [\lambda (W \cdot d) W + \mu d + \mu (W \cdot d) W]$$

for $w = A d e^{ik(w \cdot r - ct)}$ $\mathbb{I}_w = w \cdot \mathbb{I} = i k A e^{ik(w \cdot r - ct)} \left[\mu d + (\lambda + \mu) (w \cdot d) w \right]$

- d when $w \parallel d$ $\mathbb{I}_w = (\lambda + 2\mu) i k A d e^{ik(w \cdot r - ct)}$ is normal to plane of wave; since $\mathbb{I}$ is parallel & pressure stress.
- e) $w \perp d$ $\mathbb{I}_w = \mu i k A e^{ik(w \cdot r - ct)} d$ is in the plane of wave; but $\mathbb{I}$ is parallel & shear stress.

- 1) is known as pressure wave ("Pushing", "Tuck") P wave.
- 2) is known as shear wave ("Shear", "Shake wave", "Sneaking") S wave.

$\frac{c_1^2}{c_2^2} = \frac{\lambda + 2\mu}{\mu} = \frac{2(1-\nu)}{1-2\nu}$ for $\nu=0$ $\frac{c_1^2}{c_2^2} = 2$; $\frac{c_1^2}{c_2^2} = 3.5$ $\nu=0.3$
 for $\nu=0.5$ $\frac{c_1^2}{c_2^2} = \infty$

if we extend results to $w = W e^{ik(w \cdot r - ct)}$ where W is complex

$w = A d e^{-k(w \cdot r)} e^{ik(w \cdot r - ct)}$

Amplitude decays (in space) if $w \cdot r > 0$ This is a propagating wave

for inhomog. wave the amplitude is not constant i.e. w is complex.

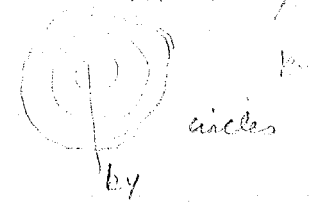
Consider $w' \cdot w' = w'' \cdot w'' = 1$ and $w' \cdot w'' = 0 \Rightarrow w \cdot w = 1$

- the planes of const. amplitude $\Rightarrow w' \cdot r = \text{constant } c_1$
 for fixed time & locs of constant phase $w' \cdot r = c_2$ (const)
 planes of constant phase are $\perp$ to planes of constant amplitude
 amplitude is constant in direction of prop but varies exponentially in $\perp$ direction

Look at 2-D wave Eqn: $\frac{\partial^2 \phi}{\partial x^2} + \frac{\partial^2 \phi}{\partial y^2} = \frac{1}{c^2} \frac{\partial^2 \phi}{\partial t^2}$
 Case 1: const amplitude

if we let $\phi = \text{const } e^{i(kx - \omega t)}$ then for $kx = k_x x$ & $\omega = k c$

let us find sol: $kx^2 + ky^2 = \frac{\omega^2}{c^2} = k^2$
 put it to DE to get ϕ



Thus const amplitude waves are circular emanating from center i.e. w is real

Case 2 $k = k_x \hat{e}_x$ only
 $\omega = \text{complex}$

Another ex. for $\phi = \Phi(y) e^{i(k_x x - \omega t)}$ $\Delta \phi - \frac{\ddot{\phi}}{c^2} = 0$

$$\left[-k_x^2 \Phi + \Phi'' + \frac{\omega^2}{c^2} \Phi \right] e^{i(k_x x - \omega t)} = 0$$

$$\Rightarrow \left(\frac{\omega^2}{c^2} - k_x^2 \right) \Phi + \Phi'' = 0 \quad \text{ODE for } y \text{ direction}$$

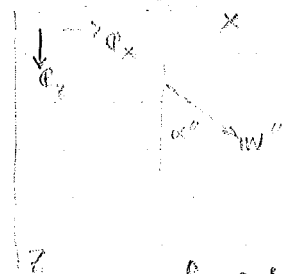
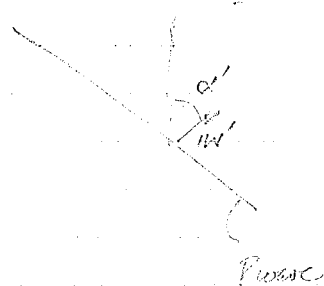
if $\alpha^2 > 0$ soln: trig fun.

if $\alpha^2 = 0$ soln: linear $\Phi = A + By$ $\phi = (A + By) e^{i(k_x x - \omega t)}$

if $\alpha^2 < 0$ soln: exponential f. total sol is not dependent on Bdy condition

Elastic Half Space - Plane waves reflecting off of a free surface $\equiv$ free surface

1) Reflection



for a p-wave $\omega \parallel d$

thus $w' = d' = -\hat{e}_z \cos \alpha' + \hat{e}_x \sin \alpha'$
 $w = A' d' e^{ik'(\omega' x - c_1 t)}$

now $\sigma_{ij} = \left[\lambda (w_{j,i} + w_{i,j}) + \mu (w_{j,i} + w_{i,j}) \right] x$
 $i k A e^{ik(\omega x - c t)}$

for σ_{zz}

$$\sigma_{zz} = [\lambda \delta_{ij} + 2\mu (w_{i,j} + w_{j,i})] i k' A' e^{ik'(\omega' x - c_1 t)}$$

$$\sigma_{zz} = [\lambda + 2\mu \cos^2 \alpha'] i k' A' e^{ik'(\omega' x - c_1 t)}$$

$$\sigma_{zx} = -\mu \sin 2\alpha' i k' A' e^{ik'(\omega' x - c_1 t)}$$

$$\sigma_{zy} = 0 \quad \text{since plane problem}$$

on bdy $z=0 \Rightarrow \sigma_{zx}, \sigma_{zz} = 0$ we must have reflected waves at surface to annihilate surface traction

Assume reflected P-wave.

$$u_1'' = A'' d'' e^{ik''(w'' \cdot r - c_1 t)}$$

$$w'' = d'' = c_2 \cos \alpha'' + c_2 \sin \alpha''$$

$$\sigma_{zz}'' = (\lambda + 2\mu \cos^2 \alpha'') i k'' A'' e^{ik''(x \sin \alpha'' - c_1 t)}$$

$$\sigma_{zx}'' = \mu \sin 2\alpha'' i k'' A'' e^{ik''(x \sin \alpha'' - c_1 t)} \quad \left. \begin{array}{l} \sigma_{zy}'' = 0 \end{array} \right\} \text{ on surface}$$

Continuation of the above.

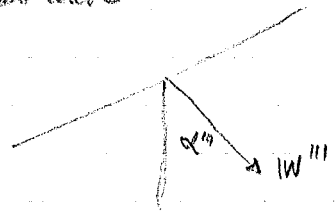
10/9/79
 $A'' + A' = 0 \Rightarrow A'' - A' = -2A' \neq 0$

the only way tractions can be annihilated $\Rightarrow$ if ① $\sigma_{zz} = 0 \Rightarrow \sigma_{zx} \neq 0$
 or if ② $\sigma_{zx} = 0 \Rightarrow \sigma_{zz} \neq 0$
 $\Rightarrow A' - A'' = 0 \Rightarrow A' + A'' = 2A' \neq 0$

thus reflected wave cannot be only a P wave but must also consist of a reflected S wave.

$\therefore$ for a shear wave

2 possible waves SH is out of plane
 SV is same plane as I



$$u_1''' = A''' d''' e^{ik'''(w''' \cdot r - c_2 t)}$$

$$w''' = c_2 \cos \alpha''' + c_2 \sin \alpha'''$$

$$d''' = -c_2 \cos \alpha''' + c_2 \sin \alpha'''$$

at $z=0$

$$\sigma_{zz}''' = \mu \sin 2\alpha''' i k''' A''' e^{ik'''(x \sin \alpha''' - c_2 t)}$$

$$\sigma_{zx}''' = -\mu \cos 2\alpha''' i k''' A''' e^{ik'''(x \sin \alpha''' - c_2 t)}$$

$$\sigma_{zy}''' = 0$$

Since it must be true that $\forall x, t \quad \sigma_{zz} = \sigma_{zx} = 0 \Rightarrow k'(x \sin \alpha' - c_1 t) = k''(x \sin \alpha'' - c_1 t) = k'''(x \sin \alpha''' - c_2 t)$

$$k' = k'' \quad \& \quad \frac{k'''}{k'} = \frac{c_1}{c_2} \quad \alpha' = \alpha'' \quad \frac{\sin \alpha'''}{\sin \alpha'} = \frac{k'}{k'''} = \frac{c_2}{c_1}$$

$$\left. \begin{array}{l} \text{Let } \alpha' = \alpha'' = \alpha \\ \alpha''' = \beta \end{array} \right\} \text{ since } c_1 > c_2 \quad \alpha > \beta$$

$$\therefore \sigma_{zz} = i e^{ik(x \sin \alpha - c_1 t)} \left[(\lambda + 2\mu \cos^2 \alpha) (kA' + kA'') + \mu k \frac{c_1}{c_2} \sin 2\beta A''' \right] = 0$$

$$\sigma_{zx} = i e^{ik(x \sin \alpha - c_1 t)} \left[-\mu \sin 2\alpha kA' + \mu \sin 2\alpha kA'' - \mu k \frac{c_1}{c_2} \cos 2\beta A''' \right] = 0$$

normally A', c_1, α are given $\Rightarrow$ we must write A'' & A''' in terms of A' & the compat. conditions given β in terms of α, c_1, c_2 .

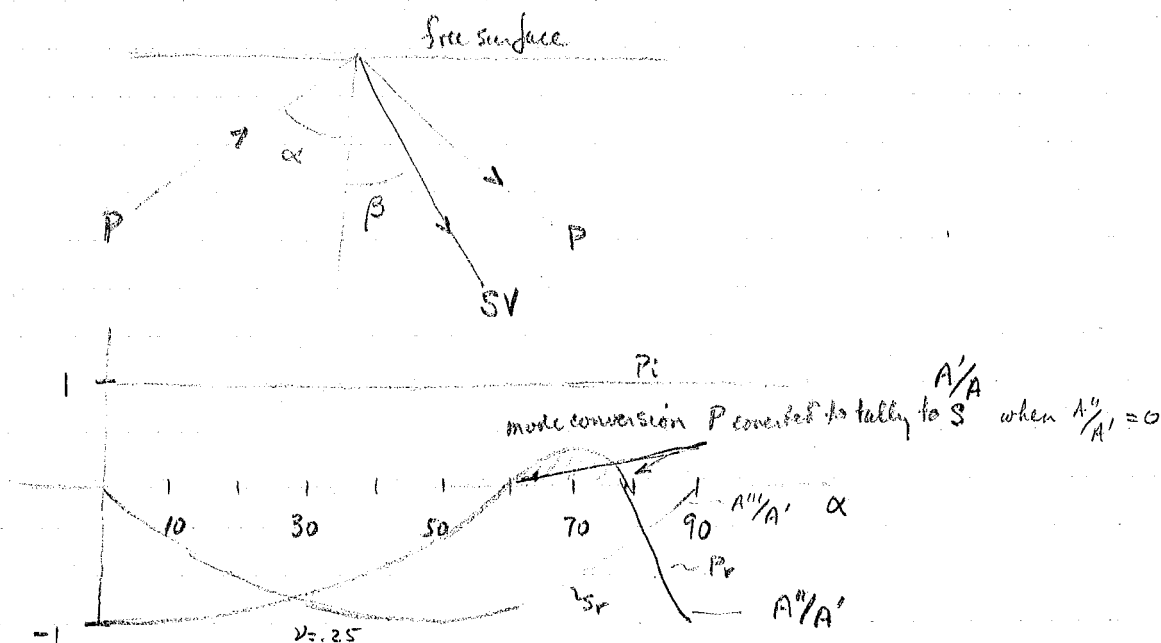
$$\therefore \frac{\lambda + 2\mu \cos^2 \alpha}{\rho} = c_1^2 - \frac{2\mu \sin^2 \alpha}{\rho} = c_1^2 - 2c_2^2 \sin^2 \alpha = c_1^2 - 2c_1^2 \sin^2 \beta = c_1^2 \cos 2\beta$$

$$\therefore \frac{A''}{A'} = \frac{c_2^2 \sin 2\alpha \sin 2\beta - c_1^2 \cos 2\beta}{c_2^2 \sin 2\alpha \sin 2\beta + c_1^2 \cos 2\beta}$$

$$\frac{A'''}{A'} = - \frac{2c_1 c_2 \sin 2\alpha \cos 2\beta}{c_2^2 \sin 2\alpha \sin 2\beta + c_1^2 \cos 2\beta}$$

$$\left| \left(\frac{A''}{A'} \right)^2 + \frac{\sin 2\beta}{\sin 2\alpha} \left(\frac{A'''}{A'} \right)^2 = 1 \right|$$

Eqn of an ellipse



for $\alpha = 0$ normal incidence

Special case

$$w' = -e_z \quad w'' = e_z \quad w''' = 0$$

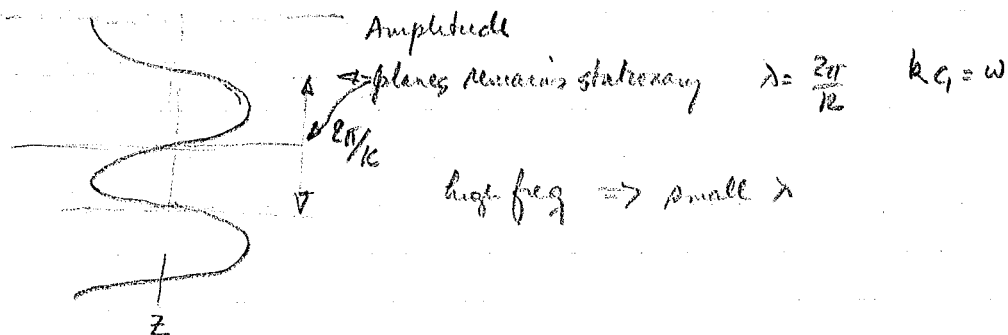
$$d'' = -d' \Rightarrow A''d'' = (A')(-d') = A'd'$$

$$u' = d'A'e^{ik(z-ct)}$$

$$u'' = d'A'e^{ik(z-ct)}$$

$$u = u' + u'' = d'A' [e^{ik(z-ct)} + e^{ik(z-ct)}] = 2d'A'e^{-ikct} \cos kz$$

$\Rightarrow$ standing vibrations motion not fn of x
waves



$\alpha = 90^\circ$ (grazing incidence) is more difficult; we will defer until after full & systematic development is made.

Reflection of waves using potentials

Recall $u = \nabla \phi + \nabla \times H$ w/ $\nabla \cdot H = 0$ & the from single Eqn of motion

$$\nabla^2 \phi = \frac{1}{c_1^2} \frac{\partial^2 \phi}{\partial t^2} \quad \nabla^2 H = \frac{1}{c_2^2} \frac{\partial^2 H}{\partial t^2}$$

$$\nabla \cdot H = 0 \Rightarrow \left(\frac{\partial H_x}{\partial x} + \frac{\partial H_y}{\partial y} + \frac{\partial H_z}{\partial z} \right) = 0$$

$$\nabla \times H = \left(\frac{\partial H_z}{\partial y} - \frac{\partial H_y}{\partial z}, \frac{\partial H_x}{\partial z} - \frac{\partial H_z}{\partial x}, \frac{\partial H_y}{\partial x} - \frac{\partial H_x}{\partial y} \right)$$

Separation for 1) Plane Strain $u_x(x, y, t)$ only & $u_y(x, y, t)$; $u_z = 0$
 2) Anti Plane Strain $u_x = u_y = 0$ $u_z(x, y, t)$

Plane Strain Problem $u_x = \frac{\partial \phi}{\partial x} + \frac{\partial H_z}{\partial y}$ $u_y = \frac{\partial \phi}{\partial y} - \frac{\partial H_z}{\partial x}$ let $H = (0, 0, H_z)$

Let us concentrate on traction free surface $\Rightarrow \sigma_{yy}, \sigma_{yx} \neq 0$ $\sigma_{yz} = 0$

Put into strain-disp & Hooke's Law $\Rightarrow$

$$\sigma_{yy} = (\lambda + 2\mu) \left(\frac{\partial u_x}{\partial x} + \frac{\partial u_y}{\partial y} \right) - 2\mu \frac{\partial u_x}{\partial x} = (\lambda + 2\mu) \left(\frac{\partial^2 \phi}{\partial x^2} + \frac{\partial^2 \phi}{\partial y^2} \right) - 2\mu \left(\frac{\partial^2 \phi}{\partial x^2} + \frac{\partial^2 H_z}{\partial x \partial y} \right)$$

$$\sigma_{xy} = \mu \left(\frac{\partial u_x}{\partial y} + \frac{\partial u_y}{\partial x} \right) = \mu \left(2 \frac{\partial^2 \phi}{\partial x \partial y} + \frac{\partial^2 H_z}{\partial y^2} - \frac{\partial^2 H_z}{\partial x^2} \right)$$

Consider solns of waves traveling // to bounding plane: i.e. $\phi = f(y) e^{i(kx - \omega t)}$

and $H_z = h_2(y) e^{i(k_H x - \omega_H t)}$

Substitute into Eqns for wave eq $\Rightarrow \frac{d^2 f}{dy^2} + \alpha^2 f = 0$ w/ $\alpha^2 = \frac{\omega^2}{c_1^2} - k^2 = k^2 \left(\frac{c_1^2}{c_2^2} - 1 \right)$

also $\frac{d^2 h_2}{dy^2} + \beta^2 h_2 = 0$ w/ $\beta^2 = \frac{\omega_H^2}{c_2^2} - k_H^2 = k_H^2 \left(\frac{c_2^2}{c_1^2} - 1 \right)$

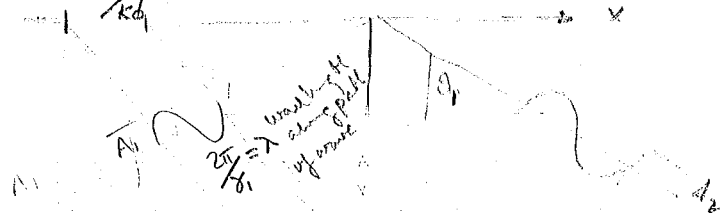
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Continuation of the problem involved

Thus $\phi = A_1 e^{i(kx - \alpha y - \omega t)} + A_2 e^{i(kx + \alpha y - \omega t)}$ — downgoing wave

since $f'' + \alpha^2 f = 0$ has soln $f = A_1 e^{i\alpha y} + A_2 e^{-i\alpha y}$

Similarly $H_z = B_1 e^{i(k_H x - \beta y - \omega_H t)} + B_2 e^{i(k_H x + \beta y - \omega_H t)}$



apparent
 $\gamma_i = \text{wave number} = \frac{\omega}{c_i}$
 along the path of the wave
 $k\phi = \frac{\omega\phi}{c}$

$\alpha^2 = \gamma_1^2 - k_\phi^2$
 $\gamma_1^2 = \gamma_2^2 + k_\phi^2$

$k\phi = \gamma_1 \sin \theta_1$

$\alpha = \gamma_1 \cos \theta_1$

$k_H = \gamma_2 \sin \theta_2$

$\beta = \gamma_2 \cos \theta_2$

$$\phi = A_1 \exp \{ i \gamma_1 (\sin \theta_1 x - \cos \theta_1 y - c_1 t) \} + A_2 \exp \{ i \gamma_1 (\sin \theta_1 x + \cos \theta_1 y - c_1 t) \}$$

$$H_2 = B_1 \exp \{ i \gamma_2 (\sin \theta_2 x - \cos \theta_2 y - c_2 t) \} + B_2 \exp \{ i \gamma_2 (\sin \theta_2 x + \cos \theta_2 y - c_2 t) \}$$

BC at $y=0$: $\sigma_{yy} = \sigma_{xy} = 0$ Let $\delta^2 = c_1^2/c_2^2$

$$\sigma_{yy} = 0 = \gamma_1^2 (2 \sin^2 \theta_1 - \delta^2) (A_1 + A_2) \exp \{ i \gamma_1 (\sin \theta_1 x - c_1 t) \} - \gamma_2^2 \sin^2 \theta_2 (B_1 - B_2) \exp \{ i \gamma_2 (\sin \theta_2 x - c_2 t) \}$$

Since σ_{yy} couples ϕ & $H_2 \Rightarrow$ exp must be same. for any $x, t \Rightarrow$

$$\begin{cases} \gamma_1 \sin \theta_1 = \gamma_2 \sin \theta_2 \\ \gamma_1 c_1 = \gamma_2 c_2 \end{cases}$$

$$\therefore c_1/c_2 = \frac{\sin \theta_1}{\sin \theta_2} = \frac{\gamma_2}{\gamma_1} \Rightarrow k_H = k_\phi \quad \& \quad \omega_\phi = \omega_H$$

$$\text{thus } \gamma_1^2 (2 \sin^2 \theta_1 - \delta^2) (A_1 + A_2) - \gamma_2^2 \sin^2 \theta_2 (B_1 - B_2) = 0$$

$$\sigma_{xy} = 0 = \gamma_1^2 \sin 2\theta_1 (A_1 - A_2) - \gamma_2^2 \cos 2\theta_2 (B_1 + B_2) = 0$$

for an impinging P wave only: $A_1 \rightarrow A_2, B_2$: B_1 (imp. $\rightarrow$ SV) = 0
SV wave only: $B_1 \rightarrow A_2, B_2$: A_1 (" P) = 0

Grazing Incidence of P waves

Let $\theta_1 \rightarrow 90^\circ \Rightarrow \cos \theta_1 \rightarrow 0 \quad \sin \theta_1 \rightarrow 1$

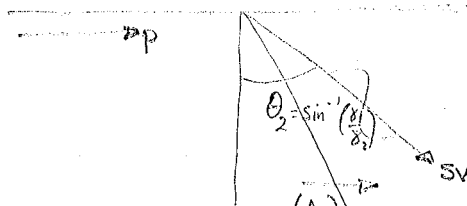
$\alpha \rightarrow 0$

$$\nabla^2 f + \alpha^2 f = 0 \Rightarrow f'' = 0 \quad f = Ay + B$$

in homogeneous plane wave.
 $\phi = (A_1 + A_2 y) e^{i(k\phi x - \omega t)}$
 $H_2 = B_2 e^{i(k\phi x + \beta y - \omega t)}$

put these into BC $\sigma_{yy} = \sigma_{xy} = 0$

$$\frac{\gamma_1}{\gamma_2} = \sin \theta_2 = \frac{1}{\delta} = \frac{c_2}{c_1}$$

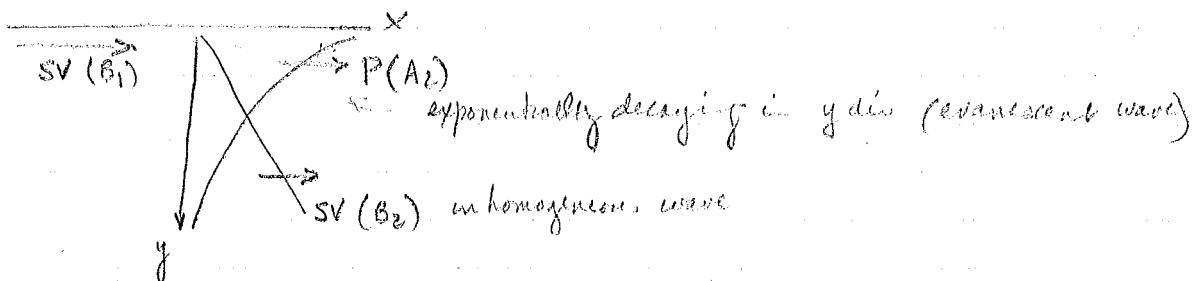


(A2) - Bishop/Gordian wave

$$\frac{A_2}{A_1} = \frac{i(\delta^2 - 1)^2}{4\sqrt{\delta^2 - 1}}$$

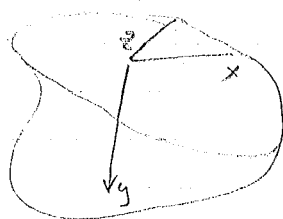
$$\frac{B_2}{A_1} = -\frac{(\delta^2 - 2)}{2\sqrt{\delta^2 - 1}}$$

2 P waves are out of phase by 90°



We want to find:

Surface waves: i.e. (Rayleigh waves) stay with some distance close to surface.



want to find waves moving in x direct. $\Rightarrow$ inhomog waves and want them to decay in y direct.

look for solns
i.e.
$$u_x = A e^{-b_1 y} e^{ik(x-ct)}$$
$$u_y = B e^{-b_2 y} e^{ik(x-ct)}$$
$$u_z = 0$$

$$\text{Im } b = 0 \quad \text{Re } b > 0$$

can use potentials (perhaps next time)

Subst. into eqns of motion. $\Rightarrow$ 2 hom. eqns on A & B. For non trivial sol. determinant must = 0. $\Rightarrow$
$$0 = [c_1^2 b^2 - (c_1^2 - c^2) k^2] [c_2^2 b^2 - (c_2^2 - c^2) k^2]$$

this eqn relates b, k & c. Quadratic on b

$$b_1 = k \left(1 - \frac{c^2}{c_1^2}\right)^{1/2}, \quad b_2 = k \left(1 - \frac{c^2}{c_2^2}\right)^{1/2}$$

$$\text{for } b > 0 \Rightarrow c < c_2 < c_1$$

$$\text{Using this we can calculate } \left(\frac{B}{A}\right)_{b=b_1} = -b_1 / ik \quad \left(\frac{B}{A}\right)_{b=b_2} = \frac{ik}{b_2}$$

$$\therefore u_x = [A_1 e^{-b_1 y} + A_2 e^{-b_2 y}] e^{ik(x-ct)}$$
$$u_y = \left[\frac{-b_1}{ik} A_1 e^{-b_1 y} + \frac{ik}{b_2} A_2 e^{-b_2 y} \right] e^{ik(x-ct)}$$

They then satisfy Eqs of motion & compatibility ...

Now apply B.C. to find A_1, A_2 & k

$$\sigma_{yy} = 0 = \sigma_{xy} = 0 \quad \text{for } y = 0$$

$$\sigma_{yy} = 0 \Rightarrow 2b_1 A_1 + (2 - \frac{c^2}{c_1^2}) k^2 A_2 / b_2 = 0$$

$$\sigma_{xy} = 0 \Rightarrow (2 - \frac{c^2}{c_1^2}) A_1 + 2b_2 \frac{A_2}{b_1} = 0$$

Thus $\det = 0 \Rightarrow$ Rayleigh Eqn.

$$\left| \left(2 - \frac{c^2}{c_2^2} \right)^2 - 4 \left(1 - \frac{c^2}{c_1^2} \right)^{\frac{1}{2}} \left(1 - \frac{c^2}{c_2^2} \right)^{\frac{1}{2}} = 0 \right| \quad k \text{ drops out}$$

or if rationalized

$$\frac{c^2}{c_2^2} \left[\frac{c^6}{c_2^6} - 8 \frac{c^4}{c_1^4} + 8 \left(3 - 2 \frac{c_2^2}{c_1^2} \right) \frac{c^2}{c_2^2} - 16 \left(1 - \frac{c_2^2}{c_1^2} \right)^2 \right] = 0$$

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Energy Propagation

Power in a wave $p = -t_i \dot{u}_i = -t \cdot \dot{u}$

Consider a longitudinal wave propagating in x direction

$$u_x = A \cos k(x - c_2 t)$$

$$\sigma_{xx} = i(\lambda + 2\mu) A k e^{ik(x - c_2 t)}$$

$$P_{\text{long}} = -\sigma_{xx} \dot{u}_x$$

$$u_x = A \cos k(x - c_2 t)$$

$$c_2 = \frac{\omega}{k}$$

$$\dot{u}_x = A k c_2 \sin k(x - c_2 t)$$

$$\sigma_{xx} = -(\lambda + 2\mu) A k \sin k(x - c_2 t)$$

$$P_{\text{long}} = (\lambda + 2\mu) c_2 k^2 A^2 \sin^2[k(x - c_2 t)] = (\lambda + 2\mu) \frac{\omega^2}{c_2} A^2 \sin^2(\omega t - kx)$$

this still looks like a wave & a better indication is the time average of P_L over one period

$$\bar{P}_{\text{aver}} = \frac{1}{T} \int_{\tau}^{\tau+T} P_{\text{long}} dt = (\lambda + 2\mu) \frac{\omega^2}{c_2} A^2 \underbrace{\frac{1}{T} \int_{\tau}^{\tau+T} \sin^2(kx - \omega t) dt}_{\frac{1}{2}}$$

$$\bar{P}_{\text{long}} = \frac{\lambda + 2\mu}{2} \frac{\omega^2}{c_2} A^2$$

we can define velocity at which energy is propagated as $c_e = \bar{P}_L / \bar{W}$

now $\bar{W}$ = time average of the total energy density = $\bar{K} + \bar{U}$ kinetic + potential energy.

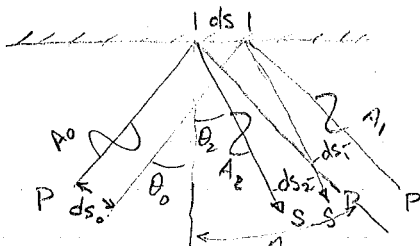
$$\bar{K} = \frac{1}{2} \frac{1}{T} \int_{\tau}^{\tau+T} \rho \dot{u}_x^2 dt = \frac{1}{4} \rho A^2 \omega^2$$

$$\bar{U} = \frac{1}{2} \frac{1}{T} \int_{\tau}^{\tau+T} \sigma_{xx} \epsilon_{xx} dt = \frac{1}{2} \frac{1}{T} \int_{\tau}^{\tau+T} (\lambda + 2\mu) \left(\frac{\partial u_x}{\partial x} \right)^2 dt = \frac{1}{4} (\lambda + 2\mu) A^2 k^2$$

$$\text{since } c_2 = \frac{\omega}{k} = \sqrt{\frac{\lambda + 2\mu}{\rho}} \Rightarrow \bar{K} = \bar{U} \quad \therefore \bar{W} = 2\bar{K} = 2\bar{U} = \frac{1}{2} (\lambda + 2\mu) A^2 k^2$$

$$\therefore c_e = \frac{\lambda + 2\mu}{2} \frac{\omega^2}{c_2^2} A^2 \cancel{A^2 k^2} = \frac{\omega^2}{k^2 c_2^2} = \frac{c_2^2}{c_2^2} = c_2 = \frac{\bar{P}_L}{\bar{W}}$$

Consider



want to find how the energy
breaks up (divides between P, S)

we had shown previously $\bar{P}_L = \frac{1}{2} (\lambda + 2\mu) \frac{\omega^2}{c_L} A^2$

we can show:

$$\bar{P}_{Tang} = \frac{1}{2} \mu \frac{\omega^2}{c_T^2} A^2$$

thus using conserv of energy energy in = energy out.

$$\frac{1}{2} (\lambda + 2\mu) \frac{\omega^2}{c_L} A_0^2 ds_0 = \frac{1}{2} (\lambda + 2\mu) \frac{\omega^2}{c_L} A_1^2 ds_1 + \frac{1}{2} \mu \frac{\omega^2}{c_T} A_2^2 ds_2 \quad (*)$$

we use geometries & get $ds_0 = ds_1 = ds \cos \theta_0$
 $ds_2 = ds \cos \theta_2$

thus putting back into (*)

$$\left(\frac{A_1}{A_0}\right)^2 + \left(\frac{A_2}{A_0}\right)^2 \frac{c_T}{c_L} \frac{\cos \theta_2}{\cos \theta_0} = 1$$

we also know from before that $\sin \theta_2 = K^{-1} \sin \theta_0$ where $K \equiv \left[\frac{2(1-\nu)}{1-2\nu} \right]^{1/2}$

$$\text{then } \left(\frac{A_1}{A_0}\right)^2 + \left(\frac{A_2}{A_0}\right)^2 \frac{1}{K \cos \theta_0} \left(1 - \frac{\sin^2 \theta_0}{K^2}\right)^{1/2} = 1$$

Should be same as boxed eqn (page 9)

Distinction between Group & phase velocity

$$\text{Given } u(x,t) = A \cos \underbrace{k(x - ct)}_{\text{phase vel}} = A \cos(kx - \omega t)$$

now add another wave with different $k_2 \neq \omega_2$ $k_2 = k_1 + \epsilon$ $\omega_2 = \omega_1 + \delta$
 $u_1 + u_2 = \phi = A \cos(k_1 x - \omega_1 t) + A \cos(k_2 x - \omega_2 t)$

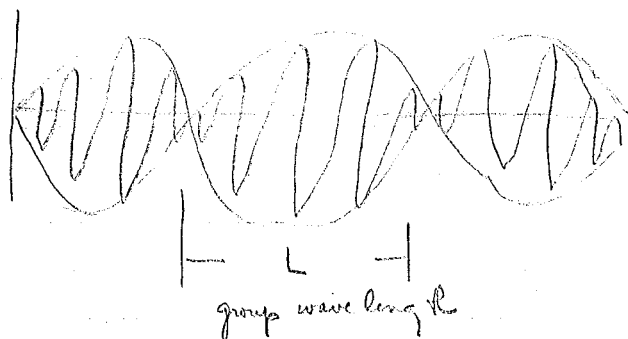
we have the trig relation $\cos \alpha + \cos \beta = 2 \cos\left(\frac{\alpha + \beta}{2}\right) \cos\left(\frac{\alpha - \beta}{2}\right)$

$$\text{thus } \phi = 2A \cos\left(\frac{k_1 + k_2}{2} x - \frac{\omega_1 + \omega_2}{2} t\right) \cos\left(\frac{k_1 - k_2}{2} x - \frac{\omega_1 - \omega_2}{2} t\right)$$

$$= \phi_0 \cos\left(\frac{k_1 + k_2}{2} x - \frac{\omega_1 + \omega_2}{2} t\right) \quad \text{w/ } \phi_0 = 2A \cos\left(\frac{k_1 - k_2}{2} x - \frac{\omega_1 - \omega_2}{2} t\right)$$

like the previous wave but different amplitude

$$\frac{k_1 - k_2}{2} = \pi \left(\frac{1}{\lambda_1} - \frac{1}{\lambda_2}\right) = \pi \left(\frac{\lambda_2 - \lambda_1}{\lambda_1 \lambda_2}\right) = \frac{\pi}{\lambda_2} \left(\frac{\lambda_2 - \lambda_1}{\lambda_2 - \lambda_1}\right) = \frac{2\pi}{L} \quad L \gg \lambda_1, \lambda_2$$

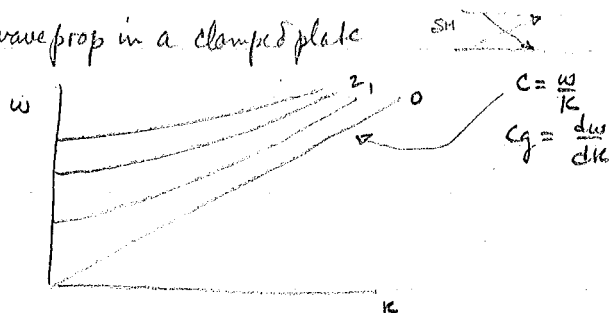


$$c_g = \frac{\omega_1 - \omega_2}{k_1 - k_2} = \frac{\Delta \omega}{\Delta k} \Rightarrow \frac{d\omega}{dk}$$

$$c = \frac{\omega_1 + \omega_2}{k_1 + k_2} \quad \text{phase vel.}$$

$$c_g = \frac{d(c_k)}{dk} = c + k \frac{dc}{dk} \quad \text{thus if } \frac{dc}{dk} \neq 0 \quad c_g \neq c$$

if we look at waveprop in a clamped plate



if $c > c_g$ normal dispersion
if $c < c_g$ anomalous dispersion

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Returning to rayleigh waves

First we postulated the displ (on pg 12) put into Eqns of Motion & applied bc to get

$$2b_1 A_1 + (2 - \frac{c^2}{c_2^2}) k^2 A_2 / b_2 = 0$$

$$(2 - \frac{c^2}{c_2^2}) A_1 + 2b_2 A_2 / b_2 = 0$$

} if $A_1 \neq A_2$ are not zero the determinant of coeffs $\equiv 0$

this then gives $(2 - \frac{c^2}{c_2^2})^2 - 4(1 - \frac{c^2}{c_1^2})^{1/2}(1 - \frac{c^2}{c_2^2})^{1/2} = 0$

k drops out since b_1, b_2 are fns of k $b_i = k / (1 - \frac{c^2}{c_i^2})^{1/2}$

When we rationalized

$$\frac{c^2}{c_2^2} f(\frac{c}{c_2}) = \frac{c^6}{c_2^2} \left[\frac{c^6}{c_2^6} - 8 \frac{c^4}{c_1^4} + 8 \left(3 - 2 \frac{c_2^2}{c_1^2} \right) \frac{c^2}{c_2^2} - 16 \left(1 - \frac{c^2}{c_1^2} \right)^2 \right] = 0$$

3 roots (only one is physical) since this is a cubic in $(\frac{c^2}{c_2^2})$

if $c = c_2$ then $f(\frac{c}{c_2}) = 1$

let $c = \epsilon c_2$ then $f(\frac{c}{c_2}) = f(\epsilon) \sim -16 + 16 \frac{c_2^2}{c_1^2} = -16(1 - \frac{c_2^2}{c_1^2}) < 0$

$\therefore$ a root must exist between c_2 & ϵc_2 since $(c_2/c_1) < 1$

Let $\nu = \frac{1}{4}$ $\lambda = \mu \Rightarrow c_1 = c_2 \sqrt{3}$ $f(\frac{c}{c_2}) = \left[\frac{c^6}{c_2^6} - 8 \frac{c^4}{c_2^4} + \frac{56}{3} \frac{c^2}{c_2^2} - \frac{32}{3} \right] = 0$

$c^2/c_2^2 = 4$; $2 \pm 2\sqrt{3}$ only soln is $\frac{c^2}{c_2^2} = 2 - \frac{2}{\sqrt{3}}$

if $c > c_2$ we don't have surface waves. If there's more than one root in the interval $\frac{d^2 f}{d(\frac{c^2}{c_2^2})^2} = 0$ somewhere in the interval: $(\frac{c^2}{c_2^2}) = 8/3$ then

but this is not in interval $\therefore$ only one root.

if $\nu = 1/4$ $c_2/c_1 = .9194$

$$u_x = A' (e^{-.8475 k y} - .5773 e^{-.3933 k y}) \cos k(x - c_2 t)$$

$$u_y = A' (-.8475 e^{-.8475 k y} + 1.4679 e^{-.3933 k y}) \sin k(x - c_2 t)$$

A' is some arbitrary constant.

