## Chapter 3 (Cont.): Water Flow in Pipe Systems

## Review - Key Hydraulic Equations (1 of 2)

Review from the last two classes.
Mean Velocity: $\quad V=\frac{Q}{A}$
Continuity:

$$
\mathrm{A}_{1}\left(\mathrm{~V}_{1}\right)=\mathrm{A}_{2}\left(\mathrm{~V}_{2}\right)
$$

Momentum:

$$
\sum F=\rho Q\left(V_{2}-V_{1}\right) \quad \text { (or Impulse-Momentum) }
$$

Bernoulli:

$$
\begin{aligned}
& h_{1}+\frac{p_{1}}{\gamma}+\frac{v_{1}^{2}}{2 g}=h_{2}+\frac{p_{2}}{\gamma}+\frac{v_{2}^{2}}{2 g} \\
& h_{1}+\frac{p_{1}}{\gamma}+\frac{v_{1}^{2}}{2 g}=h_{2}+\frac{p_{2}}{\gamma}+\frac{v_{2}^{2}}{2 g}+h_{L}
\end{aligned}
$$

## Energy:

## Review - Key Hydraulic Equations (2 of 2)

Darcy-Weisbach: $h_{f}=f\left(\frac{L}{D}\right)\left(\frac{V^{2}}{2 g}\right) \rightarrow f:$ Moody Diagram ${ }^{\text {. }}$
You can solve most pipe flow problems with these equation's!
*Note: $\frac{e}{D}=$ relative roughness; $\quad N_{R}=\frac{(D V \rho)}{\mu}=\frac{(V D)}{v}$

## Friction Factors for Flow in Pipes: The Moody Diagram



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## Friction Factors for Various Types of Flows (Review)

The Darcy-Weisbach Equation

$$
h_{f}=f(L / D)\left(V^{2} / 2 g\right)
$$

Determination of the Friction Factor

- Laminar Flow:
- Turbulent Flow:
- Complete Turbulence:
- Turb. (smooth pipe):
"f"requires $\mathrm{N}_{\mathrm{R}}$
"f"requires $N_{R}$ and (e/D)
" f "requires(e/D)
"f"requires $N_{R}$


## Other Friction Loss Formulas

- Background: Popular formulas based on experiments.
- Empirical formulas - not dimensionally consistent (must use units established for formulas in experiments).
- Applicable only to conditions and ranges of experiments.


What are the vertical tubes? What do they measure? Sketch the HGL.

## Manning's Equation

$$
V=\left(\frac{1.49}{n}\right) R_{h}^{2 / 3} S^{1 / 2}
$$

Define the variables? $R_{h}=\frac{A}{P}$ Where is $h_{f}$ ?
Is the equation dimensionally consistent?
$n \rightarrow$ Table 3.3 see Slide 13 based on pipe material.
(" $n$ " is between 0.01 and 0.025 for most pipes)


Note: This equation is often used for open channel flow.

## Table 3.3 Manning's Roughness Coefficient, $n$, for Pipe Flows

| Type of Pipe | Manning's $\boldsymbol{n}$ Min. | Manning's $\boldsymbol{n}$ Max. |
| :--- | :---: | :---: |
| Brass | 0.009 | 0.013 |
| Cast iron | 0.011 | 0.015 |
| Cement mortar surfaces | 0.011 | 0.015 |
| Cement rubble surfaces | 0.017 | 0.030 |
| Concrete, precast | 0.011 | 0.015 |
| Copper | 0.009 | 0.013 |
| Corrugated metal (CMP) | 0.020 | 0.024 |
| Ductile iron (cement mortar lined) | 0.011 | 0.013 |
| Glass | 0.009 | 0.013 |
| High-density polyethylene (HDPE) | 0.009 | 0.011 |
| Polyvinyl chloride (PVC) | 0.009 | 0.011 |
| Steel, commercial | 0.010 | 0.012 |
| Steel, riveted | 0.017 | 0.020 |
| Vitrified sewer pipe | 0.010 | 0.017 |
| Wrought iron | 0.012 | 0.017 |

## The Hazen-Williams Formula

$$
\mathrm{V}=1.318 \mathrm{C}_{\mathrm{HW}} \mathrm{R}_{\mathrm{h}}^{0.63} \mathrm{~S}^{0.54}
$$

Define the variables. Where is $h_{f}$ ? Units?
$\mathrm{C}_{\mathrm{HW}} \rightarrow$ Table 3.2 see Slide 15 based on pipe material (usually between 100 and 150 except very old pipes!)