ν	c_2/c_1
0	.862
$1/4$	.919
$1/3$	.937
$1/2$	.955

pronouncement of Rayleigh velocity as a fn of ν

In 1908 Michie data on surface waves showed $u_y \sim 0$ & $u_x \neq f(x)$ but $u_x = f(x)$. 1911 Love showed existence of other waves, - Love waves (To be discussed later)

Rayleigh surface waves using potentials

$$u = \nabla \phi + \nabla \times H \quad \nabla \cdot H = 0$$

$$\text{let } \phi = \phi(x, y, t) \quad H_3 = H_3(x, y, t)$$

$$\phi = f(y) e^{ik(x-ct)} \quad H_3 = h(y) e^{ik(x-ct)}$$

Put in Eqns of motion to get: (wave Eqn)

$$f'' + k^2 \left(\frac{c^2}{c_1^2} - 1 \right) f = 0 \quad \Rightarrow \quad \alpha = k^2 \left(\frac{c^2}{c_1^2} - 1 \right) \quad \text{let } \alpha = i\alpha_1$$

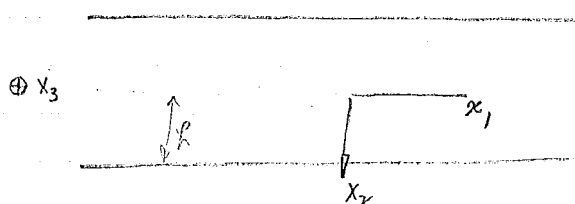
$$h'' + k^2 \left(\frac{c^2}{c_2^2} - 1 \right) h = 0 \quad \Rightarrow \quad \beta = k^2 \left(\frac{c^2}{c_2^2} - 1 \right) \quad \text{let } \beta = i\beta_1$$

$$\Rightarrow f = A e^{-\alpha_1 y} \quad h = B e^{-\beta_1 y} \quad \text{for decaying waves}$$

this then defines u_1, u_2 . Apply bc $\Rightarrow \sigma_{22}, \sigma_{12} = 0$ (as fns of (u_i, j))

$$\Rightarrow \begin{cases} -2ik\alpha_1 A + (k^2 + \beta_1^2) B = 0 \\ (k^2 + \beta_1^2) A + 2ik\beta_1 B = 0 \end{cases} \Rightarrow (k^2 + \beta_1^2)^2 - 4k^2\alpha_1\beta_1 = 0 \text{ for } A \neq B \neq 0$$

Let's look at an elastic layer (leave the half space problem.) Assume homog isotropic



Our eqns will uncouple if we assume plane motion or

$$\begin{cases} u_1 = u_1(x_1, x_2, t) \\ u_2 = u_2(x_1, x_2, t) \\ u_3 = 0 \end{cases} \quad \begin{cases} u_1(x_1, x_2, t) \\ 0 \\ u_3(x_1, x_2, t) \end{cases}$$

OR anti plane strain

put into Eqns of motion

$$u_1 = 0 \quad u_2 = 0 \quad u_3 = u_3(x_1, x_2, t)$$

$$\Delta u_3 = \frac{1}{c_2^2} u_{3,tt}$$

Assume $\sigma_{22} = \sigma_{12} = 0$ at $\pm h$ (are identically satisfied) since $u_{1,2}, u_{2,1}, u_{2,2}, u_{1,1} = 0$

$$\sigma_{32} = \mu \frac{\partial u_3}{\partial x_2} = 0 \text{ at } \pm h$$

Assume a sol of form $u_3 = f(x_2) e^{ik(x_1-ct)}$

put into DE: $-k^2 f + f'' = -k^2 c^2 f / c_2^2$

if we let $q^2 = k^2 \left(\frac{c^2}{c_2^2} - 1 \right) = \frac{\omega^2}{c_2^2} - k^2$

$$f(x_2) = B_1 \sin(qx_2) + B_2 \cos(qx_2)$$

$$\text{from BC: } B_1 \cos qh \pm B_2 \sin qh = 0$$

$$\left. \frac{\partial u_3}{\partial x_2} \right|_{x_2=\pm h} = 0$$

for satisfaction (determinant $\neq \sin^2 qh \neq 0$) $\Rightarrow B_1 = 0 \neq B_2$ trivial case

So we must take 1. $B_1 = 0$ & $\sin qh = 0$ or

2. $\cos qh = 0$ & $B_2 = 0$

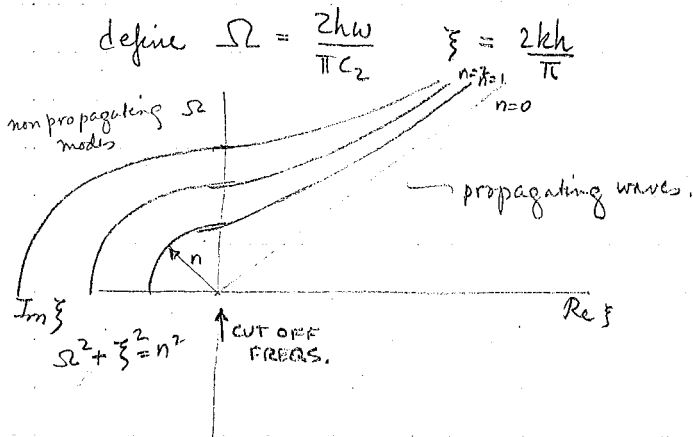
Case 1: $B_1 = 0$; $\sin qh = 0 \Rightarrow$ displacement u_3 must be symmetric wrt $x_2 = 0$

Case 2 $B_2 = 0$ $\cos qh = 0 \Rightarrow$ displacement u_3 must be anti-sym wrt $x_2 = 0$

$$\Rightarrow \text{for each problem } qh = \frac{n\pi}{2}$$

$n = \text{even}$ gives symmetric case $n=0, 2, 4, \dots$
 $n = \text{odd}$ gives anti-sym case $n=1, 3, 5, \dots$

10/23/79

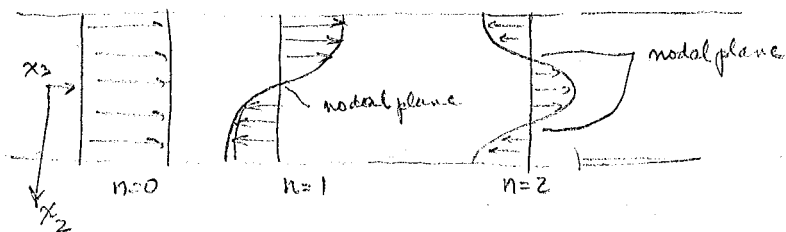


$n=0, 2, 4, \dots \Rightarrow$ symmetric modes
 $n=1, 3, 5, \dots \Rightarrow$ anti-sym modes

if k were imaginary the $ik < 0$
 and we would have non propagating waves

only for $n=0$ $\frac{\Omega}{\xi} = \frac{2hw/\pi c_2}{\frac{2khc_2}{\pi c_2}} = 1 = \frac{w}{kc_2} \Rightarrow$ there is no dispersion
 $w = kc$ $\therefore \frac{w}{kc_2} = 1 \Rightarrow c = c_2$

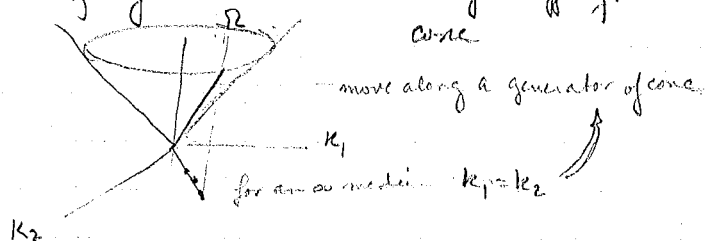
for any other $n \neq 0 \Rightarrow c \neq \text{const}$ and we have dispersion



dispersiveness is due to the boundaries & reflections from body

Consider a wave propagating in an infinite medium $u_3 = u_3(x_1, x_2, t) = Ae^{i(kx - \omega t)}$
 then $k_1^2 + k_2^2 = \frac{\omega^2}{c^2} = \Omega^2$ putting into $\Delta u_3 = \frac{u_3}{c^2}$

this is an eqn of a cone and solns. of diff eqn must have k_1, k_2, ω on the cone



$$\text{if } C_p = \frac{\omega}{|k|} n_k ; n_k = \frac{|k|}{|k|} \Rightarrow C_p = \frac{\omega |k|}{k^2}$$

$$\text{if } |k| = k_1 e_1 + k_2 e_2 \Rightarrow C_p = \frac{\omega}{k^2} [k_1 e_1 + k_2 e_2] \quad k^2 = k_1^2 + k_2^2$$

$$\text{if } k_2 = 0 \Rightarrow k^2 = (k_1^2 + k_2^2) = k_1^2 \quad |k| = k_1 e_1 \quad \therefore C_p = \frac{\omega}{k_1^2} k_1 e_1 = \frac{\omega e_1}{k}$$

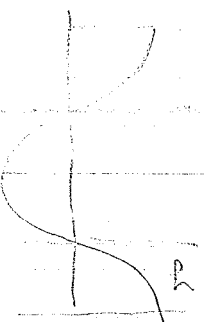
for a wave guide must have soln which has $\frac{\partial u_3}{\partial x_2} = 0$ at bdy as a condition. restricting

$$\text{suppose } n=1 \Rightarrow \lambda = 4h$$

$$\Rightarrow k = \frac{2\pi}{\lambda} = \frac{\pi}{2h}$$

(for $n=1$ $qh = \pi/2$ thus we only get

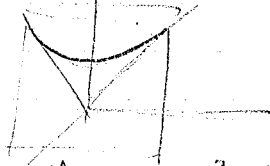
$1/4$ wave in $h \Rightarrow 1$ full wave in $4h$)



for a waveguide: $k_2 = \frac{\pi}{2h} \Rightarrow$ fixed $\Rightarrow k_2$ can be anything

Our soln is restricted by the boundaries i.e.

thus we have a soln of $\Omega^2 =$ intersect of plane ($k_2 = \pi/2h$) & cone $\Rightarrow$ hyperbola and these are hyperbolas for Re ξ part of spectrum.

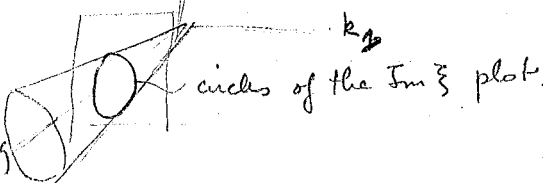


when $\xi^2 < 0$ then $\Omega^2 + \xi^2 = n^2$

again if for a waveguide let $k_2 = \frac{\pi}{2h}$ & $k_1 = \text{anything}$

then plane intersecting cone $\Rightarrow$ circles

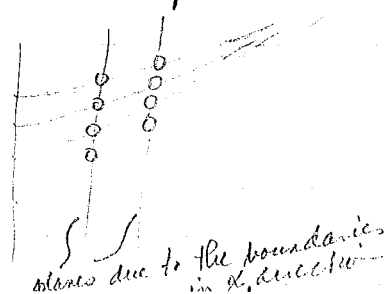
and are the circles at the Im ξ portion of spectrum.



i.e. restrict k_1

If we add boundaries in the x_1 direct $\Rightarrow$ we must do same as before $\Rightarrow$ discard system of ω

planes due to boundary in k_2 direction

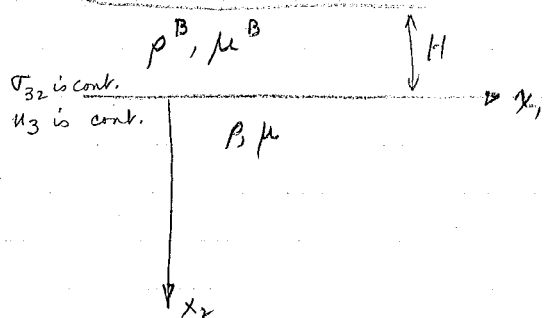


planes due to the boundaries in k_1 direction



• solutions are pts that are on intersection of $k_1 = \text{const}$, $k_2 = \text{const}$ planes

Love waves - a layer above an ∞ medium. look at horiz polarized shear waves



In the Half Space $u_3 = A e^{-b x_2} e^{i k (x_1 - c t)}$
 put into DE $\Delta u_3 - \frac{1}{c_2^2} u_{3,tt} = 0$

$$\left[b^2 - k^2 + \frac{k^2 c^2}{c_2^2} \right] u_3 = 0$$

$$\Rightarrow b = k \left(1 - \frac{c^2}{c_2^2} \right)^{1/2}$$

In the upper layer $u_3^B = [B_1 \sin q_B x_2 + B_2 \cos q_B x_2] e^{i k (x_1 - c t)}$

put into DE $q_B = k \left[\frac{c^2}{c_2^2} - 1 \right]^{1/2}$

bc. traction free at upper surface
 bonded condition at interface
 boundedness at $x_2 \rightarrow \infty$

Continuity of traction & disp at $x_2=0$

$x_2 = -H$ σ_{32} must be 0 $\frac{\partial u_3}{\partial x_2} = 0$

$$\sigma_{32} = 0 \text{ at } x_2 = -H \Rightarrow B_1 \cos q_B H + B_2 \sin q_B H = 0$$

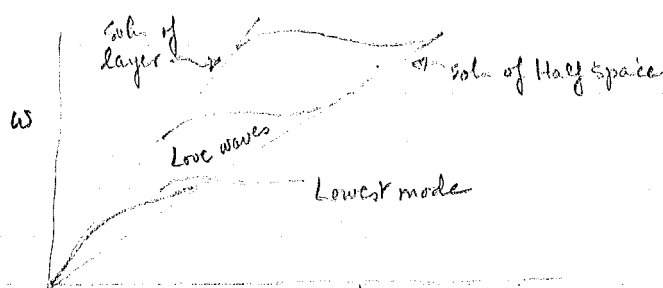
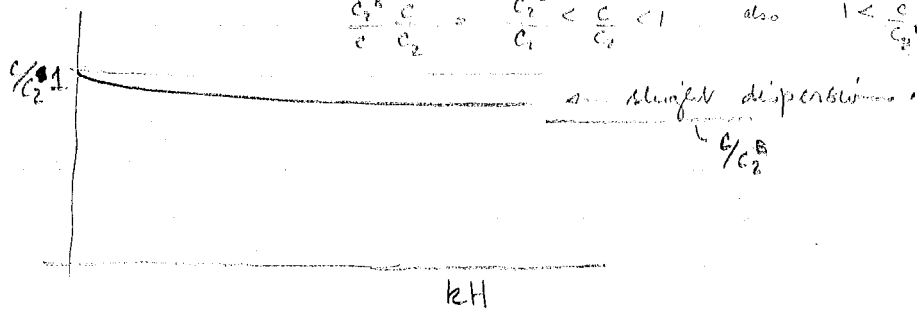
traction = continuous at $x_2=0$ $\mu q_B B_1 = -\mu b A = \mu \frac{\partial u_3}{\partial x_2} \Big|_{x_2=0^+} = \mu \frac{\partial u_3}{\partial x_2} \Big|_{x_2=0^-}$
 disp = cont. at $x_2=0$ $B_2 = A = u_3 \Big|_{x_2=0^+} = u_3 \Big|_{x_2=0^-}$

for non triv soln we have a transcendental Eq in B_1 & A

$$\therefore \begin{vmatrix} \cos q_B H & \sin q_B H \\ \mu q_B & \mu b \end{vmatrix} = 0 \Rightarrow 0 = \tan \left\{ \left[\left(\frac{c}{c_2^B} \right)^2 - 1 \right]^{1/2} k H \right\} - \frac{\mu \left[1 - \left(\frac{c}{c_2} \right)^2 \right]}{\mu b \left[\left(\frac{c}{c_2^B} \right)^2 - 1 \right]^{1/2}}$$

Real roots exists in interval $c_2^B \leq c \leq c_2$ no real roots exists if $c_2 < c_2^B$

$$\frac{c_2^B}{c} \leq \frac{c}{c_2} = \frac{c_2^B}{c_1} < \frac{c}{c_1} < 1 \text{ also } 1 < \frac{c}{c_2^B} < \frac{c}{c_1}$$



10/25/79

Elastic-Layer, (Plane Strain) Rayleigh-Lamb Problem

Given a plate of thickness $2b$ with the following Plane Strain Assumption
and we want waves for which $\tau_{xy}, \tau_{yy} = 0$ on $\pm b$



$$u_x = u_x(x, y, t)$$

$$u_y = u_y(x, y, t)$$

$$u_z = 0$$

$$u = \nabla \phi + \nabla \times H \quad \nabla \cdot H = 0$$

Let: $\phi = \phi(x, y, t)$ $H_3 = H_3(x, y, t)$ $H_1, H_2 = 0$

Since we don't know dependence in the y direction let

$$\phi = f(y) \sin \xi x e^{i\omega t}$$

$$H_3 = h(y) \cos \xi x e^{i\omega t}$$

pick this since we expect phase shift

$$\xi = \frac{\omega}{c}$$

we note that $\Delta \phi = \frac{1}{c_1^2} \phi_{,tt}$ and $\Delta H_3 = \frac{1}{c_2^2} H_{3,tt}$ ξ is the wave no.

are the eqns of motion. Put ϕ, H_3 into them to get

$$f'' + \alpha^2 f = 0 \quad \text{w/ } \alpha^2 = \xi^2 (c_1^2 - 1) \quad \beta^2 = \xi^2 (c_2^2 - 1)$$

$$h'' + \beta^2 h = 0$$

$$\Rightarrow \begin{aligned} f &= A \sin \alpha y + B \cos \alpha y \\ h &= C \sin \beta y + D \cos \beta y \end{aligned}$$

Displacements: $u_x = \frac{\partial \phi}{\partial x} + \frac{\partial H_3}{\partial y} = (\xi f + h') \cos \xi x e^{i\omega t}$ sym wrt y if f, h' are fn of $\cos y$

$$u_y = \frac{\partial \phi}{\partial y} - \frac{\partial H_3}{\partial x} = (f' + \xi h) \sin \xi x e^{i\omega t}$$
 sym wrt y if f' & h are fn of $\cos y$

now using stress-strain relation

$$\sigma = \lambda \nabla \cdot u \mathbb{I} + 2\mu \epsilon$$

$$\sigma_{ij} = \lambda (u_{k,k}) \delta_{ij} + \mu (u_{i,j} + u_{j,i})$$

putting these into stress-displ relations and we want $\tau_{xy} \& \tau_{yy} = 0$ at $y = \pm b$. In general

$$\sigma_{xx} = -\mu [(\beta^2 + \xi^2 - 2\alpha^2)f + 2\xi h'] \sin \xi x e^{i\omega t}$$

$$\tau_{yy} = -\mu [(\beta^2 - \xi^2)f - 2\xi h'] \sin \xi x e^{i\omega t}$$

$$\sigma_{zz} = -\lambda (\alpha^2 + \xi^2) f \sin \xi x e^{i\omega t}$$

$$\tau_{xy} = \mu [2\xi f' + (\xi^2 - \beta^2)h] \cos \xi x e^{i\omega t}$$

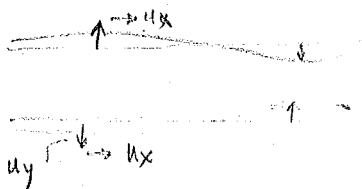
$$\tau_{zy} = \tau_{zx} = 0 \quad \text{due to plane strain condition}$$

We look at 2 conditions where u_y symmetric about $y=0$
antisymmetric about $y=0$

Let $f = B \cos \alpha y$ $h = C \sin \beta y \Rightarrow u_x = (\xi B \cos \alpha y + \beta C \cos \beta y) \cos \xi x e^{i\omega t}$

$u_y = (-\alpha B \sin \alpha y + \xi C \sin \beta y) \sin \xi x e^{i\omega t}$

wrt y u_y is ~~antisymmetric~~ ^{sym.} u_y is symmetric $\Rightarrow$ we have "extensional" motion



look at B.C. $\tau_{yy} = \tau_{xy} = 0$ for $y = \pm b$

$$(\beta^2 - \xi^2) B \sin \alpha b - 2 \xi \beta C \cos \beta b = 0$$

since $-2 \xi \alpha B \sin \alpha b + (\xi^2 - \beta^2) C \sin \beta b = 0$ solve for $B \neq C$

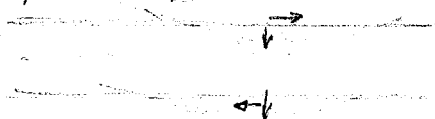
for $B, C \neq 0 \Rightarrow$ determ. $= 0 \Rightarrow \left| \frac{\tan \beta b}{\tan \alpha b} = - \frac{4 \xi^2 \alpha \beta}{(\beta^2 - \xi^2)^2} \right|$ dispersion eqn.

with this we can solve for $B/C = \frac{2 \xi \beta \cos \beta b}{(\beta^2 - \xi^2) \cos \alpha b}$

Let $f = A \sin \alpha y$ $h = D \cos \beta y \Rightarrow u_x = (\xi A \sin \alpha y - \beta D \sin \beta y) \cos \xi x e^{i\omega t}$

$u_y = (\alpha A \cos \alpha y + \xi D \cos \beta y)$

wrt y $u_x = \text{sym.}$ u_y is antisym.



we have "flexural" motions.

now we apply B.C. $\tau_{yy} = \tau_{xy} = 0$ for $y = 0$

$$(\beta^2 - \xi^2) A \sin \alpha b + 2 \xi \beta D \sin \beta b = 0$$

$2 \xi \alpha A \cos \alpha b + (\xi^2 - \beta^2) D \cos \beta b = 0$ solve for A, D

$\left| \frac{\tan \beta b}{\tan \alpha b} = - \frac{(\beta^2 - \xi^2)^2}{4 \xi^2 \alpha \beta} \right|$ dispersion relation

$$\frac{A}{D} = \frac{2 \xi \beta \sin \beta b}{(\xi^2 - \beta^2) \sin \alpha b}$$

if we admit $c_1 < c_2 < c$ then there $\alpha = i \alpha_1$, $\beta = i \beta_1$

$$\frac{\tan \beta b}{\tan \alpha b} = \frac{4 \xi^2 \alpha \beta}{(\beta^2 + \xi^2)^2} \quad \text{for extensional motion}$$

$$\alpha = \xi \sqrt{1 - \frac{c^2}{c_1^2}} \quad \beta = \xi \sqrt{1 - \frac{c^2}{c_2^2}}$$

if $\xi \rightarrow \infty \Rightarrow$ LHS of $\rightarrow 1 \Rightarrow$ wavelength $\rightarrow 0$

$$\therefore (\beta^2 + \xi^2)^2 - 4 \xi^2 \alpha \beta = 0 \quad \text{this is the Rayleigh eqn.}$$

Reason why we get this eqn. is that wave of small wavelength looks at body's as if they are very far away & hence must be governed in limit by the Rayleigh problem.

$$\text{let } \Omega = \frac{\omega}{\omega_s} ; \quad \omega_s = \frac{\pi}{2b} \sqrt{\frac{\mu}{\rho}} = \frac{\pi c_2}{2b} \quad \text{non dim freq}$$

$$Z = \frac{2b}{\pi} \xi ; \quad = \frac{4b}{L} \sim \text{wave length} \quad \text{non dim wave no. ; } \frac{2\pi}{\xi} = L$$

ξ : wave no. i.e.

$$\frac{c_1^2}{c_2^2} = \frac{2(1-\nu)}{1-2\nu} = \delta^2$$

$$\frac{2\alpha b}{\pi} = \sqrt{\frac{\Omega^2}{\delta^2} - Z^2} \quad \frac{2\beta b}{\pi} = \sqrt{\Omega^2 - Z^2}$$

Extensional Motion (Symmetric)

f ~ B cos y h ~ C sin y

$$u_x \sim (B \xi \cos \alpha y + C \beta \cos \beta y)$$

$$u_y \sim (-B \alpha \sin \alpha y + C \xi \sin \beta y)$$

$$\frac{\tan \beta b}{\tan \alpha b} + \left[\frac{4 \xi^2 \alpha \beta}{(\xi^2 - \beta^2)^2} \right] = 0$$

Case 1 $Z = 0 \quad (\xi = 0, L = \infty)$

$$\alpha b = \frac{\pi \Omega}{2 \delta}$$

$$\tan \beta b / \tan \alpha b = 0$$

$$\beta b = \frac{n\pi}{2} \quad (\text{even}) \Rightarrow \Omega = n$$

$$\alpha b = \frac{m\pi}{2} \quad (\text{odd}) \Rightarrow \Omega = m\delta$$

Flexural (Antisymmetric)

f ~ A sin y h ~ D cos y

$$u_x \sim (A \xi \sin \alpha y - D \beta \sin \beta y)$$

$$u_y \sim (A \alpha \cos \alpha y + D \xi \cos \beta y)$$

$$\frac{\tan \beta b}{\tan \alpha b} + \left[\frac{4 \xi^2 \alpha \beta}{(\xi^2 - \beta^2)^2} \right]^{-1} = 0$$

$Z < 0 \quad (\xi = 0, L = \infty)$

$$\beta b = \frac{\pi \Omega}{2}$$

$$\tan \beta b / \tan \alpha b = \infty$$

$$\beta b = \frac{m\pi}{2} \quad \text{odd} \Rightarrow \Omega = n$$

$$\alpha b = \frac{m\pi}{2} \quad \text{even} \Rightarrow \Omega = m\delta$$

Continuing

10/30/79

Assumed a layer under plane strain and the following for the potentials

Assoc w/extension $\phi = f(y) \sin \xi x e^{i\omega t}$

Assoc. w/Shear $H_3 = h(y) \cos \xi x e^{i\omega t}$

to satisfy the eqns of motion

Dilatational $f(y) = A \sin \alpha y + B \cos \alpha y$ w/ $\alpha^2 = \xi^2 \left(\frac{c^2}{c_1^2} - 1 \right)$
 equivolum. $h(y) = C \sin \beta y + D \cos \beta y$ w/ $\beta^2 = \xi^2 \left(\frac{c^2}{c_2^2} - 1 \right)$
 satisfies $\nabla^2(A) = \frac{1}{c_1^2} \frac{\partial^2 A}{\partial t^2}$
 satisfies $\Delta(u) = \frac{1}{c_2^2} \frac{\partial^2 u}{\partial t^2}$

we find then that

Shear-Displ

Egns. $u_x = \frac{\partial \phi}{\partial x} + \frac{\partial H_3}{\partial y} = (\xi f + h') \cos \xi x e^{i\omega t}$

$u_y = \frac{\partial \phi}{\partial y} - \frac{\partial H_3}{\partial x} = (f' + \xi h) \sin \xi x e^{i\omega t}$

we want extensional motion (symmetric) i.e. $B, C \neq 0$ $A, D = 0$

" " flexural motion (Antisymmetric) if $A, D \neq 0$ $B, C = 0$

BC: traction free planes BC do not couple fig. but give the relation between ξ & ω .

$\Rightarrow \frac{\tan \beta b}{\tan \alpha b} + \left[\frac{4 \xi^2 \alpha \beta}{(\xi^2 - \beta^2)^2} \right]^{\pm 1} = 0$ (*)
 +1 : extensional motion
 -1 : flexural motion

now $\omega = \xi c$ thus (*) is an implicit fn of ξ & ω .

To get insight into what goes on we set up table (on pg 22 these notes)

Since we have shown that $\frac{\tan \beta b}{\tan \alpha b}$, which governs both motions, can result in solutions which give rise to soln to extens, indep of soln to ~~equiv~~ flexural, we can have solns that are independent of the other 3.

Thus we can have extens dilat, extens equiv, flex. dilat & flex. equiv we can look at mixed by which doesn't couple equiv & dilatational modes. We can thus look at the 4 cases stated.

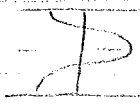
Equivoluminal waves

for infinitely long wavelength motion indep. of x ,

$\Omega = n$

Ext. (Sym) $B, C \neq 0$	Flex (Antisym) $A, D \neq 0$
$B = 0$	$A = 0$
$u_x \sim C \beta \cos \beta y$; $u_y = 0$	$u_x \sim -D \beta \sin \beta y$; $u_y = 0$
$u_x = C \beta \cos \frac{n \pi y}{2b}$ n even	$u_x = -D \beta \sin \frac{n \pi y}{2b}$ n is odd.

for $n=2$



symmetric

$y=x$

thickness shear $n=2$

for $n=1$



antisym.

thickness shear mode
(buck mode).

Deformational Waves

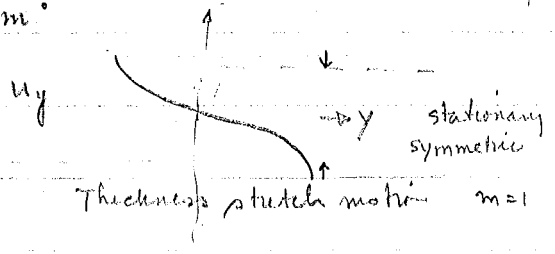
for ∞ long wavelength
motion independent
of x_1

Extensional
 $C, B \neq 0$ (sym.)
 $C=0$

$$u_x = 0; u_y = -B\alpha \sin \alpha y$$

$$u_y = -B\alpha \sin \frac{m\pi y}{2b} \quad \text{mode}$$

$$\Omega = 8m$$

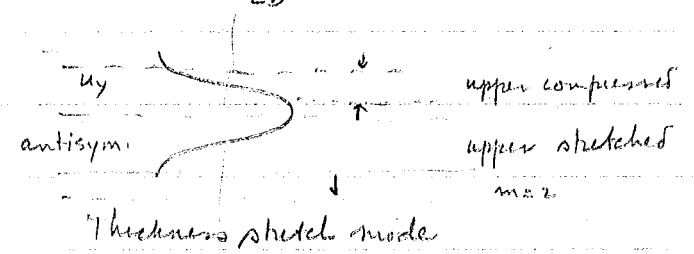


Thickness stretch motion $m=1$

Flex.
 $AD \neq 0$ (antisym.)
 $D=0$

$$u_x = 0; u_y = A\alpha \cos \alpha y$$

$$= A\alpha \cos \frac{m\pi y}{2b} \quad (m \text{ even})$$



Thickness stretch mode

11/1/79

Midterm: Next Thursday

Suppose $\xi = \beta \Rightarrow \zeta_2 = \sqrt{2}$ This is a special case: $\beta = \xi \left(\frac{\zeta_2^2}{\zeta_1^2} - 1 \right)^{1/2}$
 $\Rightarrow \alpha = \xi \left[\left(\frac{\zeta_2}{\zeta_1} \right)^2 - 1 \right]^{1/2}$

Remember $Z = \sqrt{\Omega^2 - \alpha^2}$

$$\alpha b = \frac{\pi}{2} \sqrt{\frac{2\delta^2}{\delta^2} - 1} = \frac{\pi}{2} \sqrt{\frac{2-\delta^2}{\delta^2}} = \frac{\pi}{2} \Omega \sqrt{\frac{2-\delta^2}{2\delta^2}}$$

$$\beta b = \frac{\pi}{2} Z = \frac{\pi}{2} \frac{\Omega}{\sqrt{2}}$$

Extensional ($B, C \neq 0$)

Freq eqn $\frac{\tan \beta b}{\tan \alpha b} = \omega$ see general eq (pg 22r)

to satisfy the above let

$$\beta b = \frac{r\pi}{2} \quad r \text{ odd}$$

$$\alpha b = \frac{s\pi}{2} \quad s \text{ even}$$

$$\Rightarrow \Omega = r\sqrt{2}, \quad Z = r \quad r \text{ odd}$$

(C, D $\neq 0$) Equivolum. waves (no volum changes)

Equiv Ext

$$B = 0$$

LAMÉ modes

$$u_x \sim C \beta \cos \beta y \cos \beta x$$

$$u_y \sim C \beta \sin \beta y \sin \beta x$$

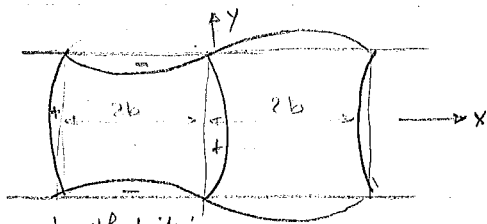
Substituting β for arguments

$$u_x \sim C \beta \cos \frac{\pi y}{2b} \cos \frac{\pi x}{2b}$$

$$u_y \sim C \beta \sin \frac{\pi y}{2b} \sin \frac{\pi x}{2b}$$

wavelength in x, y
direct are same

How will this look

note that it is
sym wrt middle plane

$$\Omega = r\sqrt{2} \quad Z = r \quad r \text{ even}$$

$$A = 0$$

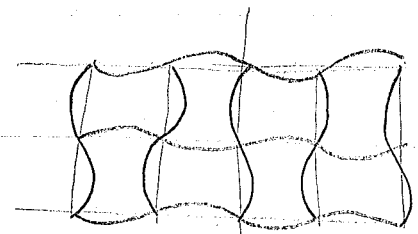
Equiv Flex

$$u_x \sim -D \beta \sin \beta y \cos \beta x$$

$$u_y \sim D \beta \cos \beta y \sin \beta x$$

$$u_x \sim -D \beta \sin \frac{\pi y}{b} \cos \frac{\pi x}{b}$$

$$u_y \sim D \beta \cos \frac{\pi y}{b} \sin \frac{\pi x}{b}$$



antisym. wrt middle plane

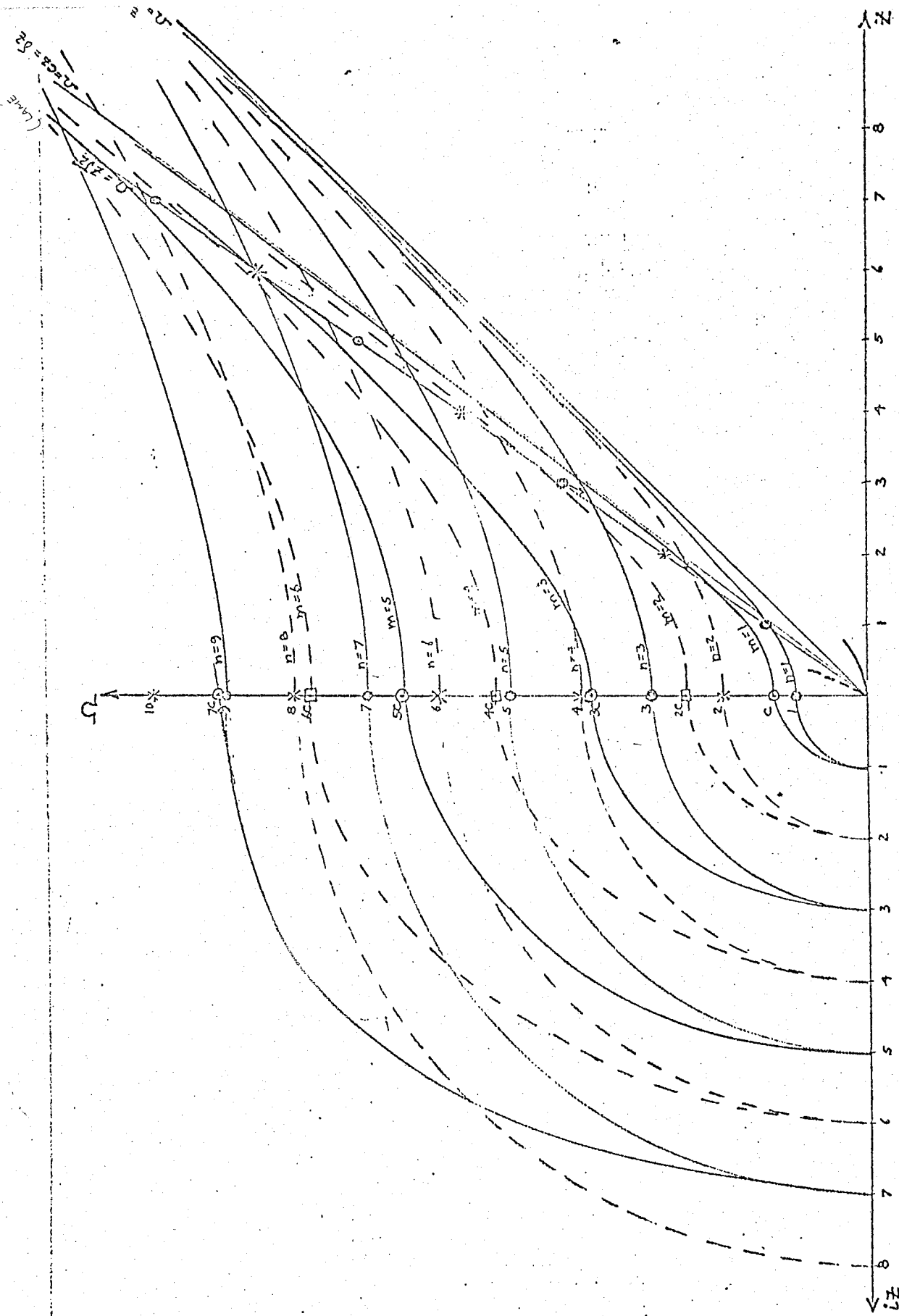
Remember $\alpha^2 = \xi^2 \left(\frac{\zeta_2^2}{\zeta_1^2} - 1 \right)$ $\beta^2 = \xi^2 \left(\frac{\zeta_2^2}{\zeta_1^2} - 1 \right)$

with the freq eq: $\frac{\tan \beta b}{\tan \alpha b} + \left[\frac{4 \xi^2 \alpha \beta}{(\xi^2 - \beta^2)^2} \right]^{\pm 1} = 0$

$$\omega = \xi c$$

this gives 3 eqns in 3 unknown α, ξ, β

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where now

$$\Omega = 2b\omega/\pi c_2, \quad \bar{\xi} = 2b\xi/\pi. \quad (8.1.47)$$

In a similar fashion, for the SV waves we have from (8.1.37) and (8.1.41)

$$\beta b = n\pi/2 \quad (n = 1, 2, 3 \dots), \quad (8.1.48)$$

where n even and odd governs, respectively, the symmetric and antisymmetric SV waves. Using the second of (8.1.23), this may be put in the form

$$\Omega^2 = (n^2 + \bar{\xi}^2) \quad (n = 1, 2, 3, \dots). \quad (8.1.49)$$

In order to plot the spectrum, Poisson's ratio must be specified, since it enters into (8.1.46) through k^2 .

The curves for the real branches of the spectrum are seen to be hyperbolas. The cutoff frequencies, given by $\bar{\xi} \rightarrow 0$, are $\Omega = km, n$ for the P and SV waves. For $\Omega < km$, the wavenumbers become imaginary. Replacing $\bar{\xi}$ by $i\bar{\xi}$ in (8.1.46), it is seen that the P wave branches of the spectrum are ellipses. For $\Omega < n$, the wavenumbers of the respective SV waves become imaginary and, it is seen from (8.1.49), in the form of circles. The resulting spectrum, plotted for $\nu = 0.31$, so that $k^2 = 1.91$, is shown in Fig. 8.5.

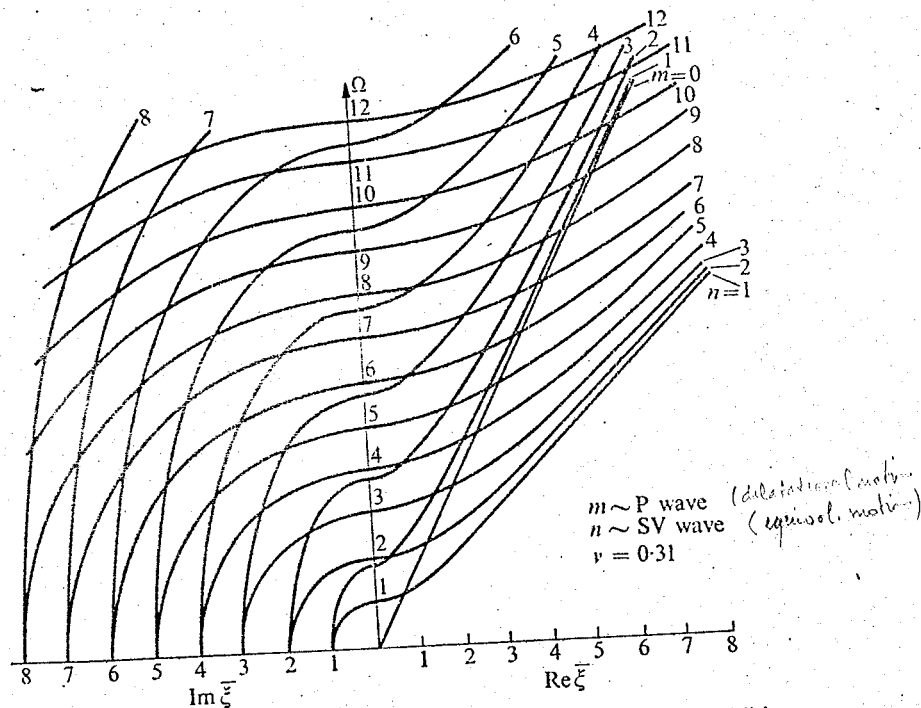


FIG. 8.5. Frequency spectrum for waves in a plate with mixed boundary conditions.

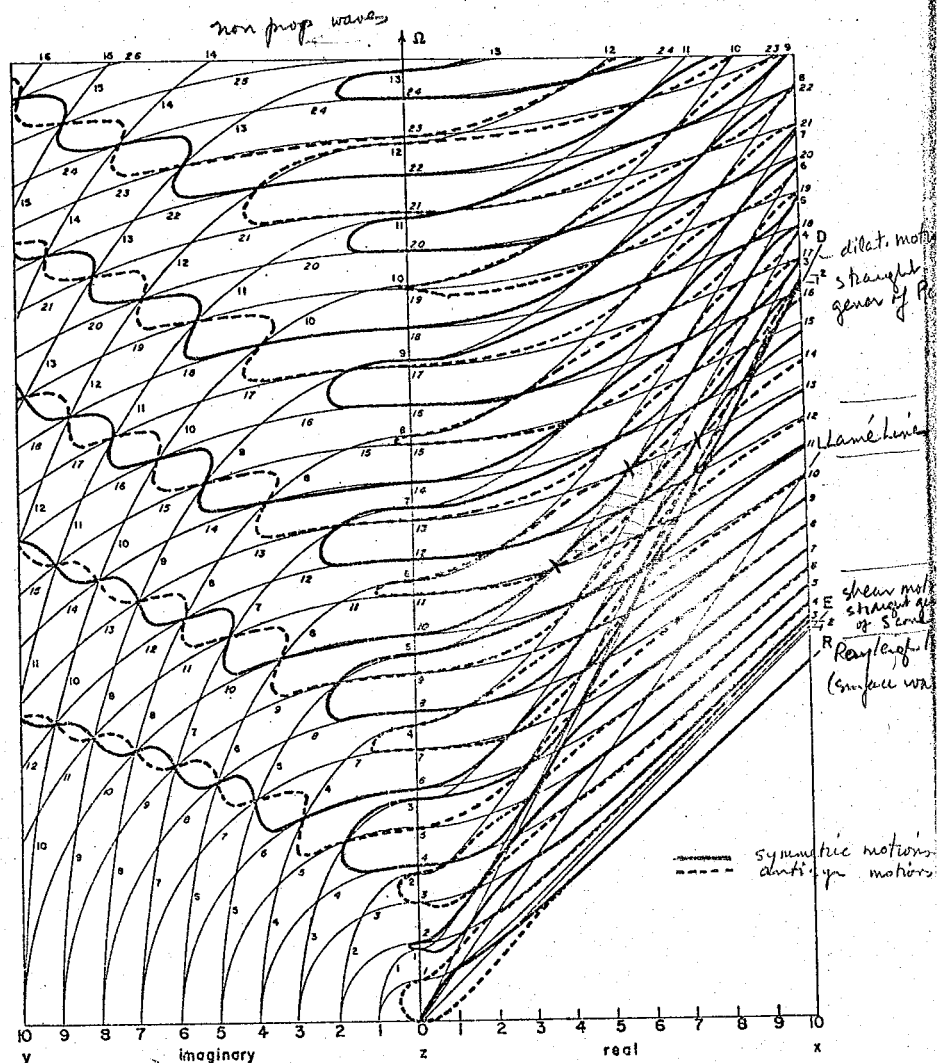


Fig. 19. Frequency spectrum of an infinite plate for real and imaginary wave-numbers ($\nu=0.31$).

quencies which fall on the curves $m=\text{constant}$ and $n=\text{constant}$, in Figs. 18 and 19, only at the intersections of curves m odd with curves n odd. If $e > 0$, $\sin \alpha b = 0$ and $\sin \beta b = 0$ are not solutions of (85) since both of (85) cannot then be satisfied by a single ratio A/D . Hence, the branches of (85) do not pass through the intersections of curves m even, n even as long as $e > 0$.

The behavior of a branch of (85), as e passes from infinity (mixed boundary conditions) to zero (traction-free boundaries) is illustrated in Fig. 20 for a portion of the spectrum. In this portion, the curves $m=5$, $n=13$ correspond to mixed boundary conditions ($e=\infty$). The two dashed curves marked $0 < e < \infty$ are for successive values of e , the lower curve corresponding to the

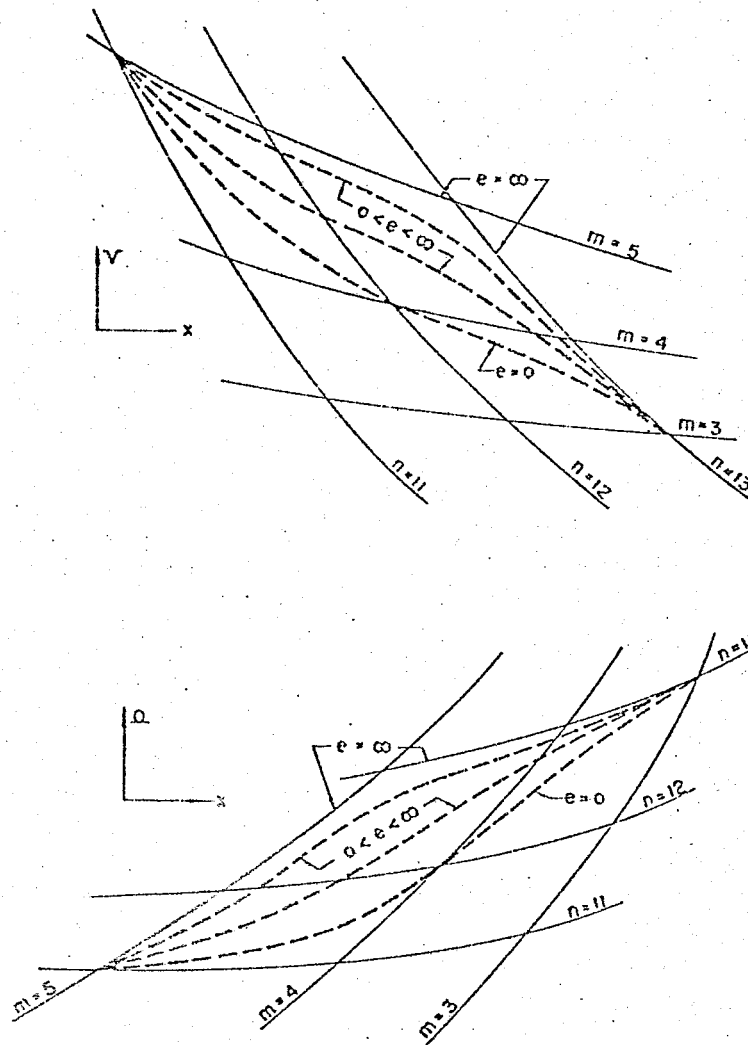
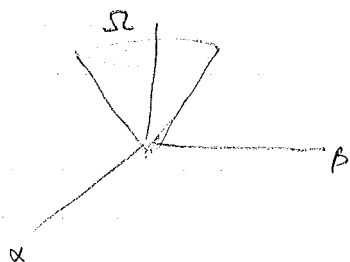


Fig. 20. Effect on the phase-velocity and frequency spectra of the antisymmetric modes of an infinite plate of the development of coupling between dilatational and equivoluminal overtones, as a result of relaxation of boundary constraint.

suppose we want to plot Ω vs. α, ξ for dilatational mode



because of Bdy we only have selected values
thus $\alpha = \alpha(\xi)$ is a right circular cone.

$$\alpha^2 = \xi^2 \left(\frac{c_1^2}{c_2^2} - 1 \right) \quad \text{for const } \alpha$$

we get sol of hyperbolas

now $\beta^2 = \xi^2 \left(\frac{c_1^2}{c_2^2} - 1 \right)$ is a cone also but
because $c_1, c_2 \neq$ then it must be an
elliptic cone.

the intersection of these cones will be our solutions

in the anti plane strain picking β in a good method satisfies BC. & we only have
to satisfy the α eqn [ie motion of hyperbola ($\alpha = \text{const}$) on a right circular
cone]

Mindlin: "Waves & Vibrations in Isotropic Elastic Plates" Proc. First Symposium
on Naval Struct. Mech.; Pergamon Press, 1960. Pg 199-232.

He considered

$u_x = u_y = 0$

$u_x = 0, \sigma_{xy} = 0$ lubricated surface

$u_x = u_y = 0$

if we write $e u_x = \sigma_{xy}$ & let $0 \leq e < \infty$ $m=5, n=12$ & $m=3, n=13$
satisfy conditions

Approximate theories based on freq spectrum we can restrict our freq so that
we can keep only lowest modes of either sym or antisym modes.

Let $\Omega = \sum a_i \xi^i$ $a_0 = 0$. If we only keep 2 terms then
for the ~~extensional~~ case
flexural $\omega = \xi^2 b \sqrt{\frac{E}{3\rho(1-\nu^2)}}$ $\Omega = \pi z^2 \sqrt{\frac{1}{3}(1-\frac{1}{8}\frac{1}{\delta^2})}$

for the ~~flexural~~ case
extensional $\omega = \xi \sqrt{\frac{E}{\rho(1-\nu^2)}}$

and $\Omega = 2z^2 \sqrt{1 - \frac{1}{8}\frac{1}{\delta^2}}$

Using method of sep. of variables

$$\text{Let } u(x,t) = X(x)T(t) \Rightarrow c^2 X''T = X\ddot{T}$$

$$c^2 \frac{X''}{X} = \frac{\ddot{T}}{T} = \text{const} = -\omega^2$$

ω is the circular freq.

$$\Rightarrow X'' + \frac{\omega^2}{c^2} X = 0 \quad \ddot{T} + \omega^2 T = 0$$

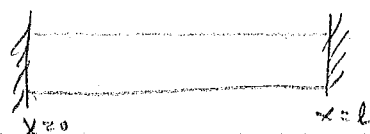
$$X(x) = D \sin \frac{\omega x}{c} + C \cos \frac{\omega x}{c}$$

$$T(t) = A \cos \omega t + B \sin \omega t$$

$\sin, \cos$ are principal mode of linear system are orthogonal each other

Specify BC.

of two



fixed ends

$$u(0,t) = u(l,t) = 0$$

$$\Rightarrow C=0 \quad \frac{\omega l}{c} = n\pi \quad : \quad \omega_n = \frac{n\pi c}{l}$$

A, B will be determined as a fun of initial conditions

$$\therefore u(x,t) = \sum X_n(x)T_n(t)$$

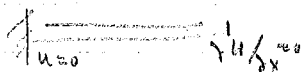
Free ends

$$\left. \frac{\partial u}{\partial x} \right|_{x=0} = 0$$

$$D=0$$

$$; \quad \omega_n = \frac{n\pi c}{l}$$

Fixed-Free



$$C=0 \quad \omega_n = \frac{n\pi c}{2l}$$

$$n=1,3,5,7,\dots$$

Initial conditions

$$u(x,0) = u_0(x)$$

$$\frac{\partial u}{\partial t}(x,0) = \bar{u}_0(x)$$

$$T(0) = \sum A_n X_n = u_0(x) \quad \left\{ \begin{array}{l} \text{mult both sides by } X_r(x) \text{ \& integrate } \int_0^l \end{array} \right.$$

$$\dot{T}(0) = \sum \omega_n B_n X_n = \bar{u}_0(x)$$

$$= \int_0^l \sum = \sum A_n \int_0^l X_n X_r dx$$

if $r \neq n$

$$A_r = \frac{\int_0^l u_0(x) X_r(x) dx}{\int_0^l X_r^2(x) dx}$$

$$\boxed{\text{also } B_r = \frac{\int_0^l \bar{u}_0(x) X_r(x) dx}{\omega_r \int_0^l X_r^2(x) dx}}$$

$$\text{only good iff } \int_0^l X_r X_n dx = \begin{cases} 0 & r \neq n \\ \neq 0 & r = n \end{cases}$$

now $\int_0^L \bar{X}_r^2 dx = ?$ use D.E.

now: $-\frac{\omega_n^2}{c^2} \bar{X}_n = \bar{X}_n'' \Rightarrow -\frac{\omega_n^2}{c^2} \int_0^L \bar{X}_n \bar{X}_r dx = \int_0^L \bar{X}_n'' \bar{X}_r dx$

$$= \bar{X}_r \bar{X}_n' \Big|_0^L - \int_0^L \bar{X}_r' \bar{X}_n' dx$$

$$= \bar{X}_r \bar{X}_n' \Big|_0^L - \bar{X}_r' \bar{X}_n \Big|_0^L + \int_0^L \bar{X}_r' \bar{X}_n' dx$$

but $\bar{X}_r'' = -\frac{\omega_r^2}{c^2} \bar{X}_r$

$$\therefore -\frac{\omega_r^2}{c^2} \int_0^L \bar{X}_r \bar{X}_n dx = \left\{ \bar{X}_r \bar{X}_n' \Big|_0^L - \bar{X}_r' \bar{X}_n \Big|_0^L \right\} \frac{c^2}{\omega_r^2 - \omega_n^2}$$

free end $\bar{X}_n' = \bar{X}_r' = 0$
 fixed end $\bar{X}_r = \bar{X}_n = 0$ } on body. $\Rightarrow \int_0^L \bar{X}_n \bar{X}_r dx = 0$ if $\omega_r \neq \omega_n$
 $r \neq n$

Forced motions

We can only allow ~~only~~ shear forces acting in the direction of central axis. define F = resultant shear / unit length.

$$P \leftarrow \boxed{F dx} \rightarrow P + \frac{\partial P}{\partial x} dx \quad \left(\frac{\partial P}{\partial x} + F \right) dx = \rho A \frac{\partial^2 u}{\partial t^2} dx$$

$$\therefore E \frac{\partial^2 u}{\partial x^2} - \rho \frac{\partial^2 u}{\partial t^2} = -\frac{F}{A}$$

or $c^2 \frac{\partial^2 u}{\partial x^2} - \frac{\partial^2 u}{\partial t^2} = -\frac{F}{\rho A}$

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Forced motions (continued)

$$c^2 u'' - u = -\frac{P(x,t)}{\rho A} \quad \text{How to solve}$$

Assume we solve the free vibs problem: Now let the solns be $X_n(x) q_n(t) = w_n(x,t)$

$$u(x,t) = \sum X_n q_n(t) \quad \& \quad \sum w_n(x,t) = -P/\rho A$$

put into DE to get $c^2 \sum X_n'' T_n - \sum X_n T_n'' = -\sum q_n(t) X_n(x)$

or $c^2 X_n'' T_n = (T_n'' - q_n) X_n$

$$q_n = \frac{\int X_n P/\rho A dx}{\int X_n^2 dx}$$

$$\therefore c^2 \frac{X_n''}{X_n} = \frac{T_n'' - q_n}{T_n} = -\omega_n^2$$

$$T_n'' + \omega_n^2 T_n = -g_n(t)$$

Solve homogeneous: $T_n(t) = A_n \cos \omega_n t + B_n \sin \omega_n t$

Particular: $T_{np}(t) = -\frac{1}{\omega_n} \int_0^t g_n(\tau) \sin(\omega_n(t-\tau)) d\tau$

$$\therefore T_n = T_{np} + T_{nh}$$

Example: Given a built-in bar fixed at one end with a periodic forcing function

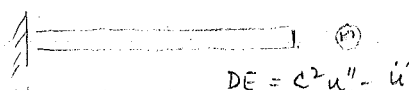


$$g_n(t) = \frac{2P_0}{\rho A l} \sin \omega_f t \sin \frac{n\pi d}{2l} \quad n = \text{odd}$$

$$u(x, t) = \sum_{n=\text{odd}}^{\infty} \left\{ A_n \cos \omega_n t + B_n \sin \omega_n t \right.$$

$$\left. + \frac{2P_0 \sin \frac{n\pi d}{2l}}{\rho A l} \left[\frac{-\omega_f}{\omega_n^2 - \omega_f^2} \sin \omega_n t - \sin \omega_f t \right] \sin \frac{n\pi x}{l} \right\}$$

Let's consider time-dependent bdy conditions



$x=0$

$x=l$

$$D_x u(t) = f_2(t)$$

$$BC: D_0 u(t) = f_1(t)$$

$$D_{0,l} \equiv \frac{\partial}{\partial x}$$

if displ is prescribed
if force pres

$$\text{Let } u(x, t) = \bar{u}(x, t) + f_1(t)g_1(x) + f_2(t)g_2(x)$$

apply $\frac{DE}{\frac{\partial}{\partial x}}$ $\Rightarrow c^2 \bar{u}'' - \bar{u}'' = -c^2 f_1 g_1'' - c^2 f_2 g_2'' + \ddot{f}_1 g_1 + \ddot{f}_2 g_2$

$$BC \Rightarrow D_0 [\bar{u}(0, t)] = f_1 - f_1 D_0 g_1(0) - f_2 D_0 g_2(0) = f_1 \{1 - D_0 g_1(0)\} - f_2 D_0 g_2(0)$$

$$D_l [\bar{u}(l, t)] = \{f_2 - f_1 D_l g_1(l) - f_2 D_l g_2(l)\} = f_2 \{1 - D_l g_2(l)\} - f_1 D_l g_1(l)$$

how can we cause BC to be zero and get rid of difficulty due to nonhom. BC?

since f_1 & f_2 are independent. $\Rightarrow$ each must be independent.

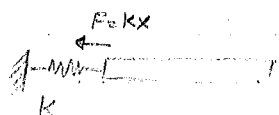
Then $f_1(1 - D_0 g_1(0)) = 0 \Rightarrow D_0 g_1(0) = 1$
 $f_2 D_0 g_2(0) = 0$
 $f_2(1 - D_l g_2(l)) = 0$
 $f_1 D_l g_1(l) = 0 \Rightarrow D_l g_1(l) = 0$

Choose

then naturally choose
any fn to satisfy
BC

similarly for g_2 ; put into DE. This will give a non-homo DE on $\bar{u}$ w/ homog. boundary conditions. Now we can use our previous results.

Example:



if K is soft assume bar is rigid & we have one-degree of freedom system.

Suppose:



the force $F = M \cdot \ddot{u}(l)$; but $\int X_n X_r dx \neq 0$ will be taken into BC

Bar Eqn of motion:

$$P' = \rho A \ddot{u} \quad 0 \leq x < l$$

$$P = A E u'$$

Mass Eq of motion:

$$-P = M \ddot{u} \quad x = l$$

$$P_n = A E X_n'$$

$$P_n' = -\rho A \omega_n^2 X_n \quad 0 \leq x < l \quad (*)$$

$$-P_n = -M \omega_n^2 X_n \quad x = l \quad (**)$$

now take $\int_0^l P_n' X_r dx = -\rho A \omega_n^2 \int_0^l X_n X_r dx \quad 0 \leq x < l$ from (*)

$$\Rightarrow P_n X_r \Big|_0^l - \int_0^l P_n X_r' dx = -\rho A \omega_n^2 \int_0^l X_n X_r dx \quad (†)$$

now $P_n X_r \Big|_0^l = +M \omega_n^2 X_n X_r \Big|_0^l$ from (**) (††)

also $-\rho \omega_r^2 A \int_0^l X_n X_r dx = \int_0^l P_r' X_n dx = P_r X_n \Big|_0^l - \int_0^l P_r X_n' dx \quad (†††)$

also $-M \omega_r^2 X_n X_r \Big|_0^l = -P_r X_n \Big|_0^l \quad (††††)$

taking (†) and (††) and (†††) & (††††)

$$\rho A (\omega_n^2 - \omega_r^2) \int_0^l X_n X_r dx + M (\omega_n^2 - \omega_r^2) X_n X_r \Big|_0^l = -P_n X_r \Big|_0^l + \int_0^l P_n X_r' dx \quad (1)$$

but $P_r = A E X_r'$ $P_n = A E X_n'$

$$+ P_r X_n \Big|_0^l - \int_0^l P_r X_n' dx \quad (2)$$

$$+ P_n X_r \Big|_0^l - P_r X_n \Big|_0^l$$

$\Rightarrow$ ① cancels ②

but rest of RHS is

$$-P_n X_r \Big|_0^l + P_r X_n \Big|_0^l + P_n X_r \Big|_0^l - P_r X_n \Big|_0^l = \frac{E A X_r'}{P_r X_n} - \frac{E A X_n'}{P_n X_r}$$

at built end $\Rightarrow X_r = 0 \quad X_n = 0$

at free end $X_n' = 0 \quad X_r' = 0$

then for $\omega_r \neq \omega_n \Rightarrow$

$$\rho A \int_0^l X_n X_r dx + M X_r X_n \Big|_0^l = 0$$

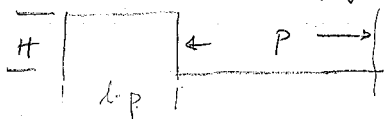
for homog. solution $T_n(t) = A_n \cos \omega_n t + B_n \sin \omega_n t$
 $\Rightarrow$ we can get the A_n & B_n using the relation just found if $u(x,0) = u_0(x)$

then $A_n = \frac{\int_0^L X_n u_0 dx + R L X_n(L) u_0(L)}{\int_0^L X_n^2 dx + R L X_n^2(L)}$ $R L = M$

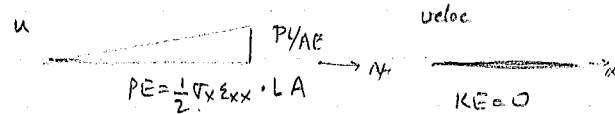
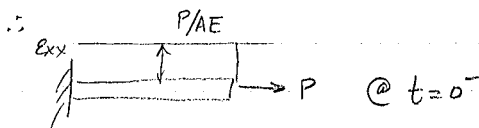
Look at homework (Problem Set #2) Ex #1

$$E_{xx} = \frac{\partial u}{\partial x} = \frac{P}{AE} \frac{4}{\pi} \sum_{n=1,3,\dots} \frac{(-1)^{\frac{n-1}{2}}}{n} \cos \frac{n\pi x}{2L} \cos \frac{n\pi c t}{2L}$$

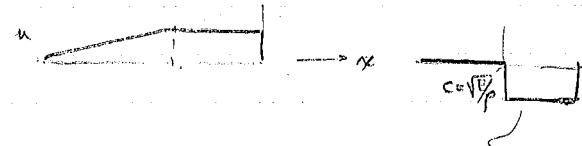
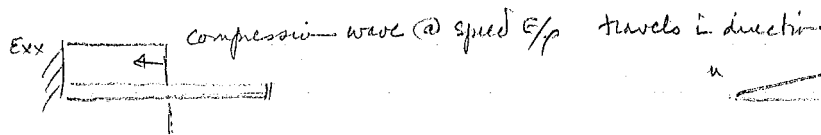
look at heaviside step fn. $\Rightarrow H(x) - H(x-p)$



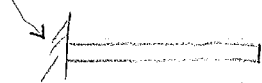
$$H = \frac{4}{\pi} \sum_{n=1,3,5} \frac{(-1)^{\frac{n-1}{2}}}{n} \cos \frac{n\pi p}{2L} \cos \frac{n\pi x}{2L}$$



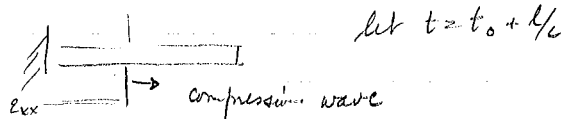
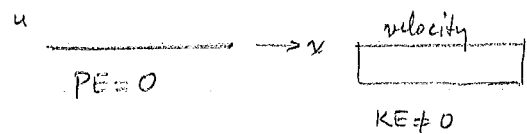
@ $t=t_0$



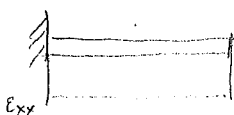
@ $t = L/c$ $\cos \frac{n\pi c t}{2L} = 0$



beam is undisturbed

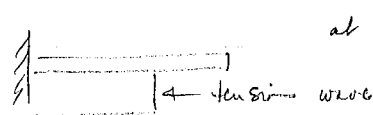


let $t = t_0 + L/c$

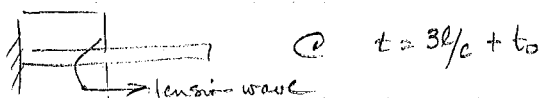


@ $t = 2L/c$ $\cos \frac{n\pi c t}{2L} = -1$

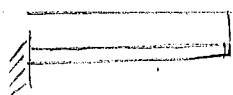
wave has reached 2L end of bar, bar in uniform compression



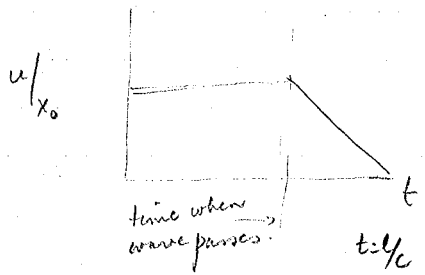
at $t = t_0 + 2L/c$



@ $t = 3L/c + t_0$

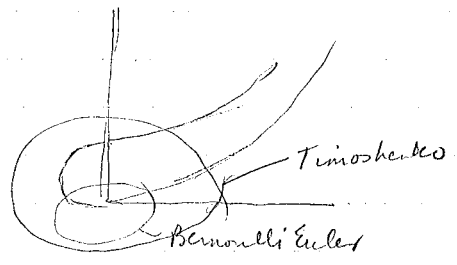


@ $t = 4L/c$ bar is in uniform tension again

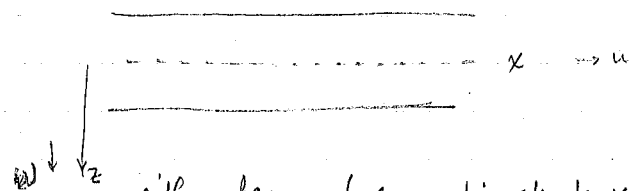


11/20/79

Different theories



Will use Hamilton Principle to obtain Timo Beam Theory then by reducing it we get Bernoulli-Euler



either beam (symmetric about x-z plane so no twisting) of plate in plane strain

Introduce displ $u_i = u_i(x, y, z, t)$

Let $u_{i,2,3} = u, v, w$. Replace the exact u_i by approx $\bar{u}_i$ by restriction of possible motions

Let $\bar{u} = -z\psi(x, t)$ only

$$\bar{v} = 0$$

$$\bar{w} = w(x, t)$$

To derive the eqns of motion using Hamilton Principle. Look at Lagrangian between t_1 & t_2 . The actual motion is such that $\delta \int_{t_1}^{t_2} L dt = 0$

$$L = KE - PE = T - U$$

$$\delta L = \delta T - \delta U$$

$$\delta U = \sigma_{ij} \delta \epsilon_{ij}$$

$$\delta U = \delta V - \delta W$$

elastic strain energy work done by applied external forces.

$$W = \int (T_i u_{i,d} + B_i u_i) dV$$

$$\delta W = \int (T_i \delta u_{i,d} - B_i \delta u_i) dV$$

Force/unit Area body force/vol.

If we use the unconstrained u_i then we would get from Hamilton's Principle the eqns of motion.

$$\begin{aligned}\bar{\epsilon}_{xx} &= -z \frac{\partial \psi}{\partial x} & \bar{\epsilon}_{yy} &= 0 & \bar{\epsilon}_{zz} &= 0 \\ \bar{\epsilon}_{yz} &= 0 & \bar{\epsilon}_{zx} &= \frac{1}{2}(w' - \psi) & \bar{\epsilon}_{xy} &= 0\end{aligned}$$

per unit
volume
unit time

$$\delta \bar{V}^* = -\bar{\sigma}_{xx} z \delta \psi' + \bar{\sigma}_{zx} \cdot \frac{1}{2} (\delta w' - \delta \psi) \cdot 2$$

$$\text{since } \sigma_{zx} \epsilon_{zx} + \sigma_{xz} \epsilon_{xz} = 2\sigma_{xz} \epsilon_{xz}$$

$$\delta V = \int \delta V^* dx dy dz \quad \text{dependence on } z \text{ \& } y \text{ is explicit for } \delta \bar{V}^*$$

$$\therefore \delta \bar{V} = \int_0^L \left[\underbrace{\iint_{\text{Area}} (\bar{\sigma}_{xx} z) dy dz}_{\text{moment}} \delta \psi' dx + \int_0^L \left[\iint_{\text{Area}} \bar{\sigma}_{zx} dy dz \right] (\delta w' - \delta \psi) dx \right]$$

Q - total shear force.

$$\therefore \delta \bar{V} = \int_0^L \left[-M \delta \psi' dx + Q (\delta w' - \delta \psi) \right] dx$$

$$\text{integrate by parts} \quad \int_0^L [M' \delta \psi - Q' \delta w - Q \delta \psi] dx - M \delta \psi \Big|_0^L + Q \delta w \Big|_0^L$$

we have assumed: $\delta(\psi') = (\delta\psi)'$ variation & differentiation are interchanging

$$\bar{T}^* = \rho_2 \dot{u}_i^2 \quad \delta \bar{T}^* = \rho \dot{u}_i \delta \dot{u}_i \quad \delta \bar{T}^* = \rho [z^2 \dot{\psi} \delta \dot{\psi} + \dot{w} \delta \dot{w}]$$

$$\therefore \delta \bar{T} = \int_{\text{vol}} \delta \bar{T}^* dx dy dz \quad \text{again } z \text{ \& } y \text{ dependence is explicit}$$

$$\text{Let } \int_{\text{area}} z^2 dy dz = I \quad \int_{\text{area}} dy dz = A$$

$$\delta \bar{T} = \int_0^L [\rho I \dot{\psi} \delta \dot{\psi} + A \dot{w} \delta \dot{w}] dx$$

$$\int_{t_1}^{t_2} \delta \bar{T} dt = \int_0^L \rho \int_{t_1}^{t_2} (I \dot{\psi} \delta \dot{\psi} + A \dot{w} \delta \dot{w}) dx dt \quad \text{use integration by part.}$$

$$\int_{t_1}^{t_2} \delta \bar{T} dt = \int_0^L \int_{t_1}^{t_2} (-I \ddot{\psi} \delta \psi - A \ddot{w} \delta w) dx dt + \left[\int_0^L \rho (I \dot{\psi} \delta \psi + A \dot{w} \delta w) dx \right]_{t_1}^{t_2}$$

by Ham- Principle the body term is zero since we allow no variation of the variables ψ , but if we keep them then we deduct to permissible BC.

to get external work done by forces (external traction & body forces)

$$\text{in general } \delta W = \int_S \underbrace{t_{ni}}_{\text{stress tensor } \perp \text{ to surface}} \delta u_i ds + \int_{\text{vol}} \underbrace{b_i}_{\text{body forces}} \delta u_i dV$$

$$\delta W = \int_A \underbrace{(-F_x z \delta \psi + F_z \delta w)}_{\text{end faces}} dy dz \Big|_0^L + \int_0^L \oint_c \underbrace{(-F_x z \delta \psi + F_z \delta w)}_{\text{cylindrical surface}} dy dz dx$$

$$b_i = \bar{x}_i \quad \text{where } \bar{x}_1 = \bar{x}, \bar{x}_2 = \bar{y}, \bar{x}_3 = \bar{z} \quad \text{body free.}$$

$$+ \int_0^L \iint_{\text{Area}} (-\bar{x} z \delta \psi + \bar{z} \delta w) dx dy dz$$

$$\int_0^L (-\bar{x} \delta \psi + \bar{z} \delta w) dx \quad \bar{x} = \int x z dy dz \quad \bar{z} = \int z dy dz$$

but: $\oint_C f_x z dy dz = M_s$ bending moment due to applied axial shear loads

and: $\oint_C f_z dy dz = q$ is the net transverse load on cylindrical surface $\int_A F_x z dy dz = M$
 $\int_A F_z dy dz = Q$

$$\therefore \delta W = \underbrace{[-M \delta \psi + Q \delta w]}_{\text{applied moment + shear due to shear on ends}} + \int_0^L (-m_s \delta \psi + q \delta w) dx + \int_0^L (-\bar{x} \delta \psi + \bar{z} \delta w) dx$$

Put all these into Hamilton's principle

$$0 = \int_0^L \int_{t_1}^{t_2} (-\rho I \ddot{\psi} \delta \psi - \rho A \ddot{w} \delta w - M' \delta \psi + Q' \delta w + Q \delta \psi - m_s \delta \psi + q \delta w - \bar{x} \delta \psi + \bar{z} \delta w) dx dt$$

+ boundary terms. Since variations $\delta \psi, \delta w$ are arbitrary we want δ independent of each other $\Rightarrow$ their coeffs must vanish.

$$\left. \begin{aligned} -M' + Q - m_s - \bar{x} &= \rho I \ddot{\psi} \delta \psi \\ Q' + q + \bar{z} &= \rho A \ddot{w} \delta w \end{aligned} \right\} \text{w/ bc \& IC}$$

normally M, Q, w, ψ are unknown thus we must find material eqns to define other 2 eqns.

$\Rightarrow \sigma_{ij} = \lambda \epsilon_{11} \delta_{ij} + 2\mu \epsilon_{ij}$ what must they become using approx theory keeping in mind that result must be physically sensible

$$\sigma_{xx} = (\lambda + 2\mu) \epsilon_{xx} + \lambda (\epsilon_{yy} + \epsilon_{zz})$$

$$\epsilon_{yy} + \epsilon_{zz} = \frac{\sigma_{yy} + \sigma_{zz}}{2\lambda + \mu} - \frac{\lambda}{\lambda + \mu} \epsilon_{xx}$$

$$\Rightarrow \sigma_{xx} = \left[\frac{(\lambda + 2\mu)(\lambda + \mu) - \lambda^2}{\lambda + \mu} \right] \epsilon_{xx} + \frac{\lambda}{2(\lambda + \mu)} (\sigma_{yy} + \sigma_{zz})$$

$$3\lambda + 2\mu^2$$

$$E = \frac{\lambda(\lambda + 2\mu)}{\lambda + \mu}$$

we now ask which do we satisfy i.e. $\sigma_{yy} + \sigma_{zz} = 0 \sim \epsilon_{yy} + \epsilon_{zz} = 0$ when bar bends we expect free expansion due to poisson ratio $\Rightarrow \epsilon_{yy} + \epsilon_{zz} \neq 0$ but M is a fn of σ_{xx}

$\therefore$ we can use a weaker condition that $\int (\sigma_{yy} + \sigma_{zz}) dy dz = 0$

$$E(-2\psi') \int z dy dz$$

$$\therefore M = \int_{\text{Area}} -E z^2 \psi' dy dz + \frac{\lambda}{2(\lambda + \mu)} \int_{\text{Area}} (\sigma_{yy} + \sigma_{zz}) dy dz$$

Thus we violate compatibility in order to get a better stress strain relation.

thus $M = -EI\psi'$

now $\int \bar{\sigma}_{zx} dy dz = Q$

$$\int \sigma_{xy} z dy dz = +E \int \epsilon_{xx} z dy dz$$

$$= E \int (-z^2 \psi') dy dz = -EI \psi'$$

$$\bar{\epsilon}_{zx} = \frac{1}{2}(w' - \psi) \Rightarrow \bar{\sigma}_{zx} = 2\mu \bar{\epsilon}_{zx} = \mu(w' - \psi)$$

$$\int \bar{\sigma}_{zx} dy dz = Q = AG(w' - \psi)$$

$$G = \mu$$

since w, ψ are not fn. of y, z

BC $-M\delta\psi|_0^L + Q\delta w|_0^L$

from 3-d elasticity we get $t_i \delta u_i$ on S .

must specify M or ψ and Q or w .

11/27/79

Timoshenko Beam - revisited

$$-M' + Q + m_s - \bar{X} = \rho I \ddot{\psi} \quad \text{Angular momentum.}$$

$$Q' + q + \bar{Z} = \rho A \ddot{w}$$

$$M = -EI\psi'$$

$$Q = AG(w' - \psi)$$

To obtain Bernoulli-Euler Eqn.

let rotatory inertia $\ddot{\psi} / m_s, \bar{X}, \bar{Z}$ all be ignored

from 1: $-M' + Q = 0 \Rightarrow -M'' + Q' = 0 \quad (3)$

from 2: $Q' + q = M'' + q = \rho A \ddot{w} \Rightarrow \rho A \ddot{w} - M'' - q = 0$

$\Rightarrow w' = \psi$ hence there is no shear strain but $Q \neq 0$. Hence where do we get Q from (3) since $Q = M'$. (Note $Q = AG(w' - \psi)$ is inoperative Q is inoperative $\Rightarrow Q$ is shifted from stress-strain law in Timmo Beam theory to a statement of equilibrium). Since $Q \neq 0 \Rightarrow G = \infty$; we assume beam is infinitely rigid.

$$\therefore M = -EI\psi' = -EIw'' \quad Q = M' = -EI\psi'''$$

$$-M'' + \rho A \ddot{w} = q \Rightarrow EIw'''' + \rho A \ddot{w} = q$$

let $q = 0$ the for free vibration $\frac{EI}{\rho A} w'''' + \ddot{w} = 0$

Note $\left\{ \begin{array}{l} \text{BC \& IC remain same} \\ \Rightarrow Mw' \& Qw \end{array} \right.$ remember we had to specify $M\psi$ or Qw and EIw''' or w must be specified

let $\frac{EI}{\rho A} = a^2 \therefore a^2 w'''' + \ddot{w} = 0$

if $w(x,t) = X T \Rightarrow a^2 X'''' T + X T'' = 0 \Rightarrow a^2 \frac{X''''}{X} = - \frac{T''}{T} = \omega^2$
 thus $T = A \cos \omega t + B \sin \omega t$

if $\omega = \frac{m^4}{l^4} a^2 \Rightarrow X'''' - \frac{m^4}{l^4} X = 0$ where $l = \text{length of beam}$.

let $e^{\lambda x} = X \Rightarrow \lambda^4 - \frac{m^4}{l^4} = 0 \therefore \left(\lambda^2 - \frac{m^2}{l^2}\right)\left(\lambda^2 + \frac{m^2}{l^2}\right)$

$\Rightarrow X = C \cos \frac{mx}{l} + D \sin \frac{mx}{l} + E \cosh \frac{mx}{l} + F \sinh \frac{mx}{l}$ (*)

Consider SS beam: $w=0$ & $w''=0$ @ $x=0, l$ $\Rightarrow \left. \begin{array}{l} @x=0: C+E=0 \\ @x=l: C+E=0 \end{array} \right\} \Rightarrow \begin{array}{l} C-E=0 \\ \frac{m}{l} \neq 0 \end{array}$

@ $x=l$ $D \sin m + F \sinh m = 0$
 $\frac{m^2}{l^2} [D \sin m + F \sinh m] = 0$

det = $2 \sin m \sinh m = 0$
 for non trivial sol.
 $\Rightarrow m_n = n\pi$

$\omega_n^2 = \frac{n^4 \pi^4 a^2}{l^4}$

$\omega_n = \frac{n^2 \pi^2}{l^2} \sqrt{\frac{EI}{\rho A}}$

$X_n = \sin \frac{n\pi x}{l}$

Consider Cantilever beam: @ $x=0$ $w=0 \therefore w'=0$ no displ or rotation

$x=l$ $w''=0$ $w'''=0$ no mom / no shear

using (*)

$w(0)=0 \Rightarrow C_n + E_n = 0$

$w'(0)=0 \Rightarrow D_n + F_n = 0$

$w''(l)=0 \Rightarrow$ using $C_n = -E_n$ $D_n = -F_n$
 $-C_n (\cos m_n + \cosh m_n) - D_n (\sin m_n + \sinh m_n) = 0$
 $C_n (\sin m_n - \sinh m_n) - D_n (\cos m_n + \cosh m_n) = 0$

for non trivial $\cos m_n \cosh m_n = -1$ transcendental eq.

$m_1 = 1.875$

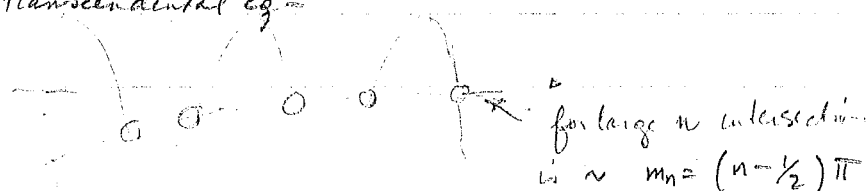
$m_2 = 4.694$

$m_3 = 7.855$

$m_4 = 10.996 \approx \frac{7}{2} \pi$

to get mode shapes put in m_n into eqns involving C_n & D_n to get ratio of C_n, D_n to get A_n & B_n for $Y = A_n \cos \omega_n t + B_n \sin \omega_n t$

I.e. @ $t=0$ $w(x,0) = w_0 = \sum A_n X_n$ using orthog $\int_0^l X_n X_r dx = \begin{cases} 0 & n \neq r \\ 1 & n = r \end{cases}$
 $\dot{w}(x,0) = \dot{w}_0$



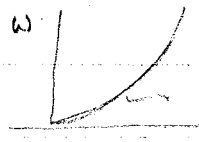
$$\therefore A_n = \frac{\int_0^l \omega_0 X_n dx}{\int_0^l X_n^2 dx}$$

$$B_n = \frac{\int_0^l \dot{\omega}_0 X_n dy}{\int_0^l X_n^2 dx}$$

for infinite beam. $a^2 w^{IV} + \ddot{w} = 0$ let $w(x,t) = W e^{ikx} e^{i\omega t}$

$$\Rightarrow a^2 k^4 - \omega^2 = 0 \quad \text{char eqn.}$$

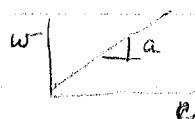
$$\omega = \pm a k^2$$



compare to lower branches of Rayleigh Lamb probs.

to show this is dispersive

$$c = \frac{\omega}{k} = \frac{ak^2}{k} = ak$$



for the finite beam we get non dispersive sol. propagates waves w/ same velocity,

" " infinite " " " dispersive sol. as $k \rightarrow 0$ $\lambda \rightarrow \infty \Rightarrow c \rightarrow 0$

infinite beam contains results of ss beam but not cantilever

For forced motions

$$a^2 w^{IV} + \ddot{w} = \frac{f}{A}(x,t)$$

1. Solve Homog. first in terms of X_n

2. Solve RHS in terms of X_n , i.e. expand $\therefore \frac{f}{A} = \sum X_n Q_n(t)$

$$Q_n(t) = \frac{\int_0^l \frac{f(x,t)}{A} X_n(x) dx}{\int_0^l X_n^2 dx}$$

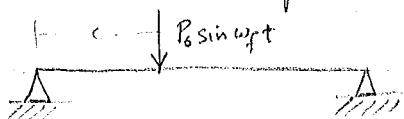
now the time dep part become, $T'' + \omega_n^2 T = Q_n(t)$

Solve the homog to get $T_h = A_n \cos \omega_n t + B_n \sin \omega_n t$

$$T_p = \frac{1}{\omega_n} \int_0^t Q_n(\tau) \sin \omega_n \left(\frac{1}{2} (t - \tau) \right) d\tau \quad \text{to include init conditions}$$

11/29/79

Forced Vibration - Example.



Assume steady state i.e. initial conditions play no part

$$q(x,t) = \sum Q_n(t) X_n(x) \quad Q_n = \frac{\int_0^l X_n q dx}{\int_0^l X_n^2 dx}$$

$$X_n = \sin \frac{n\pi x}{l}$$

$$\int_0^l X_n^2 dx = l/2$$

$$q(x,t) = P_0 \sin \omega t \delta(x-c)$$

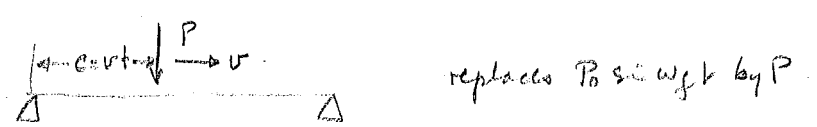
$$\therefore Q_n(t) = \frac{2}{l} P_0 \sin \omega_f t \sin \frac{n\pi c}{l}$$

$$\text{Now } T_n(t) = \frac{1}{\omega_n} \int_0^t Q_n(\tau) \sin \omega_n(t-\tau) d\tau$$

$$\therefore w(x,t) = \frac{2P_0 l^3}{\pi^4 EI} \sum_{n=1}^{\infty} \frac{\sin \frac{n\pi c}{l} \sin \frac{n\pi x}{l} \left(\sin \frac{\omega_f t}{f} - \frac{\omega_n}{\omega_f} \sin \omega_n t \right)}{n^4 (1 - \omega_f^2/\omega_n^2)}$$

if $\omega_f/\omega_n = 1 \Rightarrow$ no longer steady states but must give initial conditions

Suppose the load is not located at c $\forall t$ but moves w/ velocity v



$$P\delta(x-ct) = q(x,t) = \sum Q_n(t) X_n(x)$$

$$Q_n(t) = \frac{\int_0^l P\delta(x-ct) X_n(x) dx}{\int_0^l X_n^2 dx} = \frac{P \cdot \frac{2}{l} \sin \frac{n\pi vt}{l}}{\int_0^l X_n^2 dx}$$

$$\Rightarrow w(x,t) = \frac{2Pl^3}{\pi^4 EI} \sum_{n=1}^{\infty} \frac{\sin \frac{n\pi x}{l} \left(\sin \frac{n\pi vt}{l} - \frac{vl}{n\pi a} \sin \frac{n^2 \pi^2 at}{l^2} \right)}{n^4 \left[1 - \left(\frac{vl}{n\pi a} \right)^2 \right]} \quad a = \frac{EI}{\rho A}$$

To find vibrations after P moves past RH ~~support~~ ^{support}, use the u & u_t that exist @ the time P moves past RH support - use them as initial conditions for free vib problem

To find max. w , when $v \ll 1 \Rightarrow w_{max} = w_{max \text{ stat}}$ when $v \gg 1 \Rightarrow w_{max} = 0$ (no response time)

Control of first mode only, $n=1$ $\omega_1 = \pi^2/l^2$ if $v \gg$

$$w_1 = \frac{2P_0 l^3}{\pi^4 EI} \sin \frac{\pi x}{l} \left(\sin \frac{\pi vt}{l} - \frac{vl}{\pi a} \sin \frac{\pi^2 at}{l^2} \right) \frac{1 - \left(\frac{vl}{\pi a} \right)^2}{1 - \left(\frac{vl}{\pi a} \right)^2}$$

$$\text{Amplitude factor } A = \frac{w_{1, \text{max dyn}}}{w_{1, \text{max static}}} = \frac{96}{\pi^4} \left[\frac{\sin \frac{\pi vt}{l} - \frac{\pi v}{\omega_1 l} \sin \omega_1 t}{1 - \left(\frac{\pi v}{\omega_1 l} \right)^2} \right]$$

let $\beta = \frac{\omega_1 l}{v}$ $u = \frac{vt}{l} \Rightarrow A = \frac{96}{\pi^4} \frac{\sin \pi u - \frac{\pi}{\beta} \sin \beta u}{1 - (\pi/\beta)^2}$
 velocity positive of force

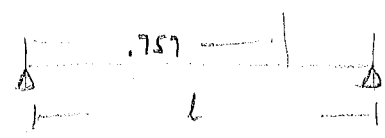
$\frac{dA}{dk} = 0$ (where is loc of load where A is max) $= \frac{26}{\pi^4} \frac{\pi \cos \pi k - \pi \cos \beta k}{1 - (\pi/\beta)^2} = 0$

$\Rightarrow \cos \pi k = \cos \beta k \quad \pi k = 2n\pi \pm \beta k$
 $k = \frac{2n\pi}{\pi \mp \beta}$

$n=0 \quad A = \frac{48}{\pi^3}$

$n=1 \quad A = A(\beta) \Leftrightarrow k = \frac{2\pi}{\pi + \beta} ; \text{ max for } \beta = 1.64\pi \quad A = 1.743$

Time of transit is .82 times the fundamental period of beam
 $k = .757$



$\Rightarrow$ dangerous velocity $= \frac{l}{t} = \frac{l}{.82 \text{ times the fund period of beam}}$
if it occurs .757 units along beam.

Forces acting on mech system that are not fixed will ~~have~~ perform work $= \int \mathbf{F} \cdot \mathbf{v} dt$
when $\mathbf{v}$ is velocity of particle of point under which the load is at, at time t

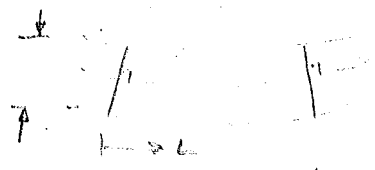
$W = P \int_0^{4\pi} \left(\frac{\partial w}{\partial t} \right)_{x=vt} dt$

12/4/79

Take homework exam: Given Thursday 12/6 return by Monday 12/10

Assumptions for elem beam theory

- 1. Plane sections remain plane & $\perp$ to central axis deformed & undeformed.



If $\lambda \gg h$ all plane's are pretty good
if $\lambda \ll h$ " are not clear.

- 2. Only account for transverse motion. If $h \ll l$ must include rotary motion

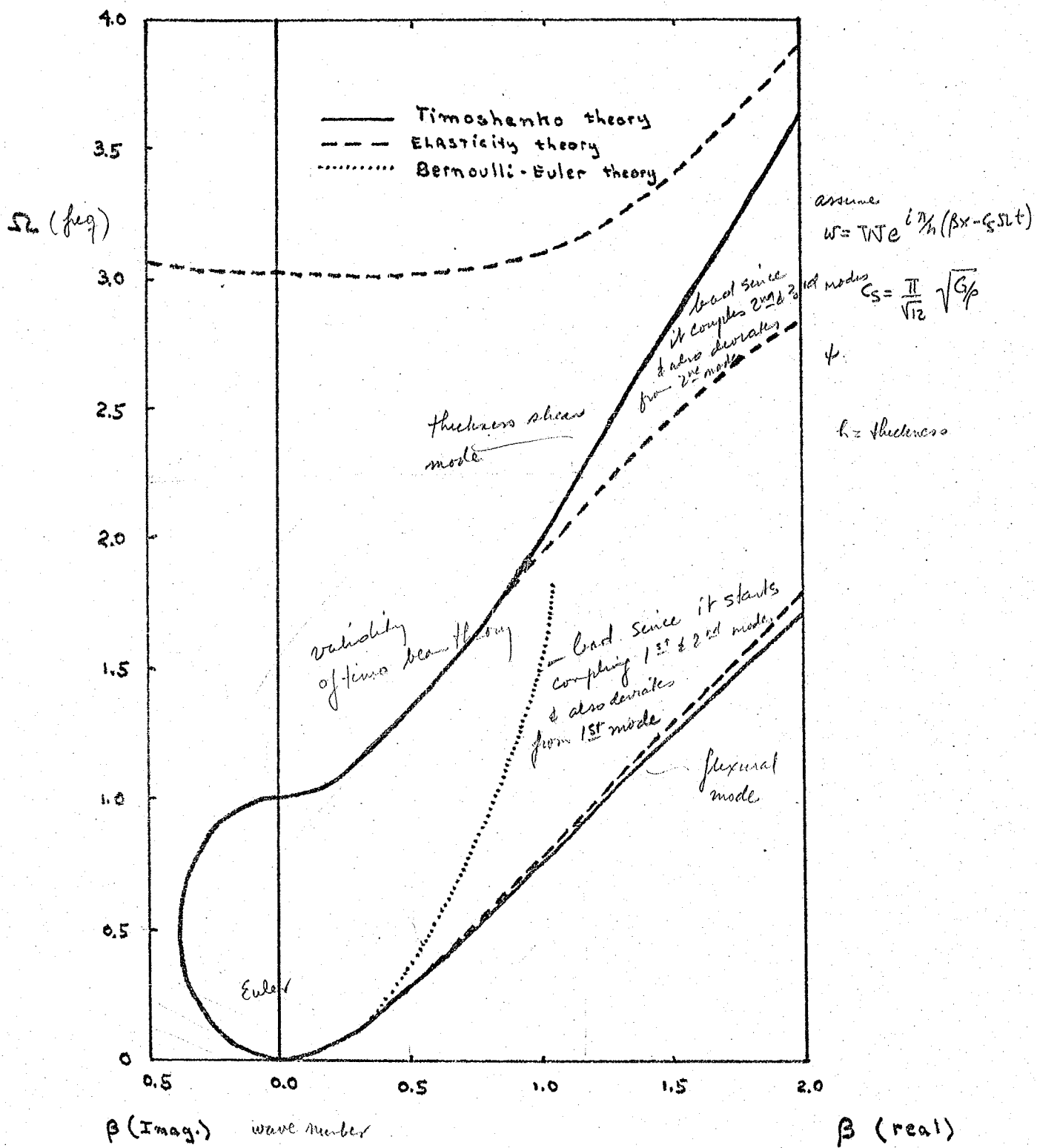
for β (wave number) $\rightarrow 0 \quad \lambda \rightarrow \infty \quad (\ddot{w}'' - \psi') \frac{G}{\rho} = \ddot{w} \Rightarrow w=0$

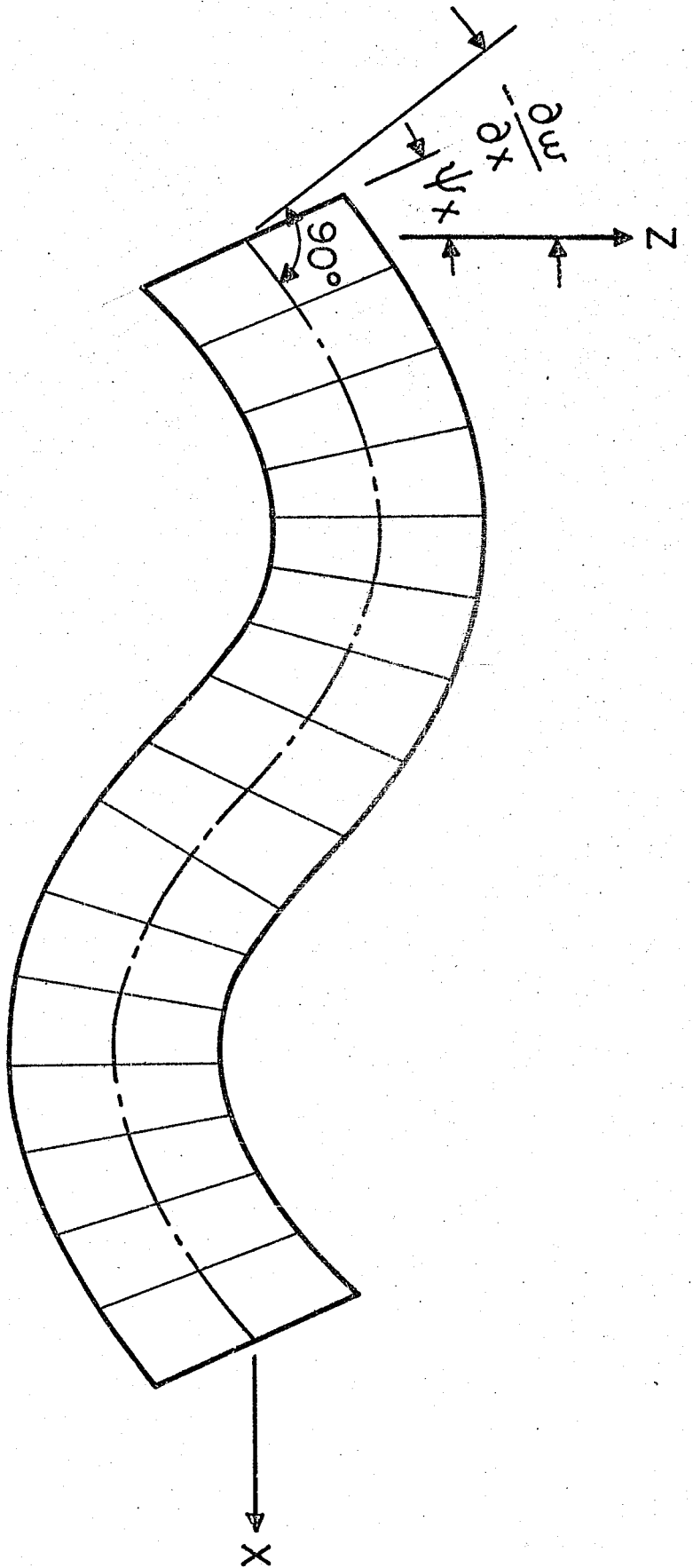
solns are independent of x

$\psi'' \rho + (w' - \psi) \frac{G}{\rho r^2} = \ddot{\psi}$

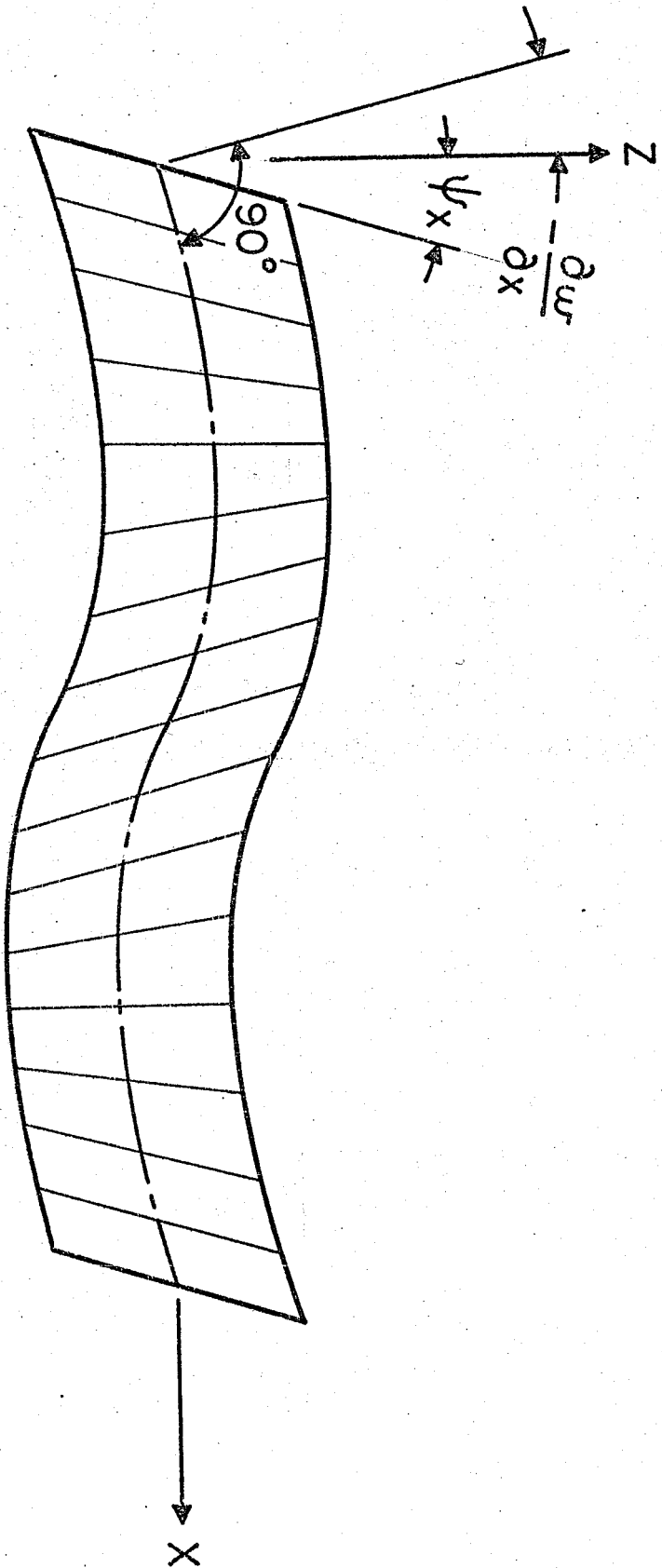
beam doesn't defo in w direction

$\Rightarrow \psi$ are sines & cosines solns are harmonic fns. $w = \frac{1}{\sqrt{G\rho}}$





(A) FLEXURAL MODE *primarily deflection*

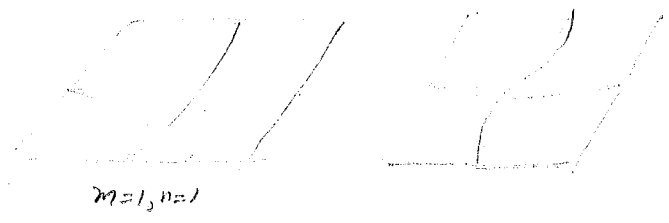


(E) THICKNESS - SHEAR MODE

primarily shear

$$\therefore \omega^2 = (j^2 + k^2) c^2 = \left(\frac{m^2 \pi^2}{b^2} + \frac{n^2 \pi^2}{a^2} \right) c^2 \quad \omega_{mn} = \pi c \sqrt{\frac{m^2}{a^2} + \frac{n^2}{b^2}}$$

$$\therefore \omega = \sum_{n=1}^{\infty} \sum_{m=1}^{\infty} X_n Y_m T_{nm}$$



for given ω we can have 2 mode shapes: $\omega = \frac{\pi c}{a} (n^2 + m^2)^{1/2}$ for a square

$$\omega_{9,2} = \frac{\pi c}{a} \sqrt{85} \quad \omega_{6,7} = \frac{\pi c}{a} \sqrt{85}$$

12/6/79

Rayleigh's Method



for cantilever beam

$$KE = \frac{A\rho}{2} \int_0^l \dot{w}^2 dx \quad PE = \frac{EI}{2} \int_0^l w''^2 dx$$

Based on element theory

assume a w to satisfy bc.

$$\text{let } w(x,t) = W x^2 \sin \omega t$$

$$\dot{w} = W \omega x^2 \cos \omega t \quad (\dot{w})^2 = W^2 \omega^2 x^4 \cos^2 \omega t$$

$$KE = \frac{A\rho W^2}{2} \int_0^l x^4 dx \cos^2 \omega t = A\rho W^2 \omega^2 \cos^2 \omega t \frac{l^5}{10}$$

$$w' = 2x W \sin \omega t \quad w'' = 2W \sin \omega t, PE = \frac{EI}{2} \cdot \int_0^l dx 4W^2 \sin^2 \omega t = 2EI \sin^2 \omega t W^2 l$$

$$T_{max} = KE = V_{max} = PE \quad \therefore \omega^2 = \frac{20EI}{A\rho l^4}$$

$$\text{take when } \sin \omega t = 1 = \cos \omega t : \frac{A\rho l^5}{10} \omega^2 = 2EI l W^2 \quad 4^4 \rho A$$

$$\omega^2 = \frac{20EI}{A\rho l^4}$$

$$\omega \sim 4.47 \sqrt{\frac{EI}{l^2 \rho A}}$$

$$\omega_{exact} \sim 3.5 \sqrt{\frac{EI}{l^2 \rho A}}$$

$$\text{if } w = W \left(\frac{3x^2}{2l^2} - \frac{x^3}{2l^3} \right) \sin \omega t$$

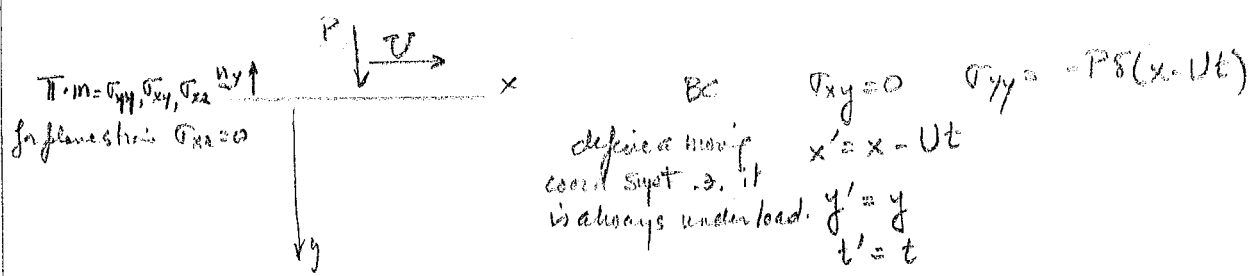
$$\omega \sim 3.55 \sqrt{\frac{EI}{l^2 \rho A}}$$

Stress Concentrations - Dynamic (D. fraction)

uniform static field has a stress concentration

Stress (dynamic) at cavity will cause a stress concentration

Get Timo / Young / Weaver Vib Prob in Eng.



write de in terms of pot.

$$\phi : \frac{\partial^2 \phi}{\partial x'^2} + \frac{\partial^2 \phi}{\partial y'^2} = \frac{U^2}{c_1^2} \frac{\partial^2 \phi}{\partial t'^2}$$

$$\text{only } H_3 \text{ pot} : \frac{\partial^2 H_3}{\partial x'^2} + \frac{\partial^2 H_3}{\partial y'^2} = \frac{U^2}{c_2^2} \frac{\partial^2 H_3}{\partial t'^2}$$

Let $M_1 = \frac{U}{c_1}$ $M_2 = \frac{U}{c_2}$ let $\beta_1 = \sqrt{1 - M_1^2}$ if $M_1 < 1$
 $\beta_2 = \sqrt{1 - M_2^2}$ if $M_2 < 1$

if $M_1, M_2 > 1 \Rightarrow \bar{\beta}_1 = \sqrt{M_1^2 - 1}$ $\bar{\beta}_2 = \sqrt{M_2^2 - 1}$

now $\sigma_{xx}/\mu = (M_2^2 - 2M_1^2 - 2) \phi_{,xx'} + 2H_{3,x'y'}$

$\sigma_{yy}/\mu = (M_2^2 - 2) \phi_{,xx'} - 2H_{3,x'y'}$

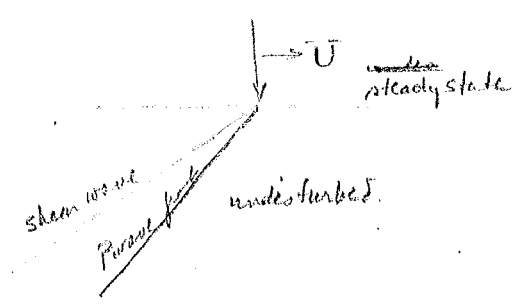
$\sigma_{xy}/\mu = 2\phi_{,xy'} + (M_2^2 - 2)H_{3,x'x'}$

apply BC $\sigma_{yy} (M_2^2 - 2) \frac{\partial^2 \phi}{\partial x'^2} - 2 \frac{\partial^2 H_3}{\partial x' \partial y'} = -\frac{P}{\mu} \delta(x')$

$\sigma_{xy} 2\phi_{,x'y'} + (M_2^2 - 2)H_{3,x'x'} = 0$

can integrate w.r.t x'

for supersonic case $\phi = f(x' + \bar{\beta}_1 y')$
 $H_3 = g(x' + \bar{\beta}_2 y')$



Example 12

Suppose $\sigma_{xy}|_{y=0} = P e^{i(\omega t + kx)}$

can be used later for $f(x)$ written as a fn of
fourier integrals

write DE for ϕ , A_3 w/ BC $\sigma_{xy}=0$ $\sigma_{yy} = P e^{i(\omega t + kx)}$

let $\phi = A e^{-\alpha_1 y} e^{i(\omega t + kx)}$

$A_3 = B e^{-\beta_1 y} e^{i(\omega t + kx)} \Rightarrow$ from $-2A i \alpha_1 k + (\beta_1^2 + k^2) B = 0$

$$A [\lambda(-k^2) + (\lambda + 2\mu)\alpha_1^2] + B 2\mu i k \beta_1 = P$$

$$w/ \alpha_1^2 = k^2 - \frac{\omega^2}{c_1^2} \quad \beta_1^2 = k^2 - \frac{\omega^2}{c_2^2}$$

solve for A, B together

$$A = \frac{2k^2 - \omega^2/c_2^2}{R(k)} \frac{P}{\mu}$$

$$B = \frac{2ik\beta_1}{R(k)} \frac{P}{\mu}$$

$$R(k) = (k^2 + \beta_1^2)^2 - 4k^2 \alpha_1 \beta_1$$

Rayleigh fn.

if $R(k) = 0$ we have Rayleigh's surface waves or resonance; $\therefore$ problem
blows up & no longer steady state - must reformulate

$$Eg \ 2k^2 = \frac{\omega^2}{c_2^2}$$

$$A = 0 \Rightarrow (\phi = 0)$$

stress will depend only on μ .

DIVISION OF APPLIED MECHANICS
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ME 236A Waves and Vibrations

Autumn 1979

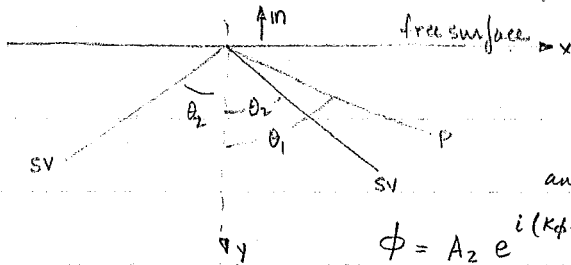
Problem Set No. 1

1. Study the reflection of SV-waves from a traction-free plane of an elastic half-space.
2. Sketch the instantaneous particle velocity and the particle path of Rayleigh surface waves on the surface.
3. For Rayleigh waves, sketch the dependence of amplitudes of displacements on depth. Normalize these amplitudes with respect to the normal displacement amplitude on the surface. Normalize depth coordinate with respect to wave length. ie draw $\bar{u}_x = f(\bar{y})$ where $\bar{u}_x = \frac{u_x(x, y)}{u_x(x, 0)}$ $\bar{y} = y/\lambda$
4. For Rayleigh waves, sketch the dependence of the three non-vanishing stress amplitudes on depth. Normalize these amplitudes with respect to the non-vanishing normal stress component on the surface. Normalize depth as in the previous problem.

$$\begin{aligned} \text{if } \sigma_{yy} &= A(y) e^{ik(x-ct)} & \text{draw } \frac{A(y)}{A(0)}, \text{ normalize } \\ \sigma_{xy} &= B(y) e^{ik(x-ct)} & \bar{y} = y/\lambda \\ \sigma_{yx} &= C(y) e^{ik(x-ct)} & \frac{B(y)}{B(0)} \\ & & \frac{C(y)}{C(0)} \end{aligned}$$

Problem Set #1

1. Study the reflection of S-V waves from a traction-free plane of an elastic half-space.



Since an S-V can be reflected off the free boundary as a P

and an S-V wave take.

$$\phi = A_2 e^{i(k_\phi x + \alpha y - \omega t)}; H_z = B_1 e^{i(k_H x - \beta y - \omega t)} + B_2 e^{i(k_H x + \beta y - \omega t)} = H_{z1} + H_{z2}$$

where we can define: $k_\phi = \gamma_1 \sin \theta_1$; $k_H = \gamma_2 \sin \theta_2$; $\alpha = \gamma_1 \cos \theta_1 = k_\phi (\frac{c_1^2}{c_2^2} - 1)^{1/2}$; $\beta = \gamma_2 \cos \theta_2 = k_H (\frac{c_2^2}{c_1^2} - 1)^{1/2}$

Now $n = -e_y \therefore t_n = \sigma \cdot n \equiv 0 = e_x \sigma_{xy} + e_y \sigma_{yy} + e_z \sigma_{yz} \Rightarrow \sigma_{xy} = \sigma_{yy} = \sigma_{yz} = 0$ at the bdy

$$u_x = \frac{\partial \phi}{\partial x} + \frac{\partial H_z}{\partial y}; \epsilon_{xx} = \frac{\partial^2 \phi}{\partial x^2} + \frac{\partial^2 H_z}{\partial x \partial y}; u_y = \frac{\partial \phi}{\partial y} - \frac{\partial H_z}{\partial x}; \epsilon_{yy} = \frac{\partial^2 \phi}{\partial y^2} - \frac{\partial^2 H_z}{\partial x \partial y}; u_z = 0 \Rightarrow \frac{\partial u_z}{\partial x} = \frac{\partial u_z}{\partial y} = \frac{\partial u_z}{\partial z} = 0$$

$$\epsilon_{xy} = \frac{1}{2} \left[\frac{\partial u_x}{\partial y} + \frac{\partial u_y}{\partial x} \right] = \frac{\partial^2 \phi}{\partial x \partial y} + \frac{1}{2} \left(\frac{\partial^2 H_z}{\partial y^2} - \frac{\partial^2 H_z}{\partial x^2} \right); \epsilon_{yz} = \frac{1}{2} \left[\frac{\partial u_y}{\partial z} + \frac{\partial u_z}{\partial y} \right] = 0 \Rightarrow \sigma_{yz} \equiv 0;$$

$$\sigma_{xy} = 2\mu \epsilon_{xy} = \mu \left[\frac{\partial^2 \phi}{\partial x \partial y} + \left(\frac{\partial^2 H_z}{\partial y^2} - \frac{\partial^2 H_z}{\partial x^2} \right) \right]; \sigma_{yy} = \lambda [\epsilon_{xx} + \epsilon_{yy}] + 2\mu \epsilon_{yy} = (\lambda + 2\mu) \nabla^2 \phi - 2\mu \left(\frac{\partial^2 \phi}{\partial x^2} + \frac{\partial^2 H_z}{\partial x \partial y} \right)$$

at the bdy: $\sigma_{yz} \equiv 0$ and $\begin{cases} \frac{\partial \phi}{\partial x} = i k_\phi \phi, \frac{\partial^2 \phi}{\partial x^2} = -k_\phi^2 \phi, \frac{\partial^2 \phi}{\partial x \partial y} = -k_\phi \alpha \phi, \frac{\partial \phi}{\partial y} = i \alpha \phi, \frac{\partial^2 \phi}{\partial y^2} = -\alpha^2 \phi \\ \frac{\partial H_z}{\partial x} = i k_H H_z, \frac{\partial H_z}{\partial x^2} = -k_H^2 H_z, \frac{\partial H_z}{\partial x \partial y} = k_H \beta (H_{z1} - H_{z2}), \frac{\partial^2 H_z}{\partial y^2} = -\beta^2 H_z \end{cases}$

$$\sigma_{xy}|_{y=0} = \mu [-2k_\phi \alpha \phi - \beta^2 H_z + k_H^2 H_z] = 0 \Rightarrow \mu [-2k_\phi \alpha A_2 - (\frac{c_1^2}{c_2^2} - 2)(B_1 + B_2)] e^{i(k_H x - \omega t)} = 0$$

where we assume that $k_H = k_\phi$ and $\omega_H = \omega_\phi$

or $\sigma_{xy}|_{y=0} = 0 \Rightarrow -2r_\alpha A_2 - (r_\beta^2 - 1)(B_1 + B_2) = 0$ where $\alpha = k_\phi r_\alpha$ $\beta = k_H r_\beta$

$$\text{or } -2r_\alpha A_2 + (1 - r_\beta^2)(B_1 + B_2) = 0$$

$$\sigma_{yy}|_{y=0} = [(\lambda + 2\mu)(-k_\phi^2)(r_\alpha^2 + 1)A_2 - 2\mu(-k_\phi^2 A_2 + k_H^2 r_\beta(B_1 - B_2))] e^{i(k_H x - \omega t)} = 0$$

$$\text{or } \sigma_{yy}|_{y=0} = (\lambda + 2\mu)(r_\alpha^2 + 1)A_2 - 2\mu(A_2 - r_\beta(B_1 - B_2)) = 0$$

$$\text{or } [\lambda(r_\alpha^2 + 1) + 2\mu r_\alpha^2]A_2 + 2\mu r_\beta(B_1 - B_2) = 0$$

then $\left| \frac{A_2}{B_1} = \frac{4\mu r_\beta(1 - r_\beta^2)}{4\mu r_\alpha r_\beta - [(\lambda + 2\mu)(r_\alpha^2 + 1) - 2\mu](1 - r_\beta^2)} \right|$ and $\left| \frac{B_2}{B_1} = \frac{4\mu r_\alpha r_\beta + [(\lambda + 2\mu)(r_\alpha^2 + 1) - 2\mu](1 - r_\beta^2)}{4\mu r_\alpha r_\beta - [(\lambda + 2\mu)(r_\alpha^2 + 1) - 2\mu](1 - r_\beta^2)} \right|$

when $\theta_2 = \theta_{2c}$ (critical angle) $\theta_1 = 90^\circ \Rightarrow r_\alpha = 0$ $\frac{A_2}{B_1} = -\frac{4\mu}{\lambda} r_\beta$ $\frac{B_2}{B_1} = -1$

for $\theta_2 > \theta_{2c}$ we must look for solutions of P waves which decay with y $\therefore$ let $i\alpha < 0$

if $r_\alpha^* = (1 - \frac{c_1^2}{c_2^2})^{1/2}$ then $r_\alpha = i r_\alpha^*$ and $i\alpha = i k_\phi \cdot i r_\alpha^* < 0$

Thus $B_2/B_1 = \frac{4\mu i r_\alpha^* r_\beta + [(\lambda+2\mu)(1-r_\alpha^{*2})+2\mu](1-r_\beta^2)}{4\mu i r_\alpha^* r_\beta - [(\lambda+2\mu)(1-r_\alpha^{*2})+2\mu](1-r_\beta^2)} = \frac{\zeta e^{i\varphi}}{-\zeta e^{-i\varphi}} = -e^{2i\varphi}$

where $\zeta = \left\{ (4\mu r_\alpha^* r_\beta)^2 + (1-r_\beta^2)^2 [(\lambda+2\mu)(1-r_\alpha^2)+2\mu]^2 \right\}^{1/2}$ and $\varphi = \tan^{-1} \frac{4\mu r_\alpha^* r_\beta}{[(\lambda+2\mu)(1-r_\alpha^{*2})+2\mu](1-r_\beta^2)}$

thus there is a phase shift in the amplitude of the reflected S-V wave

and $A_2/B_1 = \frac{4\mu r_\beta(1-r_\beta^2)}{4\mu i r_\alpha^* r_\beta - [(\lambda+2\mu)(1-r_\alpha^{*2})+2\mu](1-r_\beta^2)}$

These results can be converted to those found in Achenbach if we note that $r_\beta = \frac{\delta_2 \cos \theta_2}{k_H} = \cot \theta_2$

and similarly $r_\alpha = \cot \theta_1$. Hence $r_\alpha^2 + 1 = 1/\sin^2 \theta_1 = (c/c_1)^2$ similarly $1-r_\beta^2 = -\cos 2\theta_2 / \sin^2 \theta_2$

and as previously shown in class $\sin \theta_2 = \kappa \sin \theta_1$ where $\kappa = c_1/c_2$. If so then

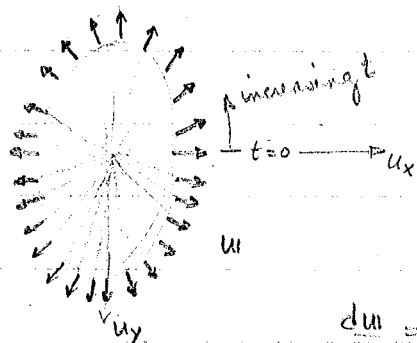
$A_2/B_1 = -\frac{\kappa \sin 4\theta_1}{\sin 2\theta_1 \sin 2\theta_2 + \kappa^2 \cos^2 2\theta_1}$ & $B_2/B_1 = \frac{\sin 2\theta_1 \sin 2\theta_2 - \kappa^2 \cos^2 2\theta_1}{\sin 2\theta_1 \sin 2\theta_2 + \kappa^2 \cos^2 2\theta_1}$ for $\cot \theta_1$ real

** see last page for discussion.

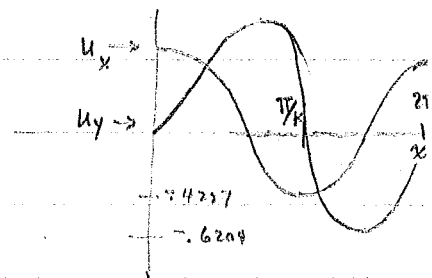
2. for Rayleigh waves on the surface with $\nu = 1/4$ $c_R/c_2 = .9194$ we find that

$u_x = A'(.4227) \cos k(x-ct)$ and $u_y = A'(1.6204) \sin k(x-ct)$

$\therefore \left(\frac{u_x}{A'(.4227)} \right)^2 + \left(\frac{u_y}{A'(1.6204)} \right)^2 = 1 \Rightarrow$ a particle at the surface travels an elliptical path. Thus $u = u_x \mathbf{e}_x + u_y \mathbf{e}_y$. Note that each of the displacement components is sinusoidal. We will look at $x=0$ (for simplicity) as $t > 0$



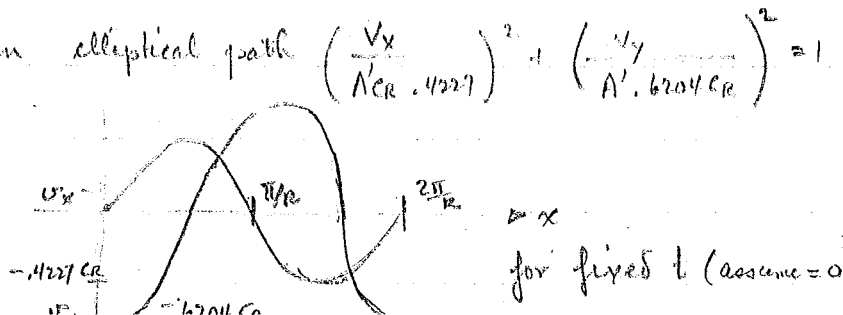
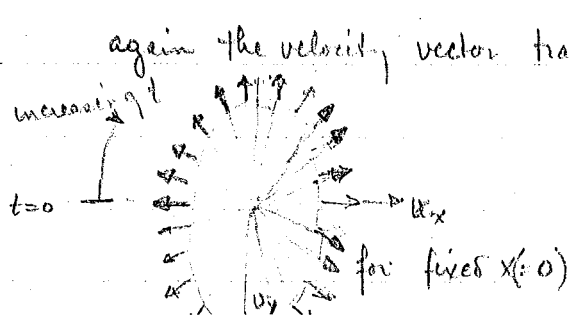
for fixed t and increasing x



$\frac{du}{dt} = +A'c_R(.4227) \sin k(x-ct) \mathbf{e}_x - c_R A'(1.6204) \cos k(x-ct) \mathbf{e}_y$

v_x

v_y



*** see last page for composite

3. We note that as the depth increases we must investigate (for $\nu = \frac{1}{4}$)

$$u_x = A' (e^{-.8475ky} - .5773 e^{-.3933ky}) \cos k(x - c_R t)$$

$$u_y = A' (-.8475 e^{-.8475ky} + 1.4679 e^{-.3933ky}) \sin k(x - c_R t)$$

Normalizing with the surface amplitudes $u_x/u_y^0 = [e^{-.8475ky} - .5773 e^{-.3933ky}] / .6204$

$$u_y/u_y^0 = [-.8475 e^{-.8475ky} + 1.4679 e^{-.3933ky}] / .6204 ; \quad \text{Now } \lambda k = 2\pi \text{ ft. } ky = k\lambda \cdot y/\lambda = 2\pi \bar{y}$$

$$\Rightarrow u_x/u_y^0 = [e^{-5.325\bar{y}} - .5773 e^{-2.4712\bar{y}}] / .6204 \text{ and } u_y/u_y^0 = [-.8475 e^{-5.325\bar{y}} + 1.4679 e^{-2.4712\bar{y}}] / .6204$$

Plotted on a separate sheet are the amplitudes of displacement as a function of normalized depth. We note that as $\bar{y} > .192$ then the direction of motion of the ellipse reverses. These plots are for $\nu = 1/4$. If we want other plots for different values of ν , we need to return to the general problem and, for a given ν , ¹⁾ obtain c_1 & c_2 ; ²⁾ Put into eqn for $(c/c_1, c/c_2)$ to find solutions; then ³⁾ recompute A_1 and B_1 to get u_x and u_y . ⁴⁾ Normalize results and then plot.

4. For $\nu = 1/4$ $u_x = A' (e^{-.8475ky} - .5773 e^{-.3933ky}) \cos k(x - c_R t)$ $c_R = .9194/c_2$

$$u_y = A' (-.8475 e^{-.8475ky} + 1.4679 e^{-.3933ky}) \sin k(x - c_R t)$$

Now $\sigma_{yy} = (\lambda + 2\mu) \left(\frac{\partial u_x}{\partial x} + \frac{\partial u_y}{\partial y} \right) - 2\mu \frac{\partial u_x}{\partial x} = [3\rho c_2^2 \{ -.2817 e^{-5.325\bar{y}} - .0008 e^{-2.4712\bar{y}} \} + 2\rho c_2^2 \{ e^{-5.325\bar{y}} - .5773 e^{-2.4712\bar{y}} \} A' k \cdot \sin k(x - c_R t)]$

$$\tau_{yx} = \mu \left(\frac{\partial u_x}{\partial y} + \frac{\partial u_y}{\partial x} \right) = \frac{c_2^2 A' k}{4} [-1.695 e^{-5.325\bar{y}} + 1.695 e^{-2.4712\bar{y}}] \cos k(x - c_R t)$$

$$\sigma_{xx} = (\lambda + 2\mu) \left(\frac{\partial u_x}{\partial x} + \frac{\partial u_y}{\partial y} \right) - 2\mu \frac{\partial u_y}{\partial y} = [3\rho c_2^2 \{ -.2817 e^{-5.325\bar{y}} - .0008 e^{-2.4712\bar{y}} \} - 2\rho c_2^2 \{ .7183 e^{-5.325\bar{y}} - .5781 e^{-2.4712\bar{y}} \} A' k \sin k(x - c_R t)]$$

@ the boundary $|\sigma_{yx}| = 0$ $|\sigma_{yy}| = 0$ $|\sigma_{xx}| = [3(-.2825) - 2\{.1402\}] A' \rho c_2^2 k = -1.1279 A' \rho c_2^2 k$

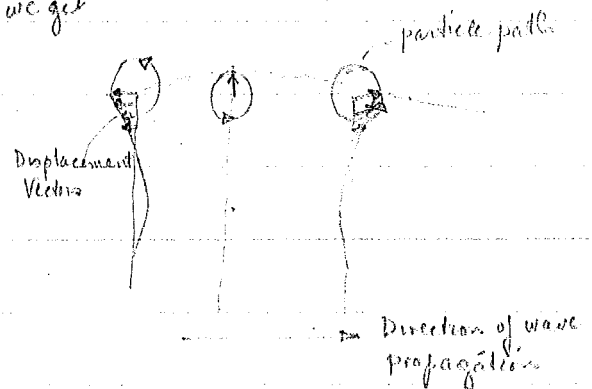
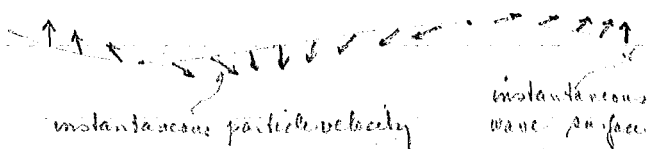
Thus $\frac{\sigma_{yy}}{\sigma_{xx}|_{y=0}} = 1.157 (e^{-5.325\bar{y}} - e^{-2.4712\bar{y}}) / -1.1279$; $\frac{\sigma_{yx}}{\sigma_{xx}|_{y=0}} = \frac{1.695}{1.1279} (e^{-5.325\bar{y}} - e^{-2.4712\bar{y}})$

$$\frac{\sigma_{xx}}{\sigma_{xx}|_{y=0}} = (-2.2817 e^{-5.325\bar{y}} + 1.1538 e^{-2.4712\bar{y}}) / -1.1279$$

The graphs are plotted on a separate sheet

Discussion of Problem #1 : We note that from $A_2/B_1 = 0 \Rightarrow \theta_1 = \pi/4, \pi/2$, and 0; i.e. the P wave vanishes. Thus for these values SV waves are reflected as SV waves. If $B_2/B_1 = 0$, meaning that if the numerator is 0, then SV waves are totally reflected as P waves. For vanishing P waves $\theta_1 = 0$ means a vertically incident SV wave, $\theta_1 = \pi/2$ means a horizontally grazing incident SV wave. We must restrict the admissible values because we must consider $\sin \theta_2 = \kappa \sin \theta_1$ and since $\kappa > 1$ then $\exists! \theta_{1c} \Rightarrow \kappa \sin \theta_{1c} = 1$ i.e. $\theta_2 = \pi/2$. Thus only those values of $\theta_1 < \theta_{1c}$ will be admissible values $\Rightarrow$ incident SV waves are reflected totally as SV waves. Similarly with the reflection of SV waves as P waves, those values of θ_1 that cause the numerator of $B_2/B_1 = 0$ must be less than θ_{1c} . From the figure 5.7 we note that for $\nu < .26$ there is a possibility of SV conversion to P waves but above $\nu = .26$ no such conversion is possible.

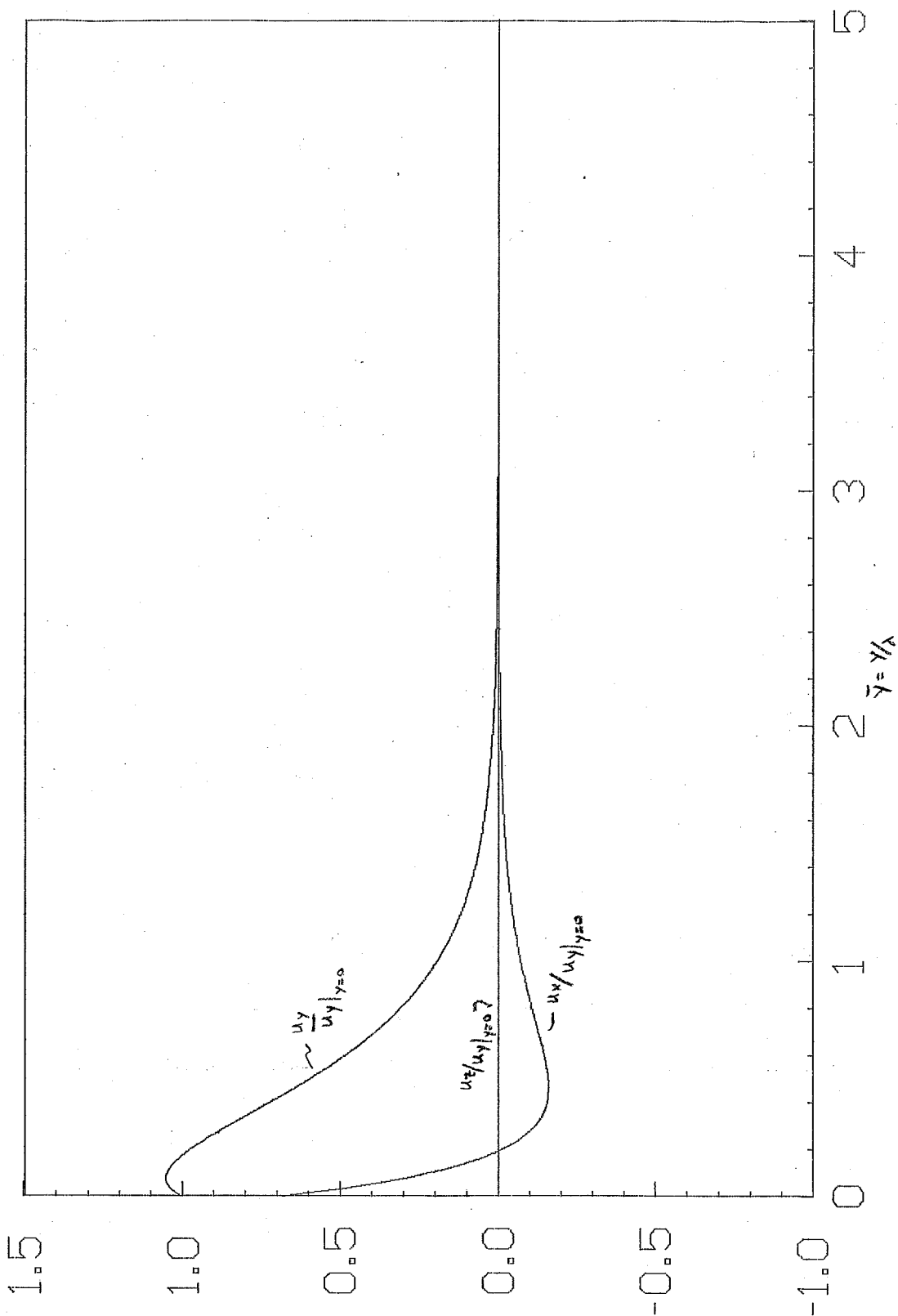
Discussion of Problem #2 : Combining both pictures we get



Y. C. Fung : Foundations of Solid Mech.

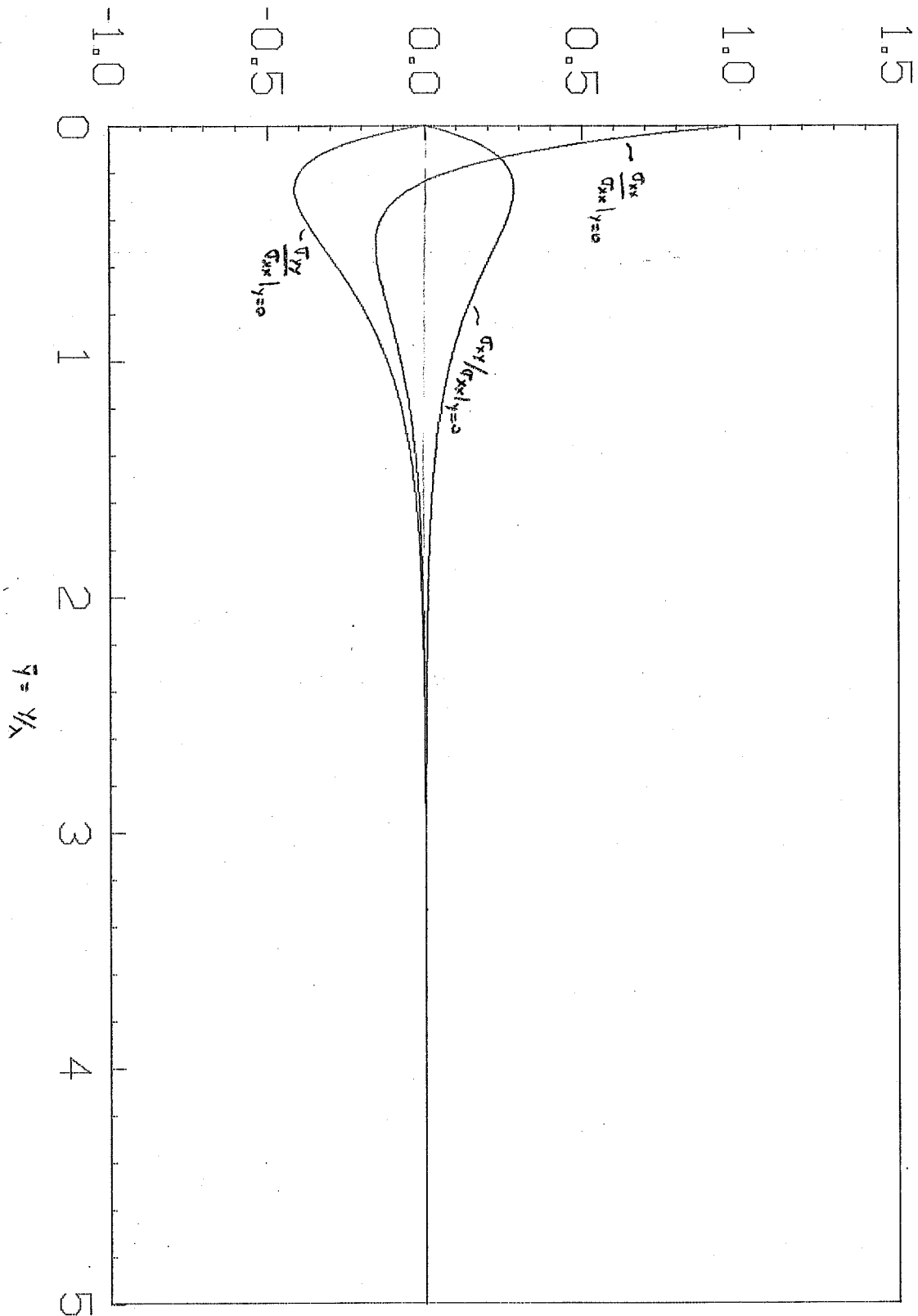
Fig 7.9:1 Pg 181

Problem #3 ($\nu = 1/4$)
 Normalized amplitudes vs. normalized depth
 zero over point $\bar{y} \approx .192$

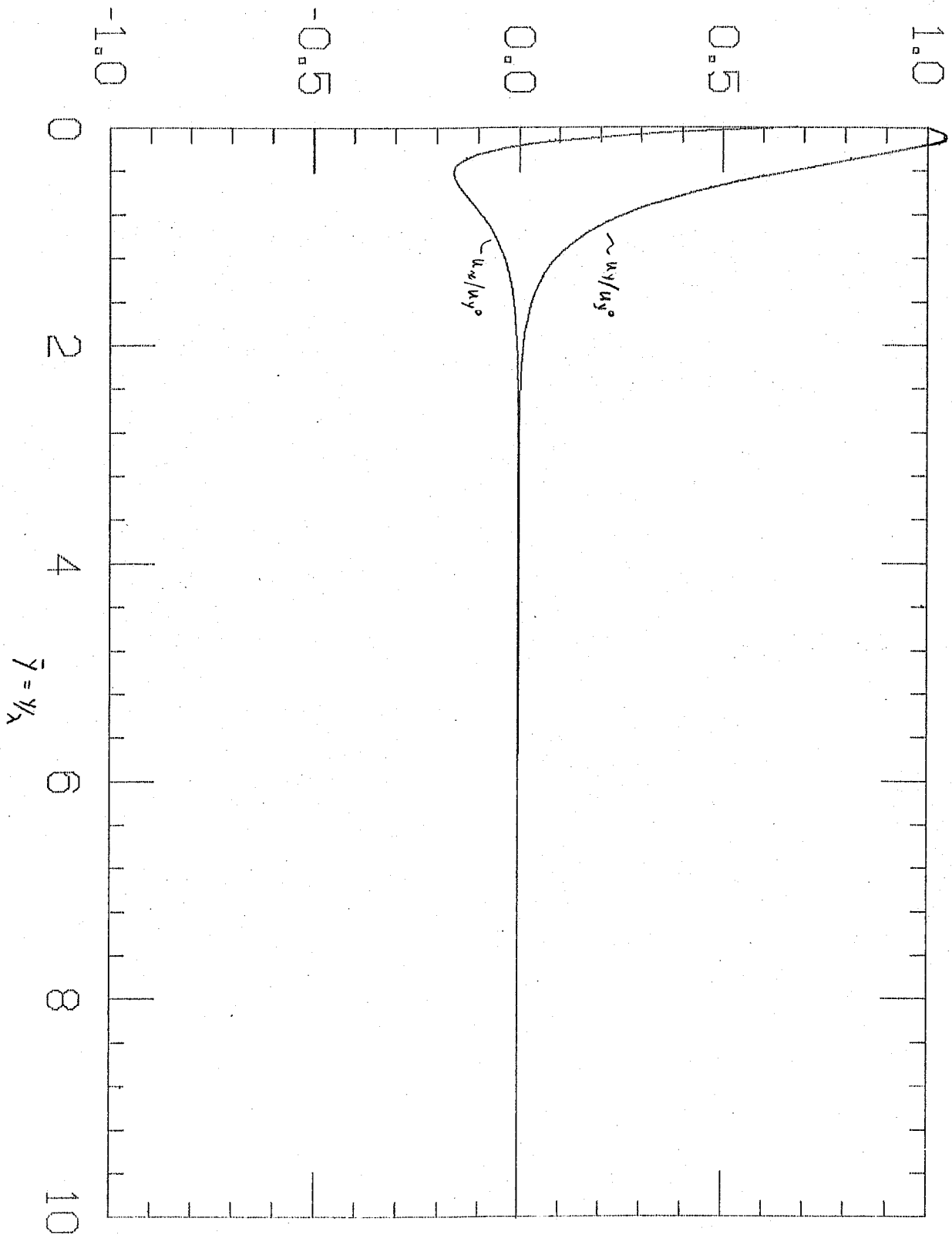


amplitude ratio = $\frac{\text{amplitude of } u_y}{\text{amplitude of } u_x|_{y=0}} = 0.6204$

$$\text{amplitude ratio} = \frac{\text{ampl. of stress}}{\text{ampl. of } \sigma_{11}|_{y=0}} = -1.1279$$



Problem #4 ($\nu = 1/4$)
Normalized stresses vs. normalized depth



Problem #3 ($\nu = 1/4$)
 Normalized Amplitude versus normalized depth
 cross over point is at $\bar{y} = .192$

#3

COLLECT

1. //SETCL JOB
2. /*JOBPARM DEST=SELF
3. //EXEC^b PORTCG
4. //FORT.SYSIN^b DD^b *

DIMENSION S11(100), S12(100), S13(100), Y(100)

DO 10 I = 1, 100

YB_I = .1*(I-1)

Y(I) = YB

EX1 = EXP(-5.325*YB)

EX2 = EXP(-2.4712*YB)

S22(I) = 1.157*(EX1 - EX2)/(-1.1279)

$(1 * EX1 - .5773 * EX2) / .6204$

S12(I) = 1.695*(EX1 - EX2)/(1.1279)

$(-.8475 * EX1 + 1.4679 * EX2) / .6204$

S11(I) = (-2.2817*EX1 + 1.1538*EX2)/(-1.1279)

0.

10 CONTINUE

DO 20 I = 1, 101

WRITE (3,25) S11(I), Y(I)

25 FORMAT (2(2X,F20,10))

20 CONTINUE

WRITE (3,26)

26 FORMAT ('1000000 JOIN')

DO 40 I = 1, 101

WRITE (3,25) S22(I), Y(I)

40 CONTINUE

WRITE (3,26)

DO 60 I = 1, 101

WRITE (3,25) S12(I), Y(I)

60 CONTINUE

WRITE (3,26)

STOP

END

// GO.FTO3FOOI_DD_DSN=WYL.NJ.MBN.PLOTDAT, DISP=(NEW,KEEP),

// VOL=SER=PUB004, UNIT=DISK, DCB=(LRECL=80, RECFM=FB, BLKSIZE=6160),

// SPACE=(TRK,(5,5),RLSE)

SAVE PROG ON PUB004

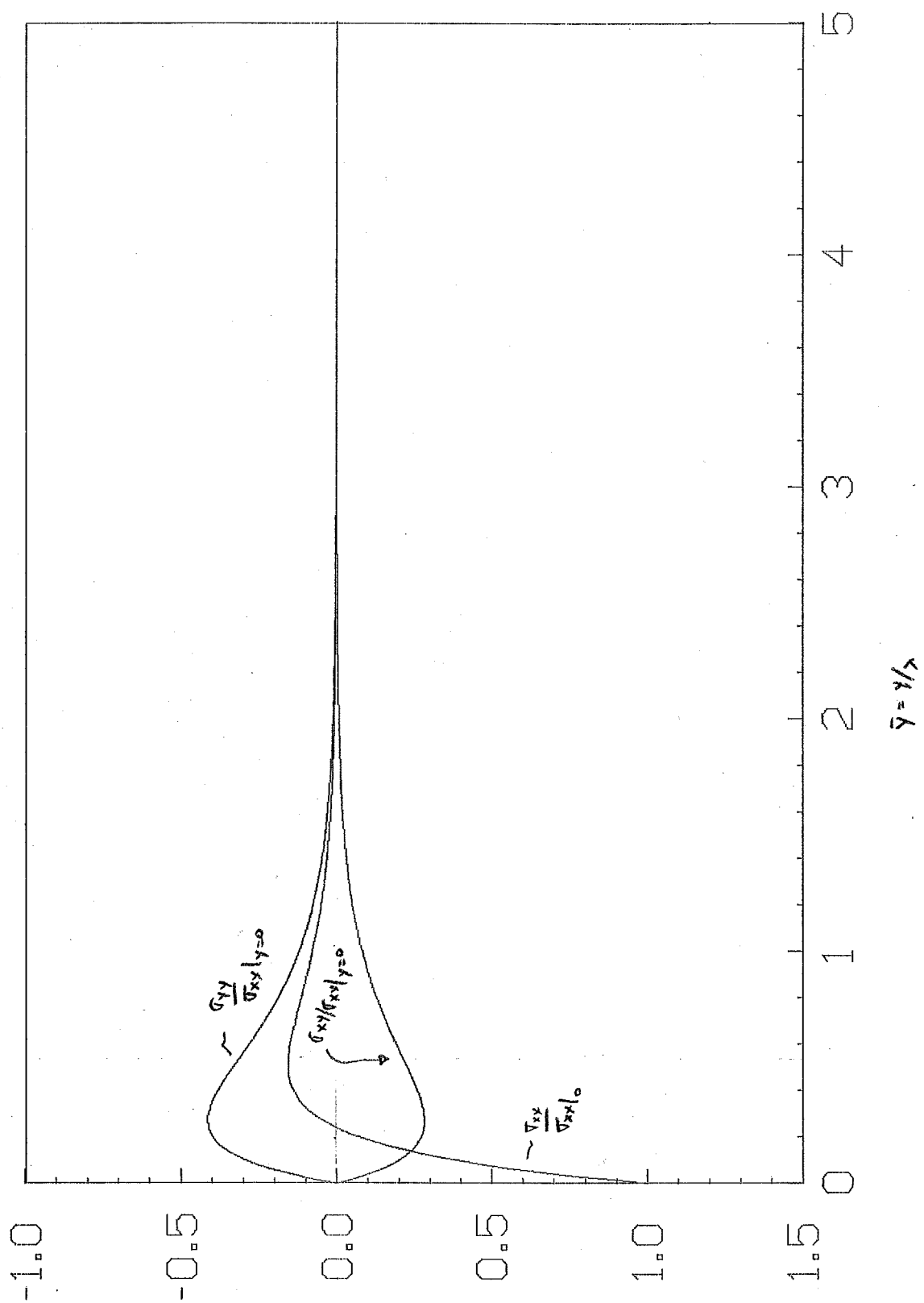
RUN HOLD

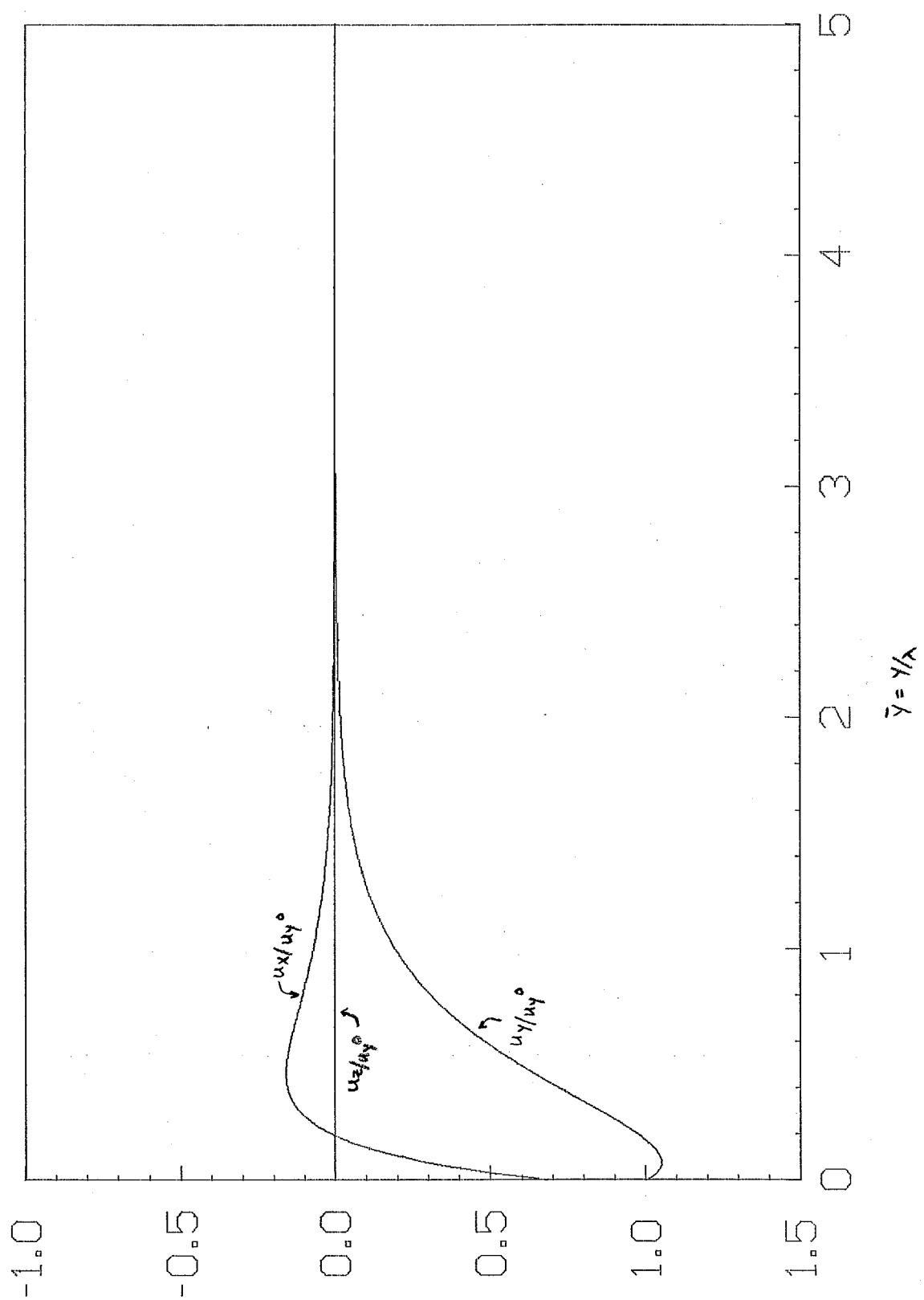
Collect

1. SET SIZE 10.5 BY 7.88
 2. SET WINDOW Y 2 TO 7 X 2 TO 9
 3. SET AXES; SET LIMITS Y 1.5 TO -1.0 X 0 TO 10
 4. SET TICKS SIZE .03 RIGHT OFF TOP OFF; PLOT AXES
- SAVE PLOT ON PUB004.

COPY FROM PLOTDAT ON PUB004 TO 5 BY 1

EXEC FROM #TOPDRAW PUB ON CAT.





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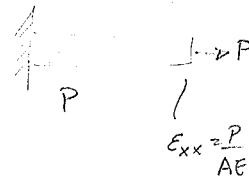
ME 236A Waves and Vibrations

Autumn 1979

Problem Set No. 2 Thursday 29th

1. A prismatic bar is fixed at one end and subjected at the other end to an axial force P . The force is suddenly removed at time $t = 0$. Show that the ensuing displacement can be put into the form

$$u(x,t) = \frac{8PL}{AE\pi^2} \sum_{n=1,3,5}^{\infty} \frac{(-1)^{\frac{n-1}{2}}}{n^2} \sin \frac{n\pi x}{2L} \cos \frac{n\pi ct}{2L} ; \quad c^2 = \frac{E}{\rho}$$



2. A bar is traveling to the left with constant velocity v . At time $t = 0$ its left end strikes a rigid stop situated at $x = 0$. Under the assumption that the left end remains fixed for $t > 0$ and the right end is free, determine the resulting motion and the maximum strain.
3. Consider a prismatic bar fixed at one end and free at the other. It is subjected to a periodic axial force $P_0 \sin \omega_f t$ at a distance d from the fixed end. Determine the displacement field in the bar.

$\rightarrow P = P_0 \sin \phi t$

$$U_T = c\lambda \cos \lambda(0) + D\lambda \sin \lambda L = 0 \Rightarrow \frac{c\lambda}{D} U$$

$$T(0) = \frac{P}{AE} U(y) = \frac{P}{AE}$$

$$\frac{\partial u}{\partial t}(x,0) = +\lambda \tilde{c} U(x) = 0 \Rightarrow \tilde{c}$$

$$\frac{P}{A} = \sigma \quad \frac{P}{AE} = \frac{\Delta u}{\Delta x} \quad \frac{P}{AE} = u$$

$\tilde{D}$

$U(x)$

$$\left(1 - \cos \frac{n\pi x}{L}\right)$$

$n = 1, 2, 3, \dots$

$$u_{xx} = \left(\frac{n\pi}{2L}\right)^2$$

$$u_{xx} - c^2 u_{tt} = 0$$

$$\left\{ \begin{array}{l} x \sin \end{array} \right.$$

$$u(0,t) = 0$$

$$\frac{\partial u}{\partial t}(x,0) = 0$$

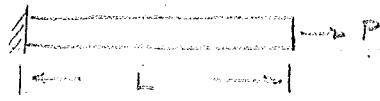
$$\frac{\partial u}{\partial x} \{$$

$$\cos \frac{n\pi x}{2L}$$

$$u_{tt} = -\left(\frac{n\pi c}{2L}\right)^2$$

Problem Set #2

1. A prismatic bar is fixed at one end and subjected to an axial force P for $t < 0$. If at time $t = 0$ the force is removed find $u(x, t)$.



The governing PDE is $u_{xx} - \frac{1}{c^2} u_{tt} = 0$ $c^2 = E/\rho$

The I.C. are that $u(x, 0) = \frac{Px}{AE}$ and $\frac{\partial u}{\partial t}(x, 0) = 0$

The B.C. are that $u(0, t) = 0$ and $\frac{\partial u}{\partial x}(L, t) = 0$, which correspond to fixed end & no impressed stress

Let $u(x, t) = U(x)T(t)$ then by placing into PDE, we can get the form.

$$c^2 \frac{U''}{U} = \frac{T''}{T} = -\lambda^2 \quad \text{where } \lambda^2 \text{ is a constant } \neq 0$$

then $U'' + (\lambda^2)U = 0$ thus $U(x) = A \sin \frac{\lambda}{c} x + B \cos \frac{\lambda}{c} x$

also $T'' + \lambda^2 T = 0$ thus $T(t) = \tilde{C} \sin \lambda t + \tilde{D} \cos \lambda t$

Thus the I.C. become $U(x)T(0) = U(x)\tilde{D} = \frac{Px}{AE}$ and $U(x)\lambda \tilde{C} = 0$; if $\lambda \neq 0 \Rightarrow \tilde{C} = 0$

The B.C. become $U(0)T(t) = 0 = BT(t) = 0 \Rightarrow B = 0 \quad \forall t$, and $A \frac{\lambda}{c} \cos \frac{\lambda L}{c} T(t) = 0 \quad \forall t$

thus if $A, \lambda \neq 0$ $\frac{\lambda L}{c} = \frac{n\pi}{2}$ for $n = 1, 3, 5, \dots$ or $\lambda_n = \frac{n\pi c}{2L}$

Hence $u(x, t) = \sum_{n=1,3,5,\dots} u_n(x, t) = \sum_{n=1,3,5,\dots} A_n \sin \frac{n\pi x}{2L} \cos \frac{n\pi c t}{2L}$ $u_n(x, t) = U_n(x)T_n(t)$

now $u(x, 0) = \frac{Px}{AE} = \sum_{n=1,3,5,\dots} A_n \sin \frac{n\pi x}{2L}$ then $A_n = \frac{2}{L} \int_0^L \frac{Px}{AE} \sin \frac{n\pi x}{2L} dx \quad n = 1, 3, 5, \dots$

$$A_n = \frac{2P}{LAE} \int_0^L x \sin \frac{n\pi x}{2L} dx; \quad \int_0^L x \sin \frac{n\pi x}{2L} dx = \frac{x \cos \frac{n\pi x}{2L}}{\frac{n\pi}{2L}} \Big|_0^L - \int_0^L \cos \frac{n\pi x}{2L} dx$$

$$= \frac{4L^2}{n^2\pi^2} \sin \frac{n\pi x}{2L} \Big|_0^L = \frac{4L^2}{n^2\pi^2} \sin \frac{n\pi}{2} = \frac{4L^2}{n^2\pi^2} (-1)^{\frac{n-1}{2}}$$

thus $A_n = \frac{2P}{LAE} \cdot \frac{4L^2}{n^2\pi^2} (-1)^{\frac{n-1}{2}} = \frac{8PL}{AE\pi^2} \cdot \frac{(-1)^{\frac{n-1}{2}}}{n^2}$

and hence $u(x, t) = \frac{8PL}{AE\pi^2} \sum_{n=1,3,5,\dots} \frac{(-1)^{\frac{n-1}{2}}}{n^2} \sin \frac{n\pi x}{2L} \cos \frac{n\pi c t}{2L}$ as required ✓

Note: If one investigates $\lambda^2 = 0$: $T_0(t) = \bar{A}t + \bar{B}$ and $U_0(x) = \bar{C}x + \bar{D}$. Applying B.C. $\Rightarrow \bar{C}, \bar{D} = 0$ & $U_0(x) = 0 \Rightarrow u_0(x, t) = 0$

Problem 2: A bar traveling to the left with constant velocity v . At $t = 0$ its left end strikes a rigid stop at $x = 0$ and remains fixed at all time $t > 0$ and its right end is vibrates freely. Determine $u(x, t)$ and $\left| \frac{\partial u}{\partial x} \right|_{\max}$

the governing PDE is $u_{xx} - \frac{1}{c^2} u_{tt} = 0$

with IC $u(x, 0) = 0$ and $\frac{\partial u}{\partial t}(x, 0) = -V$

and BC $u(0, t) = 0$ and $\frac{\partial u}{\partial x}(L, t) = 0$

Using separation of variables $u(x, t) = U(x)T(t)$ then

$$c^2 \frac{U''}{U} = \frac{T''}{T} = -\lambda^2$$

and for $\lambda \neq 0$

$$U(x) = A \cos \frac{\lambda}{c} x + B \sin \frac{\lambda}{c} x$$

$$T(t) = C \cos \lambda t + D \sin \lambda t$$

From BC $u(0, t) = 0 \Rightarrow A = 0$; $\frac{\partial u}{\partial x}(L, t) = 0 \Rightarrow \frac{\lambda L}{c} = \frac{n\pi}{2}$ u is odd or $\lambda_n = \frac{n\pi c}{2L}$ $\forall t$

From IC $u(x, 0) = 0 \Rightarrow C = 0$; $\frac{\partial u}{\partial t}(x, 0) = \sum_{n=1,3,5,\dots} \tilde{B}_n(x) D_n \lambda_n = -V$

$$\text{Thus } u(x, t) = \sum_{n=1,3,5,\dots} \tilde{B}_n \sin \frac{n\pi x}{2L} \sin \frac{n\pi ct}{2L} \quad \text{where } \tilde{B}_n = B_n D_n$$

$$\therefore \frac{\partial u}{\partial t}(x, 0) = -V = \sum_{n=1,3,5,\dots} \tilde{B}_n \lambda_n \sin \frac{n\pi x}{2L}$$

$$\text{Thus } \tilde{B}_n = \frac{-2V}{L \lambda_n} \int_0^L \sin \frac{n\pi x}{2L} dx = \frac{+2V}{L \lambda_n} \left[\cos \frac{n\pi x}{2L} \right]_0^L = \frac{+4V}{\lambda_n \pi} (-1) = \frac{-4V \cdot 2L}{n^2 \pi^2 c} = \frac{-8VL}{n^2 \pi^2 c} \quad \text{for } n=1,3,5,\dots$$

$$\therefore u(x, t) = \frac{-8VL}{\pi^2 c} \sum_{n=1,3,5,\dots} \frac{1}{n^2} \sin \frac{n\pi x}{2L} \sin \frac{n\pi ct}{2L}$$

$$\frac{\partial u}{\partial x}(x, t) = -\frac{4V}{\pi c} \sum_{n=1,3,5,\dots} \frac{1}{n} \cos \frac{n\pi x}{2L} \sin \frac{n\pi ct}{2L} \quad \text{and} \quad \frac{\partial^2 u}{\partial x^2}(x, t) = +\frac{2V}{Lc} \sum_{n=1,3,5,\dots} \sin \frac{n\pi x}{2L} \sin \frac{n\pi ct}{2L}$$

$$\frac{\partial^2 u}{\partial x \partial t}(x, t) = -\frac{2V}{L} \sum_{n=1,3,5,\dots} \cos \frac{n\pi x}{2L} \cos \frac{n\pi ct}{2L}$$

for a max or min $\frac{\partial \epsilon_x}{\partial x} = \frac{\partial^2 u}{\partial x^2} = 0$ and $\frac{\partial \epsilon_x}{\partial t} = \frac{\partial^2 u}{\partial x \partial t} = 0$ at the same point in (x, t) space.

$$\frac{\partial^2 u}{\partial x^2} = 0 \Rightarrow \frac{n\pi x}{2L} = m\pi, 0 \Rightarrow x = 2L \text{ (not physical)} \text{ or } [x=0] \text{ for any } n \text{ and } \sin \frac{n\pi ct}{2L} = 0$$

$\forall x$; the only values of t that satisfy this equality for any n $t = \frac{2L}{c} k$, $k=0,1,2,3,\dots$

From $\frac{\partial^2 u}{\partial x \partial t}(x, t) = 0 \Rightarrow \frac{n\pi x}{2L} = \frac{n\pi}{2} \text{ (odd)}$ For any n the only value of x is $x=L$ $\forall t$; also $\cos \frac{n\pi ct}{2L} = 0$

$\forall x$; the only values of t that satisfy this are $t = \frac{L}{c}$ and odd mult. of $\frac{L}{c}$

Since there are no matching points in the interior of the bar, then the two end points must be examined for absolute max.

$$\text{At } x=0 \quad \epsilon_x(0, t) = -\frac{4V}{\pi c} \sum_{n=1,3,5,\dots} \frac{1}{n} \sin \frac{n\pi ct}{2L} \quad \text{min occurs at } t = \frac{L}{c}, \frac{5L}{c}, \frac{9L}{c}, \dots (4k+1)\frac{L}{c} \quad k=0,1,\dots$$

$$\text{At } x=L \quad \epsilon_x(L, t) = 0 \quad \forall t \quad \text{max occurs at } t = \frac{3L}{c}, \frac{7L}{c}, \frac{11L}{c}, \dots (4k+3)\frac{L}{c} \quad k=0,1,\dots$$

$$\sum_{n=1,3,5,\dots} \frac{1}{n} \sin \frac{n\pi}{2} = \frac{\pi}{4} \quad \text{for min} \quad \sum_{n=1,3,5,\dots} \frac{1}{n} \sin \frac{3n\pi}{2} = -\frac{\pi}{4} \quad \text{for max.}$$

$$\text{Thus for max } \epsilon_x(0, (4k+1)\frac{L}{c}) = \frac{V}{c} \quad \text{for min } \epsilon_x(0, (4k+3)\frac{L}{c}) = -\frac{V}{c}$$

3. Given a prismatic bar fixed at one end and free at the other end. It is subjected to a

periodic axial force $p_0 \sin \omega_f t$ at a distance d from the fixed end. Find $u(x,t)$

The governing PDE is $u_{xx} - \frac{1}{c^2} u_{tt} = 0$

and define $u = u(x,t)$

The BC are that $u(0,t) = 0$ and $\frac{\partial u}{\partial x}(l,t) = 0$

The IC are that $u(x,0) = 0$ and $\frac{\partial u}{\partial t}(x,0) = 0$ if not steady state

We attack the problem in the following manner: look at this as being a sum of 2 problems if problem is not one of steady state

Problem #1: $u = \hat{u}(x,t)$ with BC $\hat{u}(0,t) = 0$ $\frac{\partial \hat{u}}{\partial x}(l,t) = 0$; IC unsatisfied & $P = P_0 \sin \omega_f t$ at $x=d$

Problem #2: $u = \check{u}(x,t)$ with BC $\check{u}(0,t) = 0$ $\frac{\partial \check{u}}{\partial x}(l,t) = 0$; IC = -IC of problem #1 & no forcing

Thus we solve Problem #1 with unrestricted IC. We then get the IC, whatever they are. We then solve Problem #2 with improved IC = -IC of those found in problem 1.

$\therefore$ Soln = Σ Problem 1 + Problem 2 and we also satisfy $u(0,t) = \frac{\partial u}{\partial x}(l,t) = 0$

$\frac{\partial u}{\partial t}(x,0) = u(x,0) = 0$ and $P = P_0 \sin \omega_f t$ @ $x=d$.

Problem #1:

define $\hat{u} = u_1(x,t)$ $0 \leq x \leq d$

$\hat{u} = u_2(x,t)$ $d \leq x \leq l$

w/BC $u_1(0,t) = 0$ and $\frac{\partial u_2}{\partial x}(l,t) = 0$ and open initial conditions

the matching conditions are that $u_1(d,t) = u_2(d,t)$ and $EA \left[\frac{\partial u_2}{\partial x}(d,t) - \frac{\partial u_1}{\partial x}(d,t) \right] = -P_0 \sin \omega_f t$

Since the jump conditions are a fn of $\sin \omega_f t$ then choose

$u_1(x,t) = X_1(x) \sin \omega_f t$ and $u_2(x,t) = X_2(x) \sin \omega_f t$

put into DE $\Rightarrow X_1'' + \frac{\omega_f^2}{c^2} X_1 = 0$ for $0 \leq x \leq d$
 $X_2'' + \frac{\omega_f^2}{c^2} X_2 = 0$ for $d \leq x \leq l$

they must satisfy $X_1(0) = 0$ $X_2'(l) = 0$ and $X_1(d) = X_2(d)$, $X_2'(d) - X_1'(d) = -\frac{P_0}{EA}$

Thus $X_1(x) = B \sin \frac{\omega_f x}{c}$ $X_2(x) = D \cos \frac{\omega_f}{c}(l-x)$

From the matching conditions $B \sin \frac{\omega_f d}{c} - D \cos \frac{\omega_f}{c}(l-d) = 0$

and $\frac{\omega_f}{c} [D \sin \frac{\omega_f}{c}(l-d) - B \cos \frac{\omega_f d}{c}] = -\frac{P_0}{EA}$

thus $B = \frac{P_0}{EA} \frac{c}{\omega_f} \frac{\cos \frac{\omega_f}{c}(l-d)}{\cos \frac{\omega_f}{c} l}$ $D = \frac{P_0}{EA} \frac{c}{\omega_f} \frac{\sin \frac{\omega_f d}{c}}{\cos \frac{\omega_f}{c} l}$

Thus

$$\hat{u}(x,t) = \begin{cases} \frac{P_0 c}{EA \omega_f} \frac{\cos \frac{\omega_f}{c}(L-d) \sin \frac{\omega_f}{c} x}{\cos \frac{\omega_f}{c} L} \sin \omega_f t & 0 \leq x \leq d \\ \frac{P_0 c}{EA \omega_f} \frac{\sin \frac{\omega_f}{c} d \cos \frac{\omega_f}{c}(L-x)}{\cos \frac{\omega_f}{c} L} \sin \omega_f t & d \leq x \leq L \end{cases}$$

Thus from this we have the following IC

$$\hat{u}(x,0) = 0 \quad 0 \leq x \leq L$$

$$\frac{\partial \hat{u}}{\partial t}(x,0) = \begin{cases} \frac{P_0 c}{EA} \frac{\cos \frac{\omega_f}{c}(L-d) \sin \frac{\omega_f}{c} x}{\cos \frac{\omega_f}{c} L} & 0 \leq x \leq d \\ \frac{P_0 c}{EA} \frac{\sin \frac{\omega_f}{c} d \cos \frac{\omega_f}{c}(L-x)}{\cos \frac{\omega_f}{c} L} & d \leq x \leq L \end{cases}$$

let $\frac{P_0 c}{EA \cos \frac{\omega_f}{c} L} = \Gamma$

for problem #2: This part is not necessary if the problem is one of steady state
define $\ddot{u}(x,t)$

w/BC $\ddot{u}(0,t) = 0$ $\frac{\partial \ddot{u}}{\partial x}(L,t) = 0$

and IC $\ddot{u}(x,0) = -\hat{u}(x,0) = 0$ and $\frac{\partial \ddot{u}}{\partial t}(x,0) = -\frac{\partial \hat{u}}{\partial t}(x,0)$

let $\ddot{u} = X(x)T(t)$; put into PDE to get

$$X''T - \frac{1}{c^2}XT'' = 0 \quad \text{or} \quad \frac{X''}{X} = \frac{T''}{c^2T} = -\lambda^2$$

$$\therefore X'' + \lambda^2 X = 0 \quad \text{and} \quad T'' + \lambda^2 c^2 T = 0$$

$$X = E \sin \lambda x + F \cos \lambda x \quad \text{and} \quad T = G \sin \lambda c t + H \cos \lambda c t$$

applying BC $\Rightarrow X(0) = 0$ and $X'(L) = 0 \Rightarrow F = 0$ and $\cos \lambda L = 0 \therefore \lambda L = \frac{n\pi}{2}$ n odd

$$\therefore \lambda_n = \frac{n\pi}{2L} \quad \text{and} \quad X_n = E_n \sin \frac{n\pi x}{2L} \quad n \text{ odd}$$

and also IC $\Rightarrow T(0) = 0 \Rightarrow H = 0$ $\forall x$ $T'(0) = \lambda c G$ $\therefore$ let $E_n G = U_n$

$$\therefore \ddot{u}(x,t) = \sum_{n=1,3,5,\dots} U_n \sin \frac{n\pi x}{2L} \sin \frac{n\pi c t}{2L}$$

$$\text{and } \frac{\partial \ddot{u}}{\partial t}(x,0) = \frac{\pi c}{2L} \sum_{n=1,3,5,\dots} n U_n \sin \frac{n\pi x}{2L} = \begin{cases} -\Gamma \cos \frac{\omega_f}{c}(L-d) \sin \frac{\omega_f}{c} x & 0 \leq x \leq d \\ -\Gamma \sin \frac{\omega_f}{c} d \cos \frac{\omega_f}{c}(L-x) & d \leq x \leq L \end{cases}$$

$$\therefore \frac{\pi c}{2L} \int_0^L \sum_{n=1,3,5,\dots} n U_n \sin \frac{n\pi x}{2L} \sin \frac{n\pi x}{2L} dx = -\Gamma \left\{ \int_0^d \cos \frac{\omega_f}{c}(L-d) \sin \frac{\omega_f}{c} x \sin \frac{n\pi x}{2L} dx + \int_d^L \sin \frac{\omega_f}{c} d \cos \frac{\omega_f}{c}(L-x) \sin \frac{n\pi x}{2L} dx \right\}$$

and for $n \neq m$ the $U_m = \frac{4\Gamma}{\pi c} \left\{ \int_0^d \cos \frac{\omega_f}{c}(L-d) \sin \frac{\omega_f}{c} x \sin \frac{m\pi x}{2L} dx + \int_d^L \sin \frac{\omega_f}{c} d \cos \frac{\omega_f}{c}(L-x) \sin \frac{m\pi x}{2L} dx \right\}$

m is odd here & we assume $\frac{n\pi}{2L} \neq \frac{m\pi}{2L}$. If it is we have resonance. -2

for $n \neq m$ we have an identity and it's mathematically terrible to show so I won't

DIVISION OF APPLIED MECHANICS
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ME 236A Waves and Vibrations

Autumn 1979

Problem Set No. 3

1. Consider a uniform cantilever beam of length ℓ . Sketch the first three modes of free vibration with reasonable accuracy.
2. One end of a uniform beam is clamped and the other is subjected to a periodic displacement of the form $W = W_0 \sin \omega t$. Determine the steady-state motion.
3. Calculate the phase velocities of the two free harmonic waves in an infinite Timoshenko beam. Construct the plot of phase velocity versus wave number and discuss the limiting values for very large and very small wave numbers. Also construct the plot of frequencies versus wave number.
4. A beam of length ℓ is resting on an elastic foundation (modulus η). Determine the lowest mode of free vibration if both ends are free.

Problem Set #3

1. Consider a uniform cantilever beam of length l . Sketch the first 3 modes of free vibrations.

PDE governing is $EI W'''' + \rho A \ddot{W} = 0$ w/ BC $W(0) = W'(0) = 0$ and $EI W'''(L) = 0$ & $EI W''(L) = 0$

thus let $W(x,t) = X(x) T(t) \Rightarrow EI X'''' T + \rho A X \ddot{T} = 0 \Rightarrow \frac{X''''}{X} = -\frac{\ddot{T}}{T} \frac{\rho A}{EI} = +\lambda^4$

$X'''' - \lambda^4 X = 0$ choose $X(x) = Ce^{mx} \Rightarrow X = C_1 \sinh \lambda x + C_2 \cosh \lambda x + C_3 \sin \lambda x + C_4 \cos \lambda x$

$\ddot{T} + \lambda^4 \frac{EI}{\rho A} T = 0 \Rightarrow T(t) = A \cos \kappa^2 t + B \sin \kappa^2 t$ $\kappa^2 = \frac{EI}{\rho A} = c^2 r^2$ where $c^2 = E/\rho$, $r = \text{rad. of } g$

applying BC $\Rightarrow C_2 + C_4 = 0$ $C_1 + C_3 = 0$ or $C_2 = -C_4$; $C_1 = -C_3$

and

$C_1 \sinh \lambda L + C_2 \cosh \lambda L - C_3 \sin \lambda L - C_4 \cos \lambda L = 0$ if $\lambda \neq 0$

$C_1 \cosh \lambda L + C_2 \sinh \lambda L - C_3 \cos \lambda L + C_4 \sin \lambda L = 0$ if $\lambda \neq 0$

$\therefore \begin{bmatrix} (\sinh \lambda L + \sin \lambda L) & (\cosh \lambda L + \cos \lambda L) \\ (\cosh \lambda L + \cos \lambda L) & (\sinh \lambda L - \sin \lambda L) \end{bmatrix} \begin{pmatrix} C_1 \\ C_2 \end{pmatrix} = 0$

$\Rightarrow \det \equiv 0$ for nontrivial solns. $\Rightarrow \sinh^2 \lambda L + \sin^2 \lambda L - \cosh^2 \lambda L - 2 \cosh \lambda L \cos \lambda L - \cos^2 \lambda L = 0$

since $\cosh^2 \lambda L - \sinh^2 \lambda L = 1$ and $\cos^2 \lambda L + \sin^2 \lambda L = 1 \Rightarrow$

$-1 - 1 - 2 \cosh \lambda L \cos \lambda L = 0 \Rightarrow \cosh \lambda L \cos \lambda L = -1$

or $\lambda_1 L = 1.875$ $\lambda_2 L = 4.694$ $\lambda_3 L = 7.855$ from class notes

now $\frac{[\sinh(1.875) + \sin(1.875)]}{[\cosh(1.875) + \cos(1.875)]} C_1 = C_2$ & let $C_1 = 1$ $C_2 = -1.362$
 $C_3 = -1$ $C_4 = 1.362$

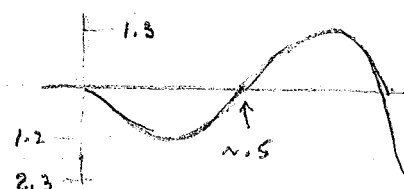
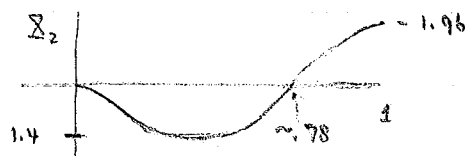
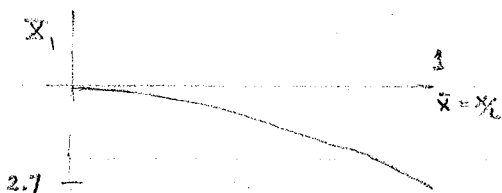
now $X_1 = (\sinh \lambda_1 x - \sin \lambda_1 x) + 1.362 (\cosh \lambda_1 x - \cos \lambda_1 x)$ $\lambda_1 x = 1.875 \bar{x}$ $\bar{x} = x/L$

also $\frac{[\sinh(4.694) + \sin(4.694)]}{[\cosh(4.694) + \cos(4.694)]} C_1 = C_2$ & let $C_1 = 1$ $C_2 = -1.982$
 $C_3 = -1$ $C_4 = 1.982$

$\therefore X_2 = (\sinh \lambda_2 x - \sin \lambda_2 x) + 1.982 (\cosh \lambda_2 x - \cos \lambda_2 x)$ $\lambda_2 x = 4.694 \bar{x}$

finally $\frac{[\sinh(7.855) + \sin(7.855)]}{[\cosh(7.855) + \cos(7.855)]} C_1 = C_2$ & let $C_1 = 1$ $C_2 = -1.001$
 $C_3 = -1$ $C_4 = 1.001$ $\lambda_3 x = 7.855 \bar{x}$

$X_3 = (\sinh \lambda_3 x - \sin \lambda_3 x) + 1.001 (\cosh \lambda_3 x - \cos \lambda_3 x)$ $\sim e^{-\lambda x} + \cosh \lambda x - \sin \lambda x$



see reverse for numbers



The beam of length L rests on an elastic foundation of modulus η . Find the lowest modes of free vibration if the ends are free. The governing PDE is $EI w'''' + \eta w + \rho A \ddot{w} = 0$

the bc are that $w''(0) = w''(L) = w'''(0) = w'''(L)$

$$\text{let } w = XT \Rightarrow EI X'''' T + \eta XT + \rho A X \ddot{T} = 0 \Rightarrow EI \frac{X''''}{X} + \eta = -\rho A \frac{\ddot{T}}{T} = \lambda^4$$

$$\text{thus } \ddot{T} + \lambda^4 \frac{\rho A}{EI} T = 0 \quad \kappa^2 = c^2 r^2 \text{ where } r = \text{radius of gyration} \therefore T = A \cos \kappa \lambda^2 t + B \sin \kappa \lambda^2 t$$

$$\text{also } X'''' - (\lambda^4 - \eta/EI) X = 0 \quad \text{let } \lambda^4 - \eta/EI = \mu^4$$

$$\text{then } X = C_1 \sinh \mu x + C_2 \cosh \mu x + C_3 \sin \mu x + C_4 \cos \mu x$$

$$\text{applying bc } w''(0) = 0 = C_2 - C_4 \Rightarrow C_2 = C_4$$

$$w'''(0) = 0 = C_1 - C_3 \Rightarrow C_1 = C_3$$

$$\left. \begin{array}{l} C_2 = C_4 \\ C_1 = C_3 \end{array} \right\} X = C_1 (\sinh \mu x + \sin \mu x) + C_2 (\cosh \mu x + \cos \mu x)$$

$$w''(L) = C_1 (\mu \sinh \mu L - \mu \sin \mu L) + C_2 (\mu \cosh \mu L - \mu \cos \mu L) = 0$$

$$w'''(L) = C_1 (\cosh \mu L - \cos \mu L) + C_2 (\sinh \mu L + \sin \mu L) = 0$$

$$\Rightarrow \det = 0 \text{ or } \{ \sinh^2 \mu L + \sinh \mu L \sin \mu L - \sinh \mu L \sin \mu L - \sin^2 \mu L - \cosh^2 \mu L + 2 \cosh \mu L \cos \mu L - \cos^2 \mu L \} = 0$$

$$\text{or } \{ -2 + 2 \cosh \mu L \cos \mu L \} = 0 \quad \text{or } \cosh \mu L \cos \mu L = 1 \quad \checkmark \quad \text{take } \mu = 0 \Rightarrow \lambda = \sqrt[4]{\eta/EI}$$

but does this mean we have a soln to bc & de : $\mu = 0 \Rightarrow X_0'''' = 0$ or $X_0 = ax^3 + bx^2 + cx + d$

$$X_0' = 3ax^2 + 2bx + c; \quad X_0'' = 6ax + 2b \Rightarrow w''(0) = 0 \Rightarrow b = 0 \text{ and } w''(L) = 0 \Rightarrow a = 0$$

$$X_0''' = 6a = 0 \quad \forall x \therefore \text{we do satisfy bc and } X_0 = cx + d \text{ thus } \ddot{T}_0 + \eta/\rho A T_0 = 0$$

$$\text{and } T_0 = A_1 \cos \sqrt{\eta/\rho A} t + B_1 \sin \sqrt{\eta/\rho A} t$$

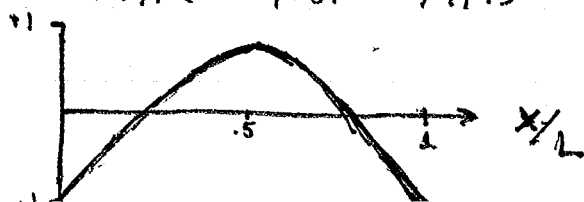
$$\therefore w_1(x, t) = C_1 x \cos \sqrt{\eta/\rho A} t + C_2 x \sin \sqrt{\eta/\rho A} t + d_1 \cos \sqrt{\eta/\rho A} t + d_2 \sin \sqrt{\eta/\rho A} t$$

X_0 represents a rigid body translation and rotation since $\frac{\partial w}{\partial x} = c \quad \forall x$

$\mu = 0$ is NOT A REALLY INTERESTING CASE

$\mu L = 4.73$ is the next one. The mode

SHAPE FOR THIS one looks like:



3. Calculate the phase velocities of the two free harmonic waves in an infinite Timoshenko Beam. Construct the plot of the phase velocity versus wave number and discuss the limiting values for a) very large and b) very small wave numbers. Also construct the plot of frequency versus wave number.

The governing PDE for the Timoshenko Beam Theory are

$$EI\psi'' + AG(w' - \psi) = \rho I \ddot{\psi} \quad (1)$$

$$AG(w' - \psi)' = \rho A \ddot{w} \quad (2)$$

by joining the two one can find the full pde on ψ only, namely

$$\psi''' - \left(\frac{E}{G} + 1\right) \frac{G}{\rho} \psi'' + \frac{G}{\rho} \frac{E}{\rho} \psi' + \frac{AG}{\rho I} \psi = 0 \quad (3)$$

Now let $\psi = \bar{\psi} e^{ikx} e^{i\omega t}$ and plug into (3) to obtain

$$\omega^4 - \left(\frac{E}{G} + 1\right) \frac{G}{\rho} k^2 \omega^2 + \frac{G}{\rho} \frac{E}{\rho} k^4 - \frac{AG}{I\rho} \omega^2 = 0 \quad (4) \quad \text{or}$$

$$c_p^4 - \left(\frac{E}{G} + 1\right) \frac{G}{\rho} c_p^2 + \frac{G}{\rho} \frac{E}{\rho} - \frac{AG}{I\rho} \frac{c_p^2}{k^2} = 0 \quad (5)$$

Solving (5) for c_p gives

$$c_p = \left\{ \frac{1}{2} \left(\frac{E}{G} + 1\right) \frac{G}{\rho} + \frac{1}{2} \frac{AG}{\rho I k^2} \pm \frac{1}{2} \left[\left\{ \left(\frac{E}{G} + 1\right) \frac{G}{\rho} + \frac{AG}{\rho I k^2} \right\}^2 - 4 \frac{GE}{\rho^2} \right]^{1/2} \right\}^{1/2} \quad (6)$$

or

$$\omega = \left\{ \frac{1}{2} \left(\frac{E}{G} + 1\right) \frac{G}{\rho} k^2 + \frac{1}{2} \frac{AG}{\rho I} \pm \frac{1}{2} \left[\left\{ \left(\frac{E}{G} + 1\right) \frac{G}{\rho} k^2 + \frac{AG}{\rho I} \right\}^2 - 4 \frac{GE}{\rho^2} k^4 \right]^{1/2} \right\}^{1/2} \quad (7)$$

Let $k \rightarrow \infty$ for (6). Then

$$c_p \approx \left\{ \frac{1}{2} \left(\frac{E}{G} + 1\right) \frac{G}{\rho} \pm \frac{1}{2} \left(\frac{E}{G} + 1\right) \frac{G}{\rho} \right\}^{1/2} \quad \text{or} \quad c_{p1} = \left(\frac{E}{\rho}\right)^{1/2} \quad \text{or} \quad c_{p2} = \left(\frac{G}{\rho}\right)^{1/2}$$

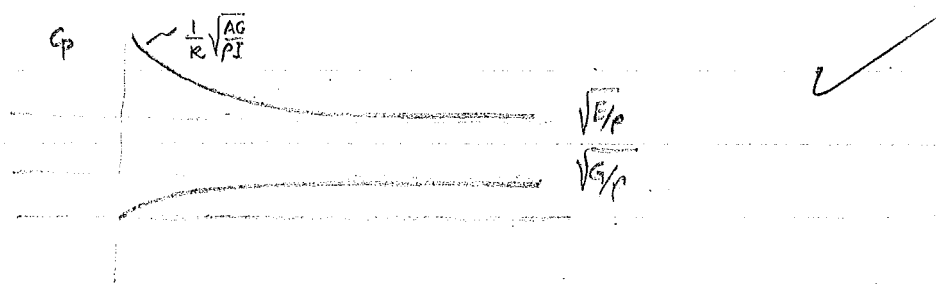
Let $k \rightarrow 0$ for (6). Then

$$c_p \approx \left\{ \frac{1}{2} \frac{AG}{\rho I k^2} + \frac{1}{2} \left(\frac{E}{G} + 1\right) \frac{G}{\rho} \pm \frac{1}{2} \frac{AG}{\rho I k^2} \left[1 + \left(\frac{E}{G} + 1\right) \frac{I}{A} k^2 + O(k^4) \right] \right\}^{1/2}$$

$$c_{p1} = \frac{1}{k} \sqrt{\frac{AG}{\rho I}} \quad \text{and} \quad c_{p2} = 0$$

Discussion: For very large $k \leftarrow$ very small λ , the boundaries appear very far away. Thus to a wave the beam appears to be an infinite body. The infinite body, as shown in class, has $c_{p1} = \sqrt{E/\rho}$ and $c_{p2} = \sqrt{G/\rho}$ as our results indicate. For very small $k \leftarrow$ very large λ , the

wave travels to the boundary in an infinitesimal time; hence the beam must have an ∞ velocity i.e. it is independent of x , which explains why $c_{p1} \rightarrow \infty$. Since w, ψ are independent of x , then ψ will have a harmonic solution of $\cos \sqrt{\frac{E}{\rho}} t$ & $\sin \sqrt{\frac{E}{\rho}} t$ with $\frac{w}{R} \frac{1}{k} \sqrt{\frac{AG}{\rho I}} = c_{p1}$. But $\ddot{w} = 0 \Rightarrow w$ is a constant hence we only have rigid body motion in the transverse direction $\Rightarrow c_{p2} = 0$ as shown. Thus



for the frequency let $k \rightarrow \infty$ for (4). Then

$$\omega \sim \left\{ \frac{1}{2} (E/G + 1) G/\rho k^2 \pm \frac{1}{2} (E/G - 1) G/\rho k^2 \right\} \therefore \omega_1 \sim \sqrt{E/\rho} k \quad \omega_2 \sim \sqrt{G/\rho} k$$

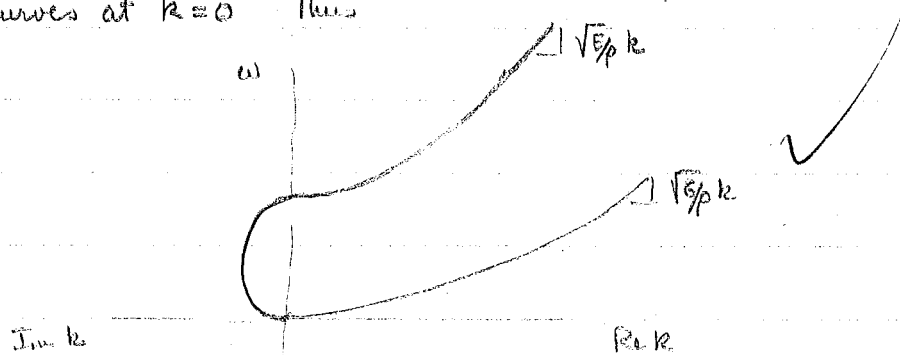
for the frequency let $k \rightarrow 0$ for (7). Then

$$\omega \sim \left\{ \frac{1}{2} \frac{AG}{\rho I} \pm \frac{1}{2} \frac{AG}{\rho I} + O(k^2) \right\}^{1/2} \therefore \omega_1 \sim \sqrt{\frac{AG}{\rho I}} \quad \omega_2 \sim 0$$

for k imaginary & let $x = \omega^2$ $y = k^2$

$$x^2 + (E/G + 1) G/\rho xy + \frac{G}{\rho} \frac{E}{\rho} y^2 - \frac{AG}{I\rho} x = 0 \text{ . This is of form } Ax^2 + Bxy + Cy^2 + Ex + Dy + F = 0$$

This is an equation of an ellipse since $B^2 - 4AC > 0$. The ellipse intersects the two curves at $k=0$ Thus



2. If one end of beam is subjected to $w = w_0 \sin \omega t$ and the other is clamped, determine the steady state motion.

 the governing PDE is $EI w'''' + \rho A \ddot{w} = 0$ ✓

thus will BC that $w(0) = 0$ and $w'(0) = 0$ $w(L) = w_0 \sin \omega t \quad \forall t$ and $EI w''(L) = 0$

then let $X(x)T(t) = w(x,t)$; put into PDE and rearrange.

then $\frac{X''''}{X} = -\frac{T''}{T} \frac{\rho A}{EI} = \lambda^4$ and $\Rightarrow T = A \cos \kappa \lambda^2 t + B \sin \kappa \lambda^2 t$ $\kappa^2 = c^2 r^2$

-is rad. of gyr.; because of the boundary condition on $w(L,t) \Rightarrow \omega = \kappa \lambda^2$ and $A = 0$

$\therefore X = c_1 \sinh \lambda x + c_2 \cosh \lambda x + c_3 \sin \lambda x + c_4 \cos \lambda x$

applying first 2 bc $\Rightarrow c_4 = -c_2$ and $c_3 = -c_1$

Now $w_0 = c_1 (\sinh \lambda L - \sin \lambda L) + c_2 (\cosh \lambda L - \cos \lambda L)$ from 3rd bc

$0 = c_1 (\sinh \lambda L + \sin \lambda L) + c_2 (\cosh \lambda L + \cos \lambda L)$ from 4th bc

then

$$c_1 = \frac{\begin{bmatrix} w_0 & \cosh \lambda L - \cos \lambda L \\ 0 & \cosh \lambda L + \cos \lambda L \end{bmatrix}}{2 [\sinh \lambda L \cos \lambda L - \sin \lambda L \cosh \lambda L]} = \frac{w_0 [\cos \lambda L + \cosh \lambda L]}{2 [\sinh \lambda L \cos \lambda L - \sin \lambda L \cosh \lambda L]}$$

$$c_2 = \frac{\begin{bmatrix} \sinh \lambda L - \sin \lambda L & w_0 \\ \sinh \lambda L + \sin \lambda L & 0 \end{bmatrix}}{2 [\sinh \lambda L \cos \lambda L - \sin \lambda L \cosh \lambda L]} = \frac{-w_0 (\sinh \lambda L + \sin \lambda L)}{2 [\sinh \lambda L \cos \lambda L - \sin \lambda L \cosh \lambda L]}$$

$\therefore X(x)T(t) = w(x,t) = [c_1 (\sinh \lambda x - \sin \lambda x) + c_2 (\cosh \lambda x - \cos \lambda x)] \sin \kappa \lambda^2 t$ where $\lambda = \sqrt{\frac{\omega}{\kappa}}$

$$w(x,t) = \frac{w_0 \{ [\cosh \lambda L + \cos \lambda L] [\sinh \lambda x - \sin \lambda x] - [\sinh \lambda L + \sin \lambda L] [\cosh \lambda x - \cos \lambda x] \}}{2 (\sinh \lambda L \cos \lambda L - \sin \lambda L \cosh \lambda L)} \sin \omega t$$

To prove this method of attack is OK. Look at following:

Suppose we assume $w(x,t) = \bar{w}(x,t) + f(t)g(x)$ $f(t) = w_0 \sin \omega t$

we then must solve $EI w'''' + \rho A \ddot{w} = EI \bar{w}'''' + \rho A \ddot{\bar{w}} + EI f(t) g'''' + \rho A \ddot{f} g = 0$

with $w(0,t) = \bar{w}(0,t) + f(t)g(0) = 0 \Rightarrow g(0) = 0$ $\bar{w}(0,t) = 0$

$w'(0,t) = \bar{w}'(0,t) + f(t)g'(0) = 0 \Rightarrow g'(0) = 0$ $\bar{w}'(0,t) = 0$

$w(L,t) = \bar{w}(L,t) + f(t)g(L) = f(t) \Rightarrow g(L) = 1$ $\bar{w}(L,t) = 0$

$w''(L,t) = \bar{w}''(L,t) + f(t)g''(L) = 0 \Rightarrow g''(L) = 0$ $\bar{w}''(L,t) = 0$

with the given bc on $g(x)$ let us pick $g(x) = Ax^3 + Bx^2 + Cx + D$

using bc we find that $C=D=0$ $A = -\frac{1}{2}l^3$ $B = \frac{3}{2}l^2$ $\therefore g(x) = -\frac{1}{2}\left(\frac{x}{l}\right)^3 + \frac{3}{2}\left(\frac{x}{l}\right)^2$

$$\therefore EI \bar{w}^{IV} + \rho A \bar{w}'' = -\rho A f(t) g(x) \quad \text{since } g(x) \text{ is a 3rd degree eqn only.}$$

$$= \rho A \omega^2 f(t) g(x)$$

The solution to $\bar{w}$ $\exists$. $\bar{w} = \bar{w}^h + \bar{w}^p$ with $EI \bar{w}^{IV} + \rho A \bar{w}'' = 0$ & $\bar{w}^h(0, t) = \bar{w}^h(l, t) = 0$

$\bar{w}^h(l, t) = \bar{w}^h(l, t) = 0$ gives the characteristic equation:

$\left[\sinh \lambda l \cosh \lambda l - \sinh \lambda l \cosh \lambda l = 0 \right]^{**}$ λ being the separation of variable constant related to the frequencies since $\omega_n = \lambda_n^2 \sqrt{\frac{EI}{\rho A}}$.

$$\Sigma_n \text{ (the eigenfn)} = (\cosh \lambda x - \cos \lambda x) (\sinh \lambda l - \sin \lambda l) - (\cosh \lambda l - \cos \lambda l) (\sinh \lambda x - \sin \lambda x)$$

$$(\sinh \lambda l - \sin \lambda l)$$

if we continue the procedure as outlined then $\rho A \omega^2 f(t) g(x) = \sum_{n=1}^{\infty} \Sigma_n(x) g_n(t)$

$$\text{where } g_n(t) = \frac{\int_0^l \rho A \omega^2 f(t) g(x) dx}{\int_0^l \Sigma_n^2(x) dx} = A_n(\rho, A; l; \omega) f(t) \quad \text{where } A_n \text{ is a const.}$$

$$\text{Then } EI \bar{w}^{IV} + \rho A \bar{w}'' = \sum_{n=1}^{\infty} A_n(\rho, A; l; \omega) f(t) = f(t) \sum_{n=1}^{\infty} A_n(\rho, A; l; \omega)$$

and since $f(t)$ is periodic, we could have taken $\bar{w} = F(x) f(t)$

since the DE would have reduced to $EI F^{IV} - \omega^2 \rho A F = \rho A \omega^2 g(x)$

with bc. $F(0) = F'(0) = F''(l) = F(l) = 0$.

All this means is that $w(x, t) = \bar{w}(x, t) + f(t) g(x) = [F(x) + g(x)] f(t)$

hence we could have started by assuming a form $w(x, t) = G(x) \sin \omega t$ without loss in generality. Physically the vibrational frequency of the beam at steady state when all transients are gone must be ω .

If you will note the frequency equation $**$ and my solution on first page this problem, you will note that the denominator are the same. Hence if ω is one of the natural frequencies, $w(x, t) \rightarrow \infty$ as expected. The reason why this works in this case is that since we are dealing in steady state solutions, the initial conditions' effect has vanished and only the bc's provide the dominant effect to the vibration of the beam.

Looks ok.

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OFFICIAL EXAMINATION BOOK

24 Page Ruled

Question	Score
1	7
2	10
3	
4	
5	
6	
7	
8	
Total	17/20

Name of student CESAR LEVY

Date of examination 13 NOV 79

Course ME236A

THE STANFORD UNIVERSITY HONOR CODE

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 - (2) that they will do their share and take an active part in seeing to it that others as well as themselves uphold the spirit and letter of the Honor Code.
- B. The faculty on its part manifests its confidence in the honor of its students by refraining from proctoring examinations and from taking unusual and unreasonable precautions to prevent the forms of dishonesty mentioned above. The faculty will also avoid, as far as practicable, academic procedures that create temptations to violate the Honor Code.
- C. While the faculty alone has the right and obligation to set academic requirements, the students and faculty will work together to establish optimal conditions for honorable academic work.

I acknowledge and accept the Honor Code.

(Signed) Cesar Levy

*Interpretations and applications of the Honor Code
appear on the back cover of this examination book.*

DIVISION OF APPLIED MECHANICS
DEPARTMENT OF MECHANICAL ENGINEERING
STANFORD UNIVERSITY

ME 236A Waves and Vibrations

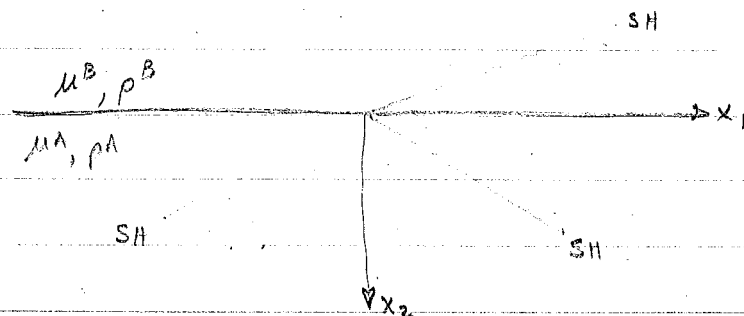
Autumn 1979

Midterm Examination

1. Consider 2 elastic half-spaces (with properties μ^A , ρ^A and μ^B , ρ^B , respectively) bonded at the plane interface. Consider an incident SH-wave and determine the amplitude ratios of the transmitted and reflected waves with respect to the incident wave. Also, determine the angle of incidence at which there will be no reflected, but only a transmitted wave.

*see my notes
on Seis mology*

2. Consider a very long, elastic cylinder of rectangular cross-section (edge length a and b). It is welded to a fixed outer housing along all four faces. Determine the frequencies of free axial vibration by considering the displacement in the axial direction to be independent of the axial coordinate, while the displacement components normal to the axis vanish (antiplane strain).



for SH wave, $u_3 = u_3(x_1, x_2, t)$

DE governing is $\Delta u_3 = \frac{1}{c_2^2} u_{3,tt}$ take $u_3 = f(x_2) e^{i(kx_1 - \omega t)}$

then $\Delta u_3 - \frac{1}{c_{2,A,B}^2} u_{3,tt} = (f'' - \frac{1}{2} k^2 f) + \frac{\omega^2}{c_2^2} f = 0$ or let $\beta_{A,B}^2 = \frac{\omega^2}{c_{2,A,B}^2} - k^2$
 $\beta^2 = k^2 \left(\left(\frac{c_2}{c_2^A} \right)^2 - 1 \right)$

thus $f'' + \beta_{A,B}^2 f = 0$ or $f = A \sin \beta_{A,B} x_2 + B \cos \beta_{A,B} x_2 = f(x_2) = A e^{i \beta_{A,B} x_2}$

thus SH $u_3 = A_1 e^{i(kx_1 - \omega t + \beta_A x_2)}$ for upgoing

$\checkmark$ $u_3 = A_2 e^{i(kx_1 - \omega t - \beta_A x_2)}$ for downgoing wave

$u_3 = A_3 e^{i(kx_1 - \omega t + \beta_B x_2)}$ for transmitted wave.

at boundary $\mu_A \frac{\partial u_3}{\partial x_2} \Big|_{x_2=0^+} = \mu_B \frac{\partial u_3}{\partial x_2} \Big|_{x_2=0^-} = \sigma_{3,2}$ must be continuous.

and $\mu_B \frac{\partial u_3}{\partial x_1} \Big|_{x_2=0^-} = \mu_A \frac{\partial u_3}{\partial x_1} \Big|_{x_2=0^+} = \sigma_{31}$ must be cont. why?

$\therefore i \beta_A A_1 \mu_A - i \beta_A A_2 \mu_A = i \beta_B A_3 \mu_B$
 $\mu_A A_1 + \mu_A A_2 = \mu_B A_3$

$\beta_A = k^2 \left[\left(\frac{c_2}{c_2^A} \right)^2 - 1 \right]$

$\beta_B = k^2 \left[\left(\frac{c_2}{c_2^B} \right)^2 - 1 \right]$

$\Rightarrow$

$\begin{bmatrix} \beta_A \mu_A \\ \mu_A \end{bmatrix} = \begin{bmatrix} \beta_A \mu_A & \beta_B \mu_B \\ -\mu_A & \mu_B \end{bmatrix} \begin{bmatrix} A_2/A_1 \\ A_3/A_1 \end{bmatrix}$

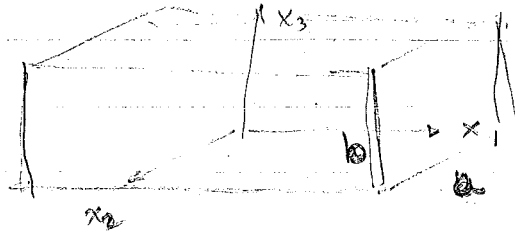
$A_2/A_1 = \frac{\beta_A \mu_A \mu_B - \mu_A \mu_B \beta_B}{\beta_A \mu_A \mu_B + \beta_B \mu_A \mu_B} = \frac{\beta_A - \beta_B}{\beta_A + \beta_B}$

no reflect $\Rightarrow \beta_A = \beta_B$

$A_3/A_1 = \frac{\beta_A \mu_A^2 + \beta_B \mu_A^2}{(\beta_A + \beta_B) \mu_A \mu_B} = \frac{2 \beta_A \mu_A}{\mu_B (\beta_A + \beta_B)}$

continuity of displacement.

2.



u_1 : displacements in x_1 direction $f(x_2, x_3)$

$$u_2 = 0, u_3 = 0$$

antiplane strain

$$\Delta u_1 = \frac{1}{c_2^2} u_{1,tt} = 0$$

$$\text{let } u_1 = f(x_2) g(x_3) e^{i\omega t}$$

Put into de.

$$f''g + g''f + \frac{\omega^2}{c_2^2} fg = 0$$

$$\therefore \frac{f''}{f} + \frac{\omega^2}{c_2^2} = -\frac{g''}{g} = +\lambda^2 \quad \therefore \quad g'' + \lambda^2 g = 0 \quad \lambda \neq 0$$

$$\text{and } f'' + \left(\frac{\omega^2}{c_2^2} - \lambda^2\right)f = 0$$

$$\therefore g = \tilde{A}_1 e^{i\lambda x_3} + \tilde{A}_2 e^{-i\lambda x_3} = A \cos \frac{\lambda}{2} x_3 + B \sin \frac{\lambda}{2} x_3$$

$$g|_{x_3 = \pm b/2} = 0 \quad \text{welded condition}$$

$$\therefore A \cos \frac{\lambda b}{2} + B \sin \frac{\lambda b}{2} = 0$$

$$A \cos \frac{\lambda b}{2} - B \sin \frac{\lambda b}{2} = 0$$

$$\Rightarrow 2 \cos \frac{\lambda b}{2} \sin \frac{\lambda b}{2} = 0 \quad \text{for } A, B \neq 0$$

$$\therefore \sin \lambda b = 0 \quad \lambda b = n\pi \quad \therefore \left| \lambda_n = \frac{n\pi}{b} \right|$$

$$f = C \sin \beta x_2 + D \cos \beta x_2 \quad \beta^2 = \frac{\omega^2}{c_2^2} - \lambda^2$$

$$f|_{x_2 = \pm a/2} = 0 \quad \text{welded condition} \Rightarrow \beta_m = \frac{m\pi}{a}$$

$$\text{but } \beta_m^2 = \frac{m^2 \pi^2}{a^2} = \frac{\omega_{mn}^2}{c_2^2} - \lambda_n^2 = \frac{\omega_{mn}^2}{c_2^2} - \frac{n^2 \pi^2}{b^2}$$

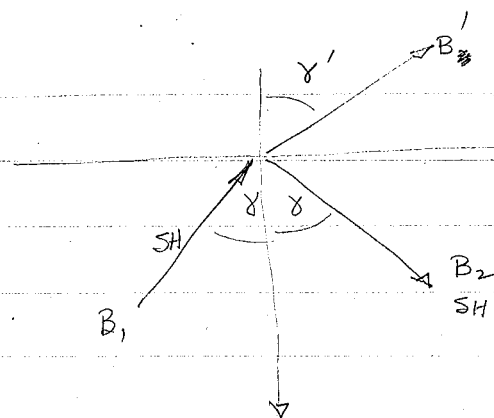
$$\therefore \omega_{mn}^2 = \pi^2 c_2^2 \left[\frac{m^2}{a^2} + \frac{n^2}{b^2} \right] \quad m, n \geq 1$$

$$\text{for } \lambda = 0 \Rightarrow g'' = 0 \quad \cancel{f''} \quad \cancel{f = \frac{\omega^2}{c_2^2} x_2^2 + b_1 x_2 + b_2}$$

$$g|_{x_3 = \pm b/2} = 0 \Rightarrow \begin{aligned} a_1 b/2 + a_2 &= 0 \\ -a_1 b/2 + a_2 &= 0 \end{aligned} \quad \det \begin{bmatrix} b/2 & 1 \\ -b/2 & 1 \end{bmatrix} \neq 0 \Rightarrow a_1 = a_2 = 0$$

$$f|_{x_2 = \pm a/2} = 0 \Rightarrow \begin{aligned} b_1 a/2 + b_2 &= 0 \\ -b_1 a/2 + b_2 &= 0 \end{aligned} \quad \Rightarrow \begin{aligned} b_1 &= -b_2 \\ b_1 &= b_2 \end{aligned} \Rightarrow b_1 = b_2 = 0$$

Solution



$$\frac{\mu_B \rho_B}{\mu_A \rho_A}$$

$$\frac{B_2}{B_1} = \frac{\mu_A \cos \gamma - \mu_B \left(\frac{c_s^A}{c_s^B} \right) \cos \gamma'}{\mu_A \cos \gamma + \mu_B \left(\frac{c_s^A}{c_s^B} \right) \cos \gamma'}$$

$$\frac{B_1'}{B_1} = \frac{2 \rho_A \mu_B \cos \gamma}{\rho_B \left[\mu_A \cos \gamma + \mu_B \left(\frac{c_s^A}{c_s^B} \right) \cos \gamma' \right]}$$

$$\text{for } B_2 = 0 \Rightarrow \cos \gamma = \frac{\mu_B}{\mu_A} \left(\frac{c_s^A}{c_s^B} \right) \cos \gamma'$$

$$\omega^2 = c_s^2 \pi^2 \left[\left(\frac{n}{a} \right)^2 + \left(\frac{m}{b} \right)^2 \right]$$

Final A

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Question	Score
1	10/10
2	9/10
3	10/10
4	10/10
5	10/10
6	
7	
8	
Total	49/50

Name of student Cesar Levy

Date of examination Dec 10, 1979

Course ME 236A Waves and Vibrations

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- (1) that they will not give or receive aid in examinations; that they will not give or receive unpermitted aid in class work, in the preparation of reports, or in any other work that is to be used by the instructor as the basis of grading;
 - (2) that they will do their share and take an active part in seeing to it that others as well as themselves uphold the spirit and letter of the Honor Code.
- B. The faculty on its part manifests its confidence in the honor of its students by refraining from proctoring examinations and from taking unusual and unreasonable precautions to prevent the forms of dishonesty mentioned above. The faculty will also avoid, as far as practicable, academic procedures that create temptations to violate the Honor Code.
- C. While the faculty alone has the right and obligation to set academic requirements, the students and faculty will work together to establish optimal conditions for honorable academic work.

I acknowledge and accept the Honor Code.

(Signed) Cesar Levy

Interpretations and applications of the Honor Code
appear on the back cover of this examination book.

Prof. Herrmann: The HW assignment is included inside

DIVISION OF APPLIED MECHANICS
DEPARTMENT OF MECHANICAL ENGINEERING
STANFORD UNIVERSITY

ME 236A Waves and Vibrations

Autumn 1979

Final Examination

1. For the Rayleigh-Lamb problem, assume displacement potentials of the form

$$\phi = (A_1 e^{iax_2} + A_2 e^{-iax_2}) e^{i(kx_1 - \omega t)}$$

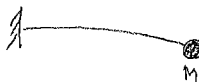
$$\psi = (A_3 e^{ibx_2} + A_4 e^{-ibx_2}) e^{i(kx_1 - \omega t)}$$

where $a = (\omega^2/c_l^2 - k^2)^{1/2}$, $b = (\omega^2/c_t^2 - k^2)^{1/2}$, and find the dispersion equation in terms of the roots of a 4×4 determinant. By means of simple row and column operations on the determinant, show that it may be factored into the product of two 2×2 determinants. Evaluate the two determinants, and show that they correspond to the symmetric and antisymmetric cases discussed in class.

- ✓ 2. An elastic rod of elastic modulus E , mass density ρ and cross-sectional area A is embedded in another elastic medium, whose effect on the rod can be approximated by postulating that the axial force (per unit length) exerted by the medium on the rod is proportional to the axial displacement (constant η). Derive the equation of motion for the rod as an extension of the elementary theory and calculate (and sketch) the frequency as a function of the wave number.

$$F/A = u\eta$$

- ✓ 3. Consider a very long beam which is resting on an elastic foundation and use elementary theory (stiffness EI , cross-sectional area A , mass density ρ , foundation modulus k). Calculate, sketch and discuss the dependence of phase velocity, group velocity and frequency on wave number for free harmonic waves.
4. Determine the frequency equation of free flexural motions of a uniform elastic beam of length ℓ , whose one end is clamped and the other supported by a spring of stiffness k . Consider limiting cases of $k = 0$ and $k = \infty$ and also the relationships for high frequencies. ? Call & ask about this: Sunday
5. A cantilever beam of length ℓ and stiffness EI carries a concentrated mass M at the free end. Choose a reasonable deflection curve and use Rayleigh's method to find an approximate value of the lowest natural frequency of bending vibration.



$$\text{where } A_1 + A_2 = B_1$$

$$(A_1 - A_2)i = B_2$$

$$(A_3 + A_4) = B_3$$

$$(A_3 - A_4)i = B_4$$

$$f(x_2) = B_1 \cos ax_2 + B_2 \sin ax_2$$

$$g(x_2) = B_3 \cos bx_2 + B_4 \sin bx_2$$

$$\phi_1 = f(x_2) e^{i(kx_1 - \omega t)}$$

$$\psi = g(x_2) e^{i(kx_1 - \omega t)}$$

$$u_2 = \frac{\partial \phi}{\partial x_1} + \frac{\partial \psi}{\partial x_1} = (ikf + g') e^{i(\dots)}$$

$$u_2 = \frac{\partial \phi}{\partial x_1} - \frac{\partial \psi}{\partial x_1} = (f' - ikg) e^{i(\dots)}$$

$$u_{1,1} = (-k^2 f + ikg') e^{i(\dots)}$$

$$u_{1,2} = (ikf' + g'') e^{i(\dots)} = (ikf' - b^2 g) e^{i(\dots)}$$

$$u_{2,1} = (ikf' + k^2 g) e^{i(\dots)}$$

$$u_{2,2} = (f'' - ikg') e^{i(\dots)} = (-a^2 f - ikg') e^{i(\dots)}$$

since $\sigma_{22} = \sigma_{21} = 0$ on bdy

$$\sigma_{22} = (\lambda + 2\mu) u_{2,2} + \lambda u_{1,1} = [(\lambda + 2\mu)(-a^2 f - ikg') + \lambda(-k^2 f + ikg')] e^{i(\dots)}$$

$$\sigma_{21} = \mu(u_{2,1} + u_{1,2}) = [\mu(2ikf' + (k^2 - b^2)g)] e^{i(\dots)} = \sigma_{21}$$

$$\sigma_{22} = [f(-a^2 \lambda - 2\mu a^2 - k^2 \lambda) + ikg'(\lambda - \lambda - 2\mu)] e^{i(\dots)}$$

$$[f\{-\lambda(a^2 + k^2) - 2\mu a^2\} - ikg'(2\mu)] e^{i(\dots)}$$

$$\text{but } \frac{\lambda}{\mu} = \frac{\lambda/\rho}{\mu/\rho} = \frac{c_1^2 - 2c_2^2}{c_1^2} = \frac{c_1^2}{c_2^2} - 2; \quad \frac{c_1^2}{c_2^2} = b^2 + k^2 \quad \frac{c_1^2}{c_2^2} = a^2 + k^2$$

$$\therefore \frac{c_1^2}{c_2^2} = \left(\frac{c_1}{c_2}\right)^2 / \left(\frac{c_1}{c_1}\right)^2 = \frac{b^2 + k^2}{a^2 + k^2} \quad \text{and} \quad \frac{c_1^2}{c_2^2} - 2 = \frac{b^2 + k^2 - 2a^2 - 2k^2}{a^2 + k^2}$$

$$\therefore -\lambda(a^2 + k^2) - 2\mu a^2 = -\frac{\lambda}{\mu}(a^2 + k^2)\mu - 2\mu a^2$$

$$= (-b^2 + k^2 + 2a^2)\mu - 2\mu a^2 = -(b^2 - k^2)\mu$$

$$\therefore \sigma_{22} = [-(b^2 - k^2)\mu f - ikg'(2\mu)] e^{i(\dots)} = -\mu[(b^2 - k^2)f + 2ikg'] e^{i(\dots)}$$

Now since $\sigma_{21}(@ x_2 = h) = 0$

$$f' = a[-B_1 \sin ax_2 + B_2 \cos ax_2] \quad g'(x_2) = b[-B_3 \sin bx_2 + B_4 \cos bx_2]$$

since we have $\sigma_{21} + \sigma_{22} = 0$ on bdy we can drop $\pm \mu e^{i(\dots)}$ from both eq.

$$@ x_2 = h$$

$$\therefore \sigma_{21} = [2ika(-B_1 \sin ah + B_2 \cos ah) + (k^2 - b^2)(B_3 \cos bh + B_4 \sin bh)] = 0$$

$$x_2 = -h$$

$$= [2ika(B_1 \sin ah + B_2 \cos ah) + (k^2 - b^2)(B_3 \cos bh - B_4 \sin bh)] = 0$$

$$x_2 = h$$

$$\sigma_{22} = [(b^2 - k^2)(B_1 \cos ah + B_2 \sin ah) + 2ikb(-B_3 \sin bh + B_4 \cos bh)] = 0$$

$$x_2 = -h$$

$$= [(b^2 - k^2)(B_1 \cos ah - B_2 \sin ah) + 2ikb(B_3 \sin bh + B_4 \cos bh)] = 0$$

Thus

$$\begin{bmatrix} -2ika \sinh & 2ika \cosh & (k^2 - b^2) \cosh & (k^2 - b^2) \sinh \\ 2ika \sinh & 2ika \cosh & (k^2 - b^2) \cosh & -(k^2 - b^2) \sinh \\ (b^2 - k^2) \cosh & (b^2 - k^2) \sinh & -2ikb \sinh & 2ikb \cosh \\ (b^2 - k^2) \cosh & -(b^2 - k^2) \sinh & 2ikb \sinh & 2ikb \cosh \end{bmatrix} \begin{pmatrix} B_1 \\ B_2 \\ B_3 \\ B_4 \end{pmatrix} = 0$$

Since b, a are fun of ω & k , this is a determinant that implicitly determines ω as a fun of k .

$$4 \begin{bmatrix} -2ika \sinh & 2ika \cosh & (k^2 - b^2) \cosh & (k^2 - b^2) \sinh \\ 0 & 2ika \cosh & (k^2 - b^2) \cosh & 0 \\ (b^2 - k^2) \cosh & (b^2 - k^2) \sinh & -2ikb \sinh & 2ikb \cosh \\ 1(b^2 - k^2) \cosh & 0 & 0 & 2ikb \cosh \end{bmatrix} \begin{pmatrix} B_1 \\ B_2 \\ B_3 \\ B_4 \end{pmatrix} = 0$$

$$4 \begin{bmatrix} -2ika \sinh & 0 & 0 & (k^2 - b^2) \sinh \\ 0 & 2ika \cosh & (k^2 - b^2) \cosh & 0 \\ 0 & (b^2 - k^2) \sinh & -2ikb \sinh & 0 \\ (b^2 - k^2) \cosh & 0 & 0 & 2ikb \cosh \end{bmatrix} \begin{pmatrix} B_1 \\ B_2 \\ B_3 \\ B_4 \end{pmatrix} = 0$$

↑ mult (1) by $\frac{(b^2 - k^2)}{2ika} \frac{1}{\tanh}$ & add to (4) mult (3) by $-\frac{(b^2 - k^2) \sinh}{2ika \cosh}$ & add to (3)

$$\begin{bmatrix} -2ika \sinh & 0 & 0 & (k^2 - b^2) \sinh \\ 0 & 2ika \cosh & 0 & 0 \\ 0 & 0 & (k^2 - b^2) \cosh & 0 \\ 0 & 0 & -2ikb \sinh + Y & 2ikb \cosh + X \end{bmatrix}$$

$Y = \frac{(k^2 - b^2)^2 \cosh \sinh}{2ika \cosh} \quad X = -\frac{(k^2 - b^2)^2 \sinh}{2ika \tanh}$

This det in form $\begin{bmatrix} A & B \\ 0 & C \end{bmatrix} = \det A \cdot \det C$.

$$\det A = 4k^2 a^2 \sinh \cosh$$

$$\det C = (k^2 - b^2) \cosh \left\{ (2ikb \cosh) + i \frac{(k^2 - b^2)^2 \sinh}{2ka \tanh} \right\}$$

$$\text{thus } \det C = i \frac{(k^2 - b^2) \cosh^2}{2ka (k^2 - b^2)^2} \left\{ 4k^2 ab \tanh + (k^2 - b^2)^2 \tanh \right\} = 0$$

$$\therefore 4k^2 ab \tanh = -(k^2 - b^2)^2 \tanh$$

This is the dispersion

relation for the symmetric modes.

$$\text{or } \frac{\tanh}{\tanh} = -\frac{(k^2 - b^2)^2}{4k^2 ab} \quad \text{or } \frac{\tanh}{\tanh} = -\frac{4k^2 ab}{(k^2 - b^2)^2}$$

or

$$4k^2 a^2 \sinh \cosh \cdot i (k^2 - b^2)$$

$$A_1 \cosh + A_2 \sinh = A_1 \cosh + A_2 \sinh$$

since $A_1 + A_2 = B_1$; $(A_1 - A_2)i = B_2$; $A_3 + A_4 = B_3$; $(A_3 - A_4)i = B_4$

then
$$\begin{bmatrix} \text{matrix} \end{bmatrix} \begin{pmatrix} B_1 \\ B_2 \\ B_3 \\ B_4 \end{pmatrix} = \left(\begin{bmatrix} a_{11} & a_{12} & a_{13} & a_{14} \\ a_{21} & a_{22} & a_{23} & a_{24} \\ a_{31} & a_{32} & a_{33} & a_{34} \\ a_{41} & a_{42} & a_{43} & a_{44} \end{bmatrix} + i \begin{bmatrix} a_{12} & -a_{11} & a_{14} & -a_{13} \\ a_{22} & -a_{21} & a_{24} & -a_{23} \\ a_{32} & -a_{31} & a_{34} & -a_{33} \\ a_{42} & -a_{41} & a_{44} & -a_{43} \end{bmatrix} \right) \begin{pmatrix} A_1 \\ A_2 \\ A_3 \\ A_4 \end{pmatrix} = \begin{bmatrix} i a_{12} & a_{11} & i a_{14} & a_{13} \\ i a_{22} & a_{21} & i a_{24} & a_{23} \\ i a_{32} & a_{31} & i a_{34} & a_{33} \\ i a_{42} & a_{41} & i a_{44} & a_{43} \end{bmatrix} \begin{pmatrix} A_1 \\ A_2 \\ A_3 \\ A_4 \end{pmatrix}$$

~~to get 2nd solution + eliminate upper left + lower right~~

$$\begin{bmatrix} -2ika \sinh & 2ika \cosh & (k^2 - b^2) \cosh b & (k^2 - b^2) \sinh b \\ 2ika \sinh & 2ika \cosh & (k^2 - b^2) \cosh b & -(k^2 - b^2) \sinh b \\ (b^2 - k^2) \cosh & (b^2 - k^2) \sinh & -2ikb \sinh b & 2ikb \cosh b \\ (b^2 - k^2) \cosh & -(b^2 - k^2) \sinh & 2ikb \sinh b & 2ikb \cosh b \end{bmatrix}$$

thus
$$\begin{bmatrix} a_{12}i & a_{11} & a_{14}i & a_{13} \\ \vdots & \vdots & \vdots & \vdots \\ i & 1 & i & 1 \end{bmatrix} \begin{pmatrix} A_1 \\ A_2 \\ A_3 \\ A_4 \end{pmatrix}$$

$$\begin{bmatrix} 2ika \cosh & -2ika \sinh & (k^2 - b^2) \sinh b & (k^2 - b^2) \cosh b \\ 2ika \cosh & 2ika \sinh & -(k^2 - b^2) \sinh b & (k^2 - b^2) \cosh b \\ (b^2 - k^2) \sinh & (b^2 - k^2) \cosh & 2ikb \cosh b & -2ikb \sinh b \\ -(b^2 - k^2) \sinh & (b^2 - k^2) \cosh & 2ikb \cosh b & 2ikb \sinh b \end{bmatrix} \begin{pmatrix} A_1 \\ A_2 \\ A_3 \\ A_4 \end{pmatrix} = 0$$

-4
$$\begin{bmatrix} 2ika \cosh & -2ika \sinh & (k^2 - b^2) \sinh b & (k^2 - b^2) \cosh b \\ 2ika \cosh & 0 & 0 & (k^2 - b^2) \cosh b \\ (b^2 - k^2) \sinh & (b^2 - k^2) \cosh & 2ikb \cosh b & -2ikb \sinh b \\ 0 & 2(b^2 - k^2) \cosh & 2ikb \cosh b & 0 \end{bmatrix}$$

$$\begin{bmatrix} 0 & -2ika \sinh & (k^2 - b^2) \sinh b & 0 \\ 2ika \cosh & 0 & 0 & (k^2 - b^2) \cosh b \\ (b^2 - k^2) \sinh & 0 & 0 & -2ikb \sinh b \\ 0 & (b^2 - k^2) \cosh & 2ikb \cosh b & 0 \end{bmatrix}$$

$$\begin{bmatrix} 0 & -2ika \sinh & (k^2 - b^2) \sinh b & 0 \\ 2ika \cosh & 0 & 0 & (k^2 - b^2) \cosh b \\ 0 & 0 & 0 & -2ikb \sinh b + \frac{(k^2 - b^2)^2}{2ika \cosh} \sinh \\ 0 & (b^2 - k^2) \cosh & 2ikb \cosh b & 0 \end{bmatrix}$$

-1
$$\begin{bmatrix} 2ika \cosh & 0 & 0 & (k^2 - b^2) \cosh b \\ 0 & -2ika \sinh & (k^2 - b^2) \sinh b & 0 \\ 0 & 0 & 0 & -2ikb \sinh b + \frac{(k^2 - b^2)^2}{2ika \cosh} \sinh \\ 0 & 0 & 2ikb \cosh b - \frac{(k^2 - b^2)^2}{2ika \cosh} \sinh & 0 \end{bmatrix}$$

-1
$$\begin{bmatrix} x & 0 & 0 & 0 \\ 0 & x & 0 & 0 \\ 0 & 0 & x & 0 \\ 0 & 0 & 0 & x \end{bmatrix}$$

$$\therefore \det A = 4k^2 a^2 \cos ah \sin ah$$

$$\det B = \left[\frac{2ikb \cos bh + i(k^2 - b^2)^2 \sin bh \cos ah}{2ika \sin ah} \right] \left[\frac{-2ikb \sin bh + i(k^2 - b^2)^2 \cos bh \sin ah}{2ika \cos ah} \right]$$

$$\frac{\sin bh}{2ka} \left[4k^2 ba + (k^2 - b^2)^2 \frac{\tan bh}{\tan ah} \right] \left[\frac{\sin bh}{2ka} \right] \left[4k^2 ab + (k^2 - b^2)^2 \frac{\tan ah}{\tan bh} \right] = 0$$

$$\Rightarrow \frac{\tan bh}{\tan ah} = - \frac{4k^2 ba}{(k^2 - b^2)^2}$$

$$\Rightarrow \frac{\tan bh}{\tan ah} = \frac{(k^2 - b^2)^2}{4k^2 ab}$$

2.



$$P \leftarrow \boxed{\text{Element}} \rightarrow P + dP$$

$$\Delta V = E \epsilon = AE u' dx$$

$$dP + f(x,t) dx = m \ddot{u}$$

$$F \propto \eta u$$

$$\frac{\partial P}{\partial x} dx + f(x,t) dx = \rho A dx \ddot{u}$$

$$f(x,t) = \eta u$$

$$\therefore E u_{xx} - \rho u_{tt} + \frac{\eta}{A} u = 0$$

$$\therefore \frac{E}{\rho} u_{xx} - u_{tt} + \frac{\eta}{\rho A} u = 0$$

$$\text{if } W = e^{ikx} e^{i\omega t} \quad -\frac{E}{\rho} k^2 + \omega^2 + \frac{\eta}{\rho A} = 0$$

$$\omega = \sqrt{c^2 k^2 - \frac{\eta}{\rho A}}$$

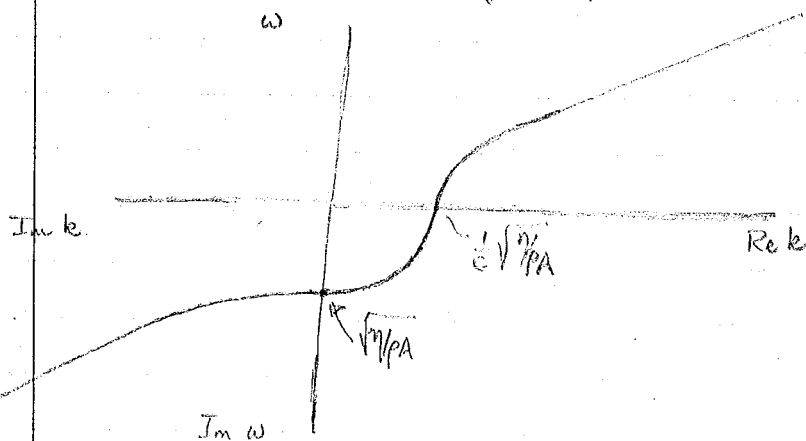
$$\text{w/ } k = \frac{1}{c} \sqrt{\frac{\eta}{\rho A}}$$

$$\therefore \omega^2 = c^2 k^2 - \frac{\eta}{\rho A}$$

$\omega = 0$ no longer harmonic

$\omega \sim ck$ for large k

$$\omega^2 = c^2 k^2$$



$$\text{if } k \ll \frac{1}{c} \sqrt{\frac{\eta}{\rho A}} \quad c^2 k^2$$

$$c^2 k^2 - \frac{\eta}{\rho A}$$

$$\omega = i \sqrt{c^2 k^2 - \frac{\eta}{\rho A}}$$

3.

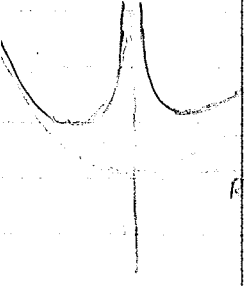


$$EI W'''' + kW + PA \ddot{W} = 0 \quad \text{is DE}$$

now if $w = e^{iqx} e^{i\omega t}$

$$EI q^4 + k + PA \omega^2 = 0$$

$$\therefore EI q^2 + \frac{k}{q^2} - PA \omega^2 = 0 \quad \text{where } \omega = \frac{c}{q}$$



no foundation
Re q

$$c = \sqrt{\frac{EI}{PA} q^2 + \frac{k}{PA q^2}}$$

for very large q $\Rightarrow$ small λ $c \sim q \sqrt{\frac{EI}{PA}}$

for small q $\Rightarrow$ large λ $c \sim \frac{1}{q} \sqrt{\frac{k}{PA}}$

large $\lambda \Rightarrow$ no x dependence

$$\Rightarrow kW + PA \ddot{W} = 0 \quad \text{or } \ddot{W} + \frac{k}{PA} W = 0$$

$$W = f(\frac{c}{q} t), \quad W = \sqrt{\frac{k}{PA}} \quad c = \frac{\omega}{q}$$

$$\frac{EI}{PA} q^4 - \frac{k}{PA} q^2 = 0$$

now $\omega^2 = \frac{EI}{PA} q^4 + \frac{k}{PA}$

$$2\omega \frac{d\omega}{dq} = \frac{4EI}{PA} q^3$$

$$\therefore \frac{d\omega}{dq} = \frac{2EI}{PA} q^3$$

$$\therefore q^4 = \frac{k}{EI}$$

$$q = \sqrt[4]{\frac{k}{EI}}$$

$$q^2 = \sqrt{\frac{k}{EI}}$$

$$\frac{EI}{PA} \cdot \frac{k}{EI} + \frac{k}{PA} = \frac{2EI}{PA} \cdot \frac{k}{EI}$$

for large q $\frac{d\omega}{dq} = c_g \sim \frac{2EI}{PA} q^3 = 2 \sqrt{\frac{EI}{PA}} q$

for $q \rightarrow 0$ $c_g \sim \frac{2EI}{PA} q^3 = \frac{2EI}{\sqrt{k/PA}} q^3 \rightarrow 0$

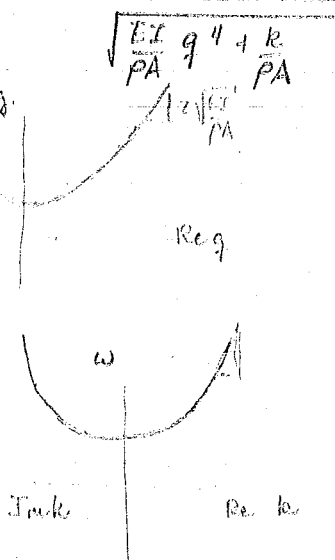
$$\omega = \sqrt{\frac{EI}{PA} q^4 + \frac{k}{PA}}$$

for large q

$$\omega = q^2 \sqrt{\frac{EI}{PA}}$$

for small q

$$\omega = \sqrt{\frac{k}{PA}}$$



c vs k

for first case when we have large q the wave length is so small the body appears infinite and the effect of what goes on the boundary. hence the velocity should be that of a beam freely oscillating, which it is. for small q, large λ the body acts as if it shows no dependence on x a just rigid body motion resisted by a spring. Hence if no x dependence, $kW + PA \ddot{W} = 0$ or $\ddot{W} + \frac{k}{PA} W = 0$ $\omega = \sqrt{\frac{k}{PA}}$ & $c = \frac{\omega}{q} = \frac{1}{q} \sqrt{\frac{k}{PA}}$ As in the freely vibrating case the fact that for large q, c depends on q is incorrect for 2nd case we note that c_g is the velocity with which energy will be propagated hence for long wavelengths the velocity that carries their energy is very low and for the short wavelengths the velocity is a lot higher and hence large

c vs k

This implies that if we suddenly apply a concentrated load, that it will be felt everywhere immediately since the concentrated load contains harmonic components with all orders of wavelength. & for small $\lambda \rightarrow \infty$ wave speed. This is physically unreasonable. ^(also) amounts of energy are carried at the higher wavenumbers (lower frequencies). From the

results of our frequency-wave number plots we found that $\frac{d\omega}{dq} \rightarrow \text{constant}$ for large q which contradicts the results shown here for large q . If we look at the cone for the wave guide problem we obtained hyperbolas for ω versus q now $\frac{d\omega}{dq} = c_g$ are basically the slopes of the asymptotes for large q and hence are virtually constant.

But for ~~small~~ ^{small} q : the hyperbolas are of the form $\omega = \sqrt{\frac{a}{b} q^2 + a^2}$ & for small q

$$\frac{d\omega}{dq} = \frac{\frac{2a^2}{b} q}{\sqrt{\left(\frac{a}{b} q\right)^2 + a^2}} \sim \text{const} \cdot q \quad \text{hence } c_g \rightarrow 0 \text{ as } q \rightarrow 0$$

$$2 \alpha_2 q = 2 \frac{\pi c_2 h}{\pi^2 n^2} q = 2 \frac{c_2 \cdot 2h}{\pi n} q$$

$$\left(\frac{2h}{\pi c_2} \right)^2 \omega^2 + \left(\frac{2h}{\pi} \right)^2 k^2 = n^2$$

where $a = \frac{\pi c_2}{2h} n$
 $b = \frac{\pi}{2h}$

ω vs. q .

for large q $\omega \sim q^2$; hence we can look at an infinite body since λ is small clearly this is like having no foundation but this also leads to an incorrect result since for large q $\omega \sim q$ from the solution of the cone. For the small values of q the beam acts like a rigid body at the end of a spring & hence we find that $\omega = \text{constant}$

if not on elastic foundation

1. First I will write $\Phi(x_1, x_2, t) = f(x_2) e^{i(kx_1 - \omega t)}$ and $\Psi(x_1, x_2, t) = g(x_2) e^{i(kx_1 - \omega t)}$

with $f(x_2) = B_1 \cos ax_2 + B_2 \sin ax_2$ and $g(x_2) = B_3 \cos ax_2 + B_4 \sin ax_2$

and finally $(A_1 + A_2) = B_1$, $(A_1 - A_2)i = B_2$, $(A_3 + A_4) = B_3$, $(A_3 - A_4)i = B_4$.

Thus $u_1 = \phi_{,x_1} + \psi_{,x_2} = (i'kf + g') e^{i(kx_1 - \omega t)}$ and $u_2 = \phi_{,x_2} - \psi_{,x_1} = (f' - i'kg) e^{i(kx_1 - \omega t)}$

Placing this into $\sigma_{ij} = \lambda(u_{k,k})\delta_{ij} + \mu(u_{i,j} + u_{j,i})$, and noting that all we want are the σ_{12} and σ_{22} components for use as boundary conditions at $x_2 = \pm h$, we obtain the following:

$$\sigma_{22} = (\lambda + 2\mu)u_{2,2} + \lambda u_{1,1} = [(\lambda + 2\mu)(-a^2 f - i'kg') + \lambda(-k^2 f + i'kg')] e^{i(kx_1 - \omega t)}$$

$$\sigma_{21} = \mu(u_{2,1} + u_{1,2}) = \left[\mu [2ikf' + (k^2 - b^2)g] e^{i(kx_1 - \omega t)} = \sigma_{21} \right]$$

we've used the fact that $f'' + a^2 f = 0$; $g'' + b^2 g = 0$

we also use the fact that $-[\lambda(a^2 + k^2) + 2\mu a^2] = -(b^2 - k^2)\mu$ to rewrite σ_{22} as

$$\sigma_{22} = -\mu [(b^2 - k^2)f + 2ikg'] e^{i(kx_1 - \omega t)}$$

applying the b.c. at $x_2 = \pm h$ we obtain

$$\begin{bmatrix} -2ika \sin ah & 2ika \cos ah & (k^2 - b^2) \cos bh & (k^2 - b^2) \sin bh \\ 2ika \sin ah & 2ika \cos ah & (k^2 - b^2) \cos bh & -(k^2 - b^2) \sin bh \\ (b^2 - k^2) \cos ah & (b^2 - k^2) \sin ah & -2ikb \sin bh & 2ikb \cos bh \\ (b^2 - k^2) \cos ah & -(b^2 - k^2) \sin ah & 2ikb \sin bh & 2ikb \cos bh \end{bmatrix} \begin{pmatrix} B_1 \\ B_2 \\ B_3 \\ B_4 \end{pmatrix} = \underline{0}$$

we now write this as $A \underline{b} = 0$, where A is the matrix, and resubstitute the $b_i = B_i$ in terms of the A_i 's, where $a_i = A_i$

$$\text{Thus } [A] \underline{b} = ([\underline{C}] + i[\underline{D}]) \underline{a} = 0 \quad \underline{C} = \begin{bmatrix} a_{11} & a_{11} & a_{13} & a_{13} \\ a_{41} & a_{41} & a_{43} & a_{43} \end{bmatrix} \quad \underline{D} = \begin{bmatrix} a_{12} & -a_{12} & a_{14} & -a_{14} \\ a_{42} & -a_{42} & a_{44} & -a_{44} \end{bmatrix}$$

by some matrix reduction techniques we can get this to

$$(\underline{C} + i\underline{D}) \underline{a} = \begin{bmatrix} a_{12}i & a_{11} & a_{14}i & a_{13} \\ a_{42}i & a_{41} & a_{44}i & a_{43} \end{bmatrix} \underline{a} = \underline{E} \underline{a} = 0$$

Thus all we need to do is solve $\det E = 0$ for non-trivial solutions.

Note:

If one solves the above determinant for $A \underline{b} = 0$ & row reduce it to the form $\det \begin{bmatrix} M_1 & M_2 \\ M_3 & M_4 \end{bmatrix} = \det \begin{bmatrix} M_1' & 0 \\ 0 & M_4' \end{bmatrix} = 0$ we will get the symmetric mode. If we row reduce it to $\det \begin{bmatrix} 0 & M_2' \\ M_3' & 0 \end{bmatrix} = 0$ we will get the anti-symmetric mode.

Thus

$$\begin{bmatrix} -2ka \cos ah & -2ika \sin ah & i(k^2 - b^2) \sin bh & (k^2 - b^2) \cos bh \\ -2ka \cos ah & 2ika \sin ah & -i(k^2 - b^2) \sin bh & (k^2 - b^2) \cos bh \\ i(b^2 - k^2) \sin ah & (b^2 - k^2) \cos ah & -2kb \cos bh & -2ikb \sin bh \\ -i(b^2 - k^2) \sin ah & (b^2 - k^2) \cos ah & -2kb \cos bh & 2ikb \sin bh \end{bmatrix} \Delta = 0$$

for non trivial solns the determinant = 0. This matrix, since a & b are implicit fns of k and ω , is an implicit relation between k and ω , hence $\frac{d\omega}{dk}$ and k . Thus this is a dispersion relation.

by row reducing this matrix we can get the determinant in the final form.

$$\det \begin{bmatrix} 2ika \cos ah & 0 & 0 & 0 \\ 0 & -2ika \sin ah & 0 & 0 \\ 0 & 0 & Y & 0 \\ 0 & 0 & 0 & X \end{bmatrix} = \Delta \det \begin{bmatrix} S & 0 \\ 0 & T \end{bmatrix} = \det S \cdot \det T$$

$$\text{where } Y = 2ikb \cos bh - (k^2 - b^2)^2 \sin bh \cos ah \quad X = -2ikb \sin bh + (k^2 - b^2)^2 \cos bh \sin ah$$

$$\text{or } \left[\frac{4k^2 ba}{(k^2 - b^2)^2} + \frac{\tan bh}{\tan ah} \right] \left(\frac{i \cos bh (k^2 - b^2)^2}{2ka} \right) = Y; \quad \left[\frac{\tan bh}{\tan ah} + \frac{(k^2 - b^2)^2}{4k^2 ba} \right] \left(\frac{2kb \cos bh \tan ah}{i} \right) = X$$

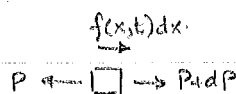
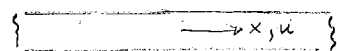
Thus the determinant can be written $\det S \cdot \det T = 0$

Thus $4k^2 a^2 \cos ah \sin ah X Y = 0$ either $k=0$ or $ah = n\pi$ or $k = \pm b$ or $bh = \frac{n\pi}{2}$ (odd) or $ah = m\pi$ (m even) or $Y=0$ or $X=0$; X and $Y=0$ turn out to be the antisymmetric and symmetric modes respectively.

$$\text{Thus } \frac{\tan bh}{\tan ah} = - \frac{4k^2 ba}{(k^2 - b^2)^2} \text{ for symmetric modes}$$

$$\frac{\tan bh}{\tan ah} = - \frac{(k^2 - b^2)^2}{4k^2 ba} \text{ for antisymmetric modes.}$$

2.



and $f(x,t) = \eta u(x,t)$

thus $dP + f(x,t)dx = \rho A dx \ddot{u}$

$dP = \frac{\partial P}{\partial x} dx = (AEu')' dx$; we note that $f(x,t)$ is a force/length.

$$\therefore (AEu')' + f(x,t) - \rho A \ddot{u} = 0$$

if A, E are not functions of x we can rewrite this as $\left| c^2 u'' + \frac{\eta}{\rho A} u - \ddot{u} = 0 \right| \quad c^2 = E/\rho$

if $We^{ikx} e^{i\omega t} = u(x,t)$ then $-k^2 c^2 u + \frac{\eta}{\rho A} u + \omega^2 u = 0$

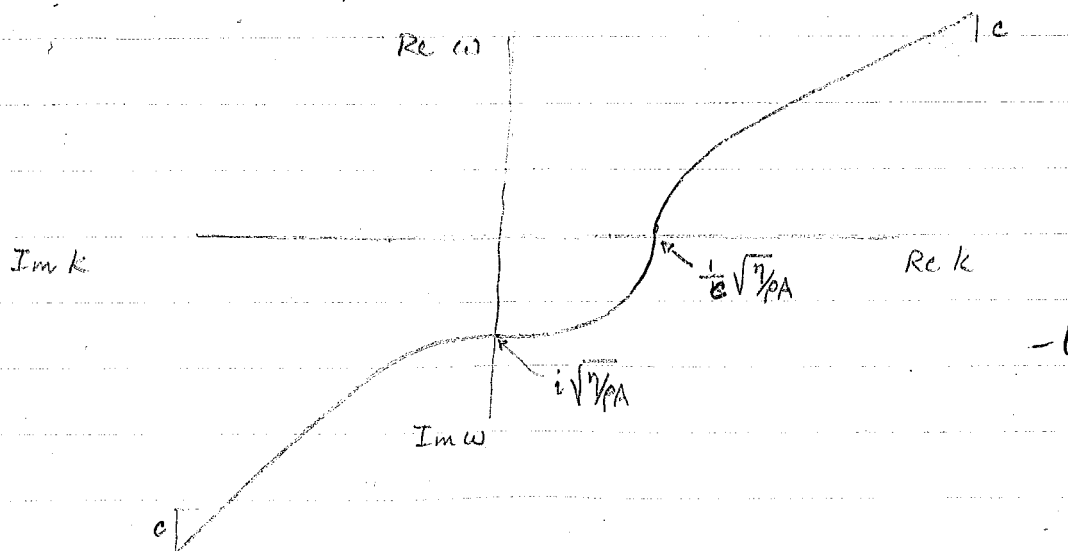
and our frequency eqn is

$$\omega^2 = k^2 c^2 - \frac{\eta}{\rho A} \quad \text{thus} \quad \omega = k \sqrt{c^2 - \frac{\eta}{\rho A k^2}}$$

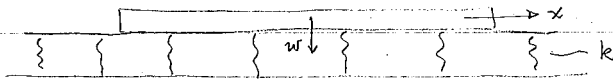
$$\omega = 0 \Rightarrow k = \frac{1}{c} \sqrt{\frac{\eta}{\rho A}}; \quad \text{as } k \rightarrow \infty \quad \omega \sim ck; \quad \text{as } k \rightarrow 0 \quad \omega \rightarrow i \sqrt{\frac{\eta}{\rho A}}$$

$$\text{if } k \text{ is imaginary } \omega = ik \sqrt{c^2 + \frac{\eta}{\rho A k^2}} \quad \text{or } \omega = i \sqrt{c^2 k^2 + \frac{\eta}{\rho A}}$$

$$\omega = 0 \Rightarrow c = \frac{i}{k} \sqrt{\frac{\eta}{\rho A}}; \quad \text{as } k \rightarrow \infty \quad \omega \sim ikc; \quad \text{as } k \rightarrow 0 \quad \omega \rightarrow i \sqrt{\frac{\eta}{\rho A}}$$



3.



The DE is $EI W'''' + kW + PA W'' = 0$; assume a wave $w(x,t) = W e^{iqx} e^{i\omega t}$

$$\therefore (EI q^4 + k - PA q^2) W = 0; \text{ let } \frac{\omega}{q} = c \text{ then}$$

$$c^2 = \frac{EI}{PA} q^2 + \frac{k}{PA} \frac{1}{q^2} \quad \text{or} \quad c = \sqrt{\frac{EI}{PA} q^2 + \frac{k}{PA} \frac{1}{q^2}}$$

also $\omega = \left(\frac{EI}{PA} q^4 + \frac{k}{PA} \right)^{1/2}$

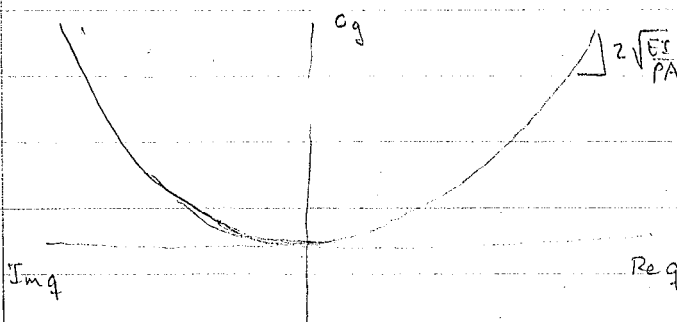
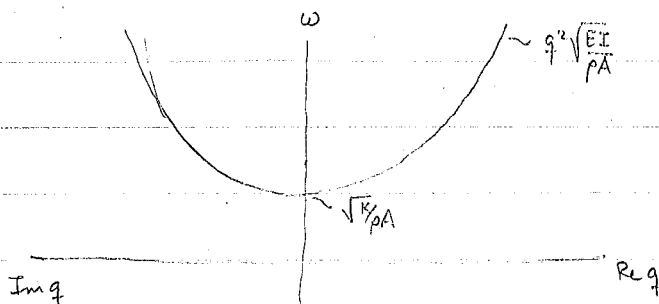
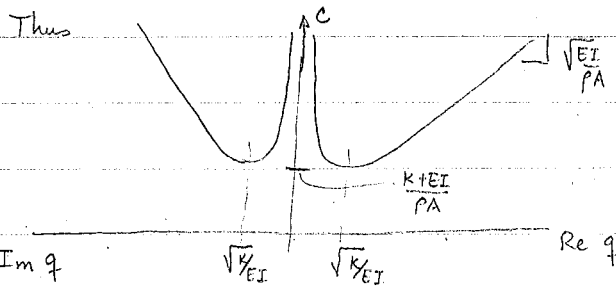
→ for very large q (small λ) $c \sim q \sqrt{\frac{EI}{PA}}$; for small q (large λ) $c \sim \frac{1}{q} \sqrt{\frac{k}{PA}}$

if we have imaginary q we get the same frequency equation and the same phase velocity equation and the same limiting values.

→ for very large q (small λ) $\omega \sim q^2 \sqrt{\frac{EI}{PA}}$; for small q (large λ) $\omega \sim \sqrt{\frac{k}{PA}}$

To get the group velocity $c_g = \frac{d\omega}{dk} = \frac{q \frac{EI}{PA} q^3}{2 \left(\frac{EI}{PA} q^4 + \frac{k}{PA} \right)^{1/2}} = \frac{2 \frac{EI}{PA} q^3}{\left(\frac{EI}{PA} q^4 + \frac{k}{PA} \right)^{1/2}}$

→ for very large q (small λ) $c_g \sim 2 \sqrt{\frac{EI}{PA}} q$; for small q (large λ) $c_g \sim \frac{2 EI}{\sqrt{PAk}} q^3$



Discussion

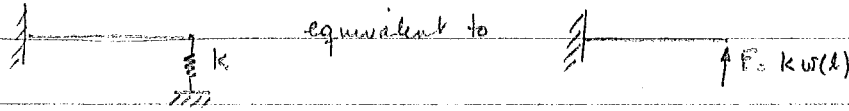
c vs. q : When we have large q , the wavelength is so small the body appears to be infinite and the effect of the boundaries aren't felt; hence the velocity should be that of the infinite body i.e. $\sqrt{E/\rho} = c_1$ and $\sqrt{G/\rho} = c_2$. We note that this is not the results shown; hence the theory is invalid for large q . For small q , large λ , the body acts as if it is independent of x or just a rigid body oscillating at the end of a spring. Thus $w(x,t) = \bar{w}(t)$ which is a fn of $\cos \omega t$ & $\sin \omega t$ where $\omega = \sqrt{K/\rho A}$ and $\frac{\omega}{q} = c = \frac{1}{q} \sqrt{K/\rho A}$ and $c_g = \frac{dw}{dq} = 0$. Our theory is good here since we find that $c \sim \frac{1}{q} \sqrt{K/\rho A}$ as $q \rightarrow 0$.

c_g vs. q : We note that c_g is the velocity with which the energy of the system will be propagated; hence for long wavelengths the velocity that carries the energy is very low and for low wavelengths the velocity that carries the energy is very high; hence large amounts of energy are carried at the high wavenumbers (lower frequencies). This implies that if we suddenly apply a concentrated load, that it will be felt everywhere immediately, since the concentrated load has a fourier expansion containing harmonic components with all orders of wavelength. Thus for small λ (large q) we will have ∞ group speed; This is physically unreasonable. Also from the results of our frequency - wavenumber plots for the ∞ body (approximating our case when q is large) we found that $\frac{dw}{dq}$ tends to a constant for large q , which contradicts the results shown for large q . This is so because if we look at the cone for the waveguide problem, we obtained hyperbolas for ω versus q . Now $\frac{dw}{dq} = c_g$ are basically the slope of the asymptotes for large q and hence are virtually constant.

ω vs. q : for large q we found that $\omega \sim q^2$. This is an incorrect result since for large q (small λ) the body appears to be ∞ . From the ω vs. q plot of the cone $\omega \sim q$ [Remember that $\Omega^2 = n^2 + \xi^2 \Rightarrow \omega = \frac{\pi c}{2h} \sqrt{(\frac{2h}{\pi})^2 q^2 + n^2} \sim c q$].

For small values of q , the beam acts like a rigid body at the end of a spring & hence we find that $\omega = \text{constant}$.

4.



Thus our DE is $EI w^{IV} + \rho A \ddot{w} = 0$ w/ BC $w(0) = w'(0) = w''(l) = 0$ and
 $-EI w''' = V = -F = -k w(l)$

let $w(x, t) = \bar{w}(x) e^{i\omega t}$ then $EI \bar{w}^{IV} - \omega^2 \bar{w} = 0$
 with BC / $\bar{w}(0) = \bar{w}'(0) = 0$ $\bar{w}''(l) = 0$ and $EI \bar{w}'''(l) = k \bar{w}(l)$

let $\omega^2 \frac{\rho A}{EI} = \lambda^4$ then

$$\bar{w}(x) = C_1 \sin \lambda x + C_2 \cos \lambda x + C_3 \sinh \lambda x + C_4 \cosh \lambda x$$

from $\bar{w}(0) = 0 \Rightarrow C_2 + C_4 = 0$

$\bar{w}'(0) = 0 \Rightarrow C_1 + C_3 = 0$

$$\left. \begin{array}{l} \bar{w}(0) = 0 \Rightarrow C_2 + C_4 = 0 \\ \bar{w}'(0) = 0 \Rightarrow C_1 + C_3 = 0 \end{array} \right\} \bar{w}(x) = C_1 (\sin \lambda x - \sinh \lambda x) + C_2 (\cos \lambda x - \cosh \lambda x)$$

Applying the last two BC we get

$$\left[\begin{array}{cc} \sin \lambda l + \sinh \lambda l & \cosh \lambda l + \cos \lambda l \\ -\lambda^3 (\cos \lambda l + \cosh \lambda l) - \frac{k}{EI} (\sin \lambda l - \sinh \lambda l) & \lambda^3 (\sin \lambda l - \sinh \lambda l) - \frac{k}{EI} (\cos \lambda l - \cosh \lambda l) \end{array} \right] \begin{pmatrix} C_1 \\ C_2 \end{pmatrix} = 0$$

$\therefore$ det matrix $= 0 \Rightarrow$

$$\left| \lambda^3 (1 + \cosh \lambda l \cos \lambda l) + \frac{k}{EI} (\cosh \lambda l \sin \lambda l - \sinh \lambda l \cos \lambda l) \right| = 0 \quad \checkmark$$

since λ is a fn of ω , this is the frequency equation

if $k \rightarrow 0$ then for nontrivial solutions $\cosh \lambda l \cos \lambda l = -1$. This is the free vibrations equation for a cantilever beam. ✓

if $k \rightarrow \infty$ then for nontrivial solutions $\cosh \lambda l \sin \lambda l - \sinh \lambda l \cos \lambda l = 0$. This is the free vibrations equation for a simply supported - clamped beam.

for high frequencies, hence large λ , $\cosh \lambda l \sim \frac{e^{\lambda l}}{2}$ & $\sinh \lambda l \sim \frac{e^{\lambda l}}{2}$ then
 our frequency equation $\sim \left[\lambda^3 \cos \lambda l + \frac{k}{EI} (\sin \lambda l - \cos \lambda l) \right] \frac{e^{\lambda l}}{2} = 0$

$$\text{or } \left(\lambda^3 - \frac{k}{EI} \right) \cos \lambda l + \frac{k}{EI} \sin \lambda l = 0 \Rightarrow \tan \lambda l = 1 - \frac{\lambda^3 EI}{k}$$

as $k \rightarrow \infty$ $\tan \lambda l \rightarrow 1 \therefore \lambda l \sim \frac{4n+1}{4} \pi$ n very large

or since $\lambda = \left(\frac{\omega^2}{\rho^2 r^2} \right)^{1/4}$ where r is the radius of gyration $\omega \sim \sqrt{\frac{EI}{\rho A}} \left(\frac{4n+1}{4} \frac{\pi}{l} \right)^2$

as $k \rightarrow 0$ $\tan \lambda l \rightarrow -\infty \therefore \lambda l \sim -\frac{2n+1}{2} \pi$ n very large then
 $\omega \sim \sqrt{\frac{EI}{\rho A}} \left(\frac{2n+1}{2} \frac{\pi}{l} \right)^2$

4.

4

$$\begin{aligned}
 w(0) &= 0 & \uparrow k w(L) &= V & \therefore -EI w'''(L) &= V = -k w(L) \\
 w'(0) &= 0 & w''(L) &= 0
 \end{aligned}$$

$$\therefore EI w^{IV} + \rho A \ddot{w} = 0 \quad \text{or} \quad EI w^{IV} + \omega^2 \ddot{w} = 0$$

$$\begin{aligned}
 w/BC \quad w(0) &= 0 & EI w'''(L) &= +k w(L) \\
 w'(0) &= 0 & w''(L) &= 0
 \end{aligned}$$

$$\text{let } w(x,t) = \bar{w}(x) e^{i\omega t} \quad \text{then} \quad EI \bar{w}^{IV} - \omega^2 \bar{w} = 0$$

$$\therefore \bar{w}(0) = 0 \quad EI \bar{w}'''(L) = \frac{\rho A}{EI} \bar{w}(L)$$

$$\bar{w}'(0) = 0 \quad \bar{w}''(L) = 0 \quad \text{let } \omega^2 \frac{\rho A}{EI} = \lambda^4$$

$$\therefore \bar{w} = c_1 \sin \lambda x + c_2 \cos \lambda x + c_3 \sinh \lambda x + c_4 \cosh \lambda x$$

$$\bar{w}(0) = 0 \Rightarrow c_2 + c_4 = 0$$

$$\bar{w}'(0) = 0 \Rightarrow c_1 + c_3 = 0$$

$$\left. \begin{aligned} c_2 + c_4 &= 0 \\ c_1 + c_3 &= 0 \end{aligned} \right\} \bar{w} = c_1 (\sin \lambda x - \sinh \lambda x) + c_2 (\cos \lambda x - \cosh \lambda x)$$

$$\bar{w}' = \lambda [c_1 (\cos \lambda x - \cosh \lambda x) + c_2 (-\sin \lambda x - \sinh \lambda x)]$$

$$\bar{w}'' = \lambda^2 [c_1 (-\sin \lambda x - \sinh \lambda x) + c_2 (-\cos \lambda x - \cosh \lambda x)]$$

$$\bar{w}''' = \lambda^3 [c_1 (-\cos \lambda x - \cosh \lambda x) + c_2 (\sin \lambda x - \sinh \lambda x)]$$

$$\begin{aligned}
 \therefore \text{from 3rd BC} \quad \lambda^3 [c_1 (\cos \lambda L + \cosh \lambda L) + c_2 (\sin \lambda L - \sinh \lambda L)] &= \frac{k}{EI} [c_1 (\sin \lambda L - \sinh \lambda L) \\
 &+ c_2 (\cos \lambda L - \cosh \lambda L)] \\
 \bar{w}'''(L) = 0 &= +c_1 [\sin \lambda L + \sinh \lambda L] + c_2 (\cos \lambda L + \cosh \lambda L) - \frac{k}{EI} (\cos \lambda L - \cosh \lambda L)
 \end{aligned}$$

$$\therefore \begin{bmatrix} \sin \lambda L + \sinh \lambda L & \cos \lambda L + \cosh \lambda L \\ -\lambda^3 (\cos \lambda L + \cosh \lambda L) - \frac{k}{EI} (\sin \lambda L - \sinh \lambda L) \end{bmatrix} \begin{bmatrix} c_1 \\ c_2 \end{bmatrix} = 0$$

$$\lambda^3 (\sin \lambda L - \sinh \lambda L) - \frac{k}{EI} (\cos \lambda L - \cosh \lambda L)$$

$$\begin{aligned}
 \lambda^3 [(\sin \lambda L - \sinh \lambda L)(\sin \lambda L + \sinh \lambda L)] - \frac{k}{EI} (\cos \lambda L - \cosh \lambda L)(\sin \lambda L + \sinh \lambda L) &+ \lambda^3 (\cos \lambda L + \cosh \lambda L) \\
 + \frac{k}{EI} (\sin \lambda L - \sinh \lambda L)(\cos \lambda L + \cosh \lambda L) &= 0
 \end{aligned}$$

for non-trivial soln.

$$\begin{aligned}
 \lambda^3 [\sin^2 \lambda L - \sinh^2 \lambda L] + \lambda^3 [\cos^2 \lambda L + \cosh^2 \lambda L] - \frac{k}{EI} [c_1 + c_3 - c_1 - c_3 - \sinh \lambda L - \cosh \lambda L + \sinh \lambda L + \cosh \lambda L] &= 0 \\
 \lambda^3 [\sin^2 \lambda L - \sinh^2 \lambda L + \cos^2 \lambda L + 2 \cosh \lambda L \cos \lambda L + \cosh^2 \lambda L] - \frac{k}{EI} [2 c_2 \sinh \lambda L + 2 c_4 \cosh \lambda L] &= 0 \\
 \lambda^3 (2 + 2 \cosh \lambda L \cos \lambda L) + \frac{2k}{EI} [\cosh \lambda L \sin \lambda L - \sinh \lambda L \cos \lambda L] &= 0
 \end{aligned}$$

if $k \rightarrow 0$ then $\cosh \lambda L \cos \lambda L = -1$ which is the free vib eq for a cantilever beam.
 if $k \rightarrow \infty$ $\cosh \lambda L \sin \lambda L - \sinh \lambda L \cos \lambda L = 0$ which is the free vib prob of a

Suppose we write $\frac{\omega^2}{c^2 r^2} = \lambda^4$ where r is the radius of gyration $\lambda^4 = \left(\frac{\omega^2}{c^2 r^2}\right)^{1/4}$



since $\lambda \sim \omega^{1/2}$ for high frequencies $\cosh \lambda l \sim \frac{e^{\lambda l}}{2}$ $\sinh \lambda l \sim \frac{e^{\lambda l}}{2}$

$$\therefore \lambda^3 \left(2 + \frac{2e^{\lambda l}}{2} \cosh \lambda l\right) + \frac{2k}{EI} \frac{e^{\lambda l}}{2} [\sinh \lambda l - \cosh \lambda l] = 0$$

$$\text{if } \sigma \sim \cosh \lambda l \left(\lambda^3 - \frac{k}{EI}\right) + \frac{k}{EI} \sinh \lambda l = 0 \quad \text{or} \quad \frac{-\lambda^3 + k/EI}{k/EI} = \tanh \lambda l = 1 - \frac{\lambda^3 EI}{k}$$

$$\text{if } k \rightarrow 0 \quad 2\lambda^3 + e^{\lambda l} \cosh \lambda l = 0 \quad \text{or} \quad \lambda l \sim \frac{n\pi}{2} \quad n \text{ odd} \quad \therefore \lambda^4 \sim \left(\frac{n\pi}{2l}\right)^4 = \frac{\omega^2}{c^2 r^2}$$

$$\therefore \frac{EI}{\rho A} \left(\frac{n\pi}{2l}\right)^4 = \omega^2 \quad \text{or} \quad \omega \sim \sqrt{\frac{EI}{\rho A}} \left(\frac{n\pi}{4l}\right)^2 \quad \text{high frequencies as } k \rightarrow 0$$

for $k \rightarrow 0$ we have from free vib $\cosh \lambda l \cosh \lambda l = -1$ the following $2e^{\lambda l/2} [e^{i\lambda l} + e^{-i\lambda l}] = -1$
 or $\cosh \lambda l = \frac{-1}{\cosh \lambda l} \rightarrow 0 \quad \lambda l = \frac{n\pi}{2} \quad n \text{ odd}$ as shown above.

for $k \rightarrow \infty$ & large ω we have $\frac{e^{\lambda l}}{2} [\sinh \lambda l - \cosh \lambda l] = 0 \quad \therefore \sinh \lambda l = \cosh \lambda l$
 $\lambda l \sim \frac{\pi}{4}, \frac{5\pi}{4}, \frac{9\pi}{4}, \frac{13\pi}{4}, \dots \quad \lambda l = \frac{(4n+1)\pi}{4} \quad \therefore \lambda \sim \frac{(4n+1)\pi}{4l} \text{ and } \lambda^4 \sim \left(\frac{(4n+1)\pi}{4l}\right)^4$

$$\therefore \omega^2 \sim \frac{EI}{\rho A} \left(\frac{(4n+1)\pi}{4l}\right)^4 \quad \therefore \omega \sim \sqrt{\frac{EI}{\rho A}} \left(\frac{(4n+1)\pi}{4l}\right)^2$$

$$\sim \frac{e^{\lambda l}}{2} \left[\lambda^3 \cosh \lambda l + \frac{k}{EI} \sinh \lambda l - \frac{k}{EI} \cosh \lambda l \right] = 0$$

$$- \cosh \lambda l \left(\frac{k}{EI} - \lambda^3 \right) + \frac{k}{EI} \sinh \lambda l = 0 \Rightarrow \tanh \lambda l = 1 - \frac{\lambda^3 EI}{k}$$

for large λ ~~λl is multiple~~ and $k \rightarrow \infty$ $\tanh \lambda l \rightarrow 1$

or λl must be a multiple $\sim \frac{(4n+1)\pi}{4}$

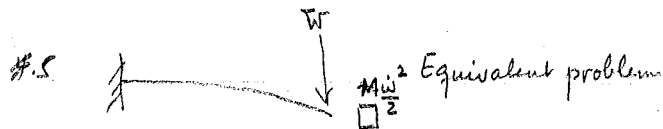
for large λ as $k \rightarrow 0$ $\tanh \lambda l \rightarrow -\infty \quad \therefore \lambda l \sim -\frac{n\pi}{2} \quad n \text{ odd}$

$$\lambda^2 = \frac{n^2 \pi^2}{2l^2} = \omega \sqrt{\frac{EI}{\rho A}}$$

$$\omega = \sqrt{\frac{EI}{\rho A}} \left(\frac{n\pi}{2l}\right)^{3/2} \quad n \text{ odd}$$

$$\lambda^2 = \omega \sqrt{\frac{EI}{\rho A}} = \left(\frac{(4n+1)\pi}{4l}\right)^2$$

$$\omega = \sqrt{\frac{EI}{\rho A}} \left(\frac{(4n+1)\pi}{4l}\right)^{3/2}$$



if we solve static deflection problem for force W on end $w/ EI \bar{w}^{IV}(x) = 0$
and $\bar{w}(0) = \bar{w}'(0) = \bar{w}''(l) = 0$ and $-EI \bar{w}'''(l) = W$ we find that

$$\bar{w}(x) = \frac{Wl^3}{6EI} \left[3\left(\frac{x}{l}\right)^2 - \left(\frac{x}{l}\right)^3 \right]$$

$\therefore$ let $w(x,t) = \bar{w}(x) A \sin \omega t$ then

$$KE = \int_0^l \frac{1}{2} m \dot{w}(x,t)^2 dx + \frac{1}{2} M \dot{w}^2(l,t) \quad PE = \int_0^l \frac{1}{2} EI w''(x,t)^2 dx = \frac{1}{2} k_c w(l,t)^2$$

$m = \rho A l \quad M = \frac{Wl}{g}$

where k_c is an equivalent spring constant. since $\bar{w}(l) = \frac{Wl^3}{3EI}$ we can then say that $W = k_c \bar{w}(l) \therefore k_c = \frac{3EI}{l^3}$

$$\begin{aligned} \therefore KE &= \frac{1}{2} m \omega^2 \cos^2 \omega t \int_0^l \bar{w}(x)^2 dx + \frac{1}{2} M \omega^2 \cos^2 \omega t \bar{w}(l)^2 \\ &= \frac{\omega^2 \cos^2 \omega t}{2} \left[\frac{m}{l} \int_0^l \bar{w}(x)^2 dx + M \bar{w}(l)^2 \right] \end{aligned}$$

$$\begin{aligned} \int_0^l \bar{w}(x)^2 dx &= l \int_0^1 \bar{w}(\bar{x})^2 d\bar{x} \quad \text{where } \bar{x} = x/l \quad \therefore \int_0^l \bar{w}(x)^2 dx = l \frac{W^2 l^6}{36EI^2} \left\{ \frac{9}{5} \bar{x}^5 - \bar{x}^6 + \frac{\bar{x}^7}{7} \right\}_0^1 \\ &= l \frac{W^2 l^6}{36EI^2} \cdot \frac{33}{35} \end{aligned}$$

or in terms of the $\bar{w}(l)$ $\int_0^l \bar{w}(x)^2 dx = \frac{33}{140} l \bar{w}(l)^2$

$$\therefore KE = \frac{\omega^2 \cos^2 \omega t}{2} \left[\frac{m l}{l} \cdot \frac{33}{140} \bar{w}(l)^2 + M \bar{w}(l)^2 \right] = \frac{\omega^2 \cos^2 \omega t}{2} \left[M + \frac{33}{140} m \right] \bar{w}(l)^2$$

$$PE = \frac{1}{2} k_c w(l,t)^2 = \frac{1}{2} \left(\frac{3EI}{l^3} \right) \bar{w}(l)^2 \sin^2 \omega t$$

since $KE_{\max} = PE_{\max} \Rightarrow \frac{\omega^2}{2} \left[M + \frac{33}{140} m \right] \bar{w}(l)^2 = \frac{1}{2} \left(\frac{3EI}{l^3} \right) \bar{w}(l)^2$

thus $\omega^2 = \frac{3EI}{\left(M + \frac{33}{140} m \right) l^3}$ where $m = \rho A l$

when $M \rightarrow 0 \Rightarrow W = 0$ hence $\omega^2 = \frac{420}{33} \frac{EI}{m l^3}$ which is the result for the cantilever beam