Pipe cleaning and lining projects will increase pipe flow and pressure. Homework Problems:


Pipe Tuberculation and Lining http://www.ci.wilmington.de.us

## Table 3.2 Hazen-Williams Coefficient, CH $_{W}$, for Different Types of Pipes

| Pipe Materials | $\mathbf{C H}_{w}$ |
| :--- | ---: |
| Brass | $130-140$ |
| Cast iron (common in older <br> water lines) <br> New, unlined | 130 |
| 10-year-old | $107-113$ |
| 20-year-old | $89-100$ |
| 30-year-old | $75-90$ |
| 40-year-old | $64-83$ |
| Concrete or concrete lined | 140 |
| Smooth | 120 |
| Average | 100 |
| Rough | $130-140$ |
| Copper | 140 |
| Ductile iron (cement mortar <br> lined) |  |


| Pipe Materials | $\mathbf{C H}_{\boldsymbol{w}}$ |
| :--- | :---: |
| Glass | 140 |
| High-density polyethylene <br> (HDPE) | 150 |
| Plastic | $130-150$ |
| Polyvinyl chloride (P V C) | 150 |
| Steel <br> Commercial | $140-150$ |
| Riveted | $90-110$ |
| Welded (seamless) | 100 |
| Vitrified clay | 110 |

## Pipe Flow: Circular Culvert



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## Fundamentals of Hydraulic Engineering Systems

Fifth Edition


## Chapter 3d

## Water Flow in Pipes

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## In-Class Siphon Experiment (1 of 4)

## Bernoulli's Equation

Let's test the validity of the Bernoulli Equation.
Does $\mathbf{Q}_{\mathrm{B}}($ Bernoulli $)=\mathbf{Q}_{\mathrm{m}}($ measured $)$ ?

## Questions:

1. How would we obtain the experimental flow rate?

$$
\mathbf{Q}=\frac{\text { Volume }}{\text { time }}
$$



## In-Class Siphon Experiment (2 of 4)

## Bernoulli's Equation

2. How would we obtain the Bernoulli flow rate?

Energy balance between 2 points
3. What 2 points to find Q ? Resulting eq'n? Data needed?

$$
h_{1}+\frac{P_{1}}{\gamma}+\frac{\mathrm{V}_{1}^{2}}{2 g}=h_{2}+\frac{\mathrm{P}_{2}}{\gamma}+\frac{\mathrm{V}_{2}^{2}}{2 g} \rightarrow h_{1}-h_{2}=h=\frac{\mathrm{V}_{2}^{2}}{2 g}
$$

## In-Class Siphon Experiment (3 of 4)

Perform the experiment and compare results.
Does $\mathbf{Q}_{\mathrm{B}}($ Bernoulli $)=\mathbf{Q}_{\mathrm{m}}($ measured $)$ ?

$$
\mathrm{Q}_{\mathrm{m}}=\frac{2 \text { litres }}{(75 \mathrm{sec})}=0.0267 \mathrm{~L} / \mathrm{s}
$$

Bernoulli Calculations:

$$
\begin{aligned}
& \mathrm{h}=0.6 \mathrm{~m}=\frac{\mathrm{V}^{2}}{2 \mathrm{~g}} ; \mathrm{V}=3.43 \mathrm{~m} / \mathrm{sec} ; \\
& \mathbf{Q}_{\mathrm{B}}=\mathrm{V} . \mathrm{A}=\mathbf{0 . 0 9 7 0 \mathrm { L } / \mathbf { ~ s e c }}
\end{aligned}
$$



Why don't the flow rates compare closely? Perform the calculations a second time with the modifications* you think are necessary.
*Include friction loss in the analysis.

## In-Class Siphon Experiment (4 of 4)

Energy Equation Calculations:

$$
h=\frac{V^{2}}{2 g}+\frac{f(L / D) V^{2}}{2 g}=\frac{V^{2}}{2 g}[1+f(L / D)]
$$

Moody Diagram for f (next slide):

$$
\begin{aligned}
& \frac{e}{D}=\frac{0.0015}{6}=0.00025 ; \text { w/measured } \\
& V ® N_{R}=\frac{0.863(0.006)}{1^{\prime} 10^{-6}}=5,200
\end{aligned}
$$



$$
h=0.6=\frac{V^{2}}{2 g[1+\underline{0.038(1.56 / 0.006)]}}
$$

$$
\mathrm{V}=1.05 \mathrm{~m} / \mathrm{s} ; \mathrm{Q}_{\mathrm{m}}=\mathrm{AV}=0.0297 \mathrm{~L} / \mathrm{s}
$$

Better!! Can we improve the results?

* Include entrance loss: Q = 0.0281.

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## Pipe Flow Problems (Iterative Solution)

Example Problems - Solve on White Board
Determine the flow rate ( $\mathrm{L} / \mathrm{sec}$ ) in a 4-cm-diameter copper pipe. The pressure at point $A$ is 210 kPa , and the pressure at point B is 180 kPa . The elevation at A is 90 cm higher than point $B$ and the two points are separated by 91.9 meters of pipeline. Assume no minor losses and water @ $10^{\circ} \mathrm{C}$.

## Solution:

$\gamma=9800 \mathrm{~N} / \mathrm{m}^{3} ; v=1.306 \times 10^{-6} \mathrm{~m}^{2} / \mathrm{sec} ; \mathrm{e}=0.0015 \mathrm{~mm} ; \mathrm{h}_{\mathrm{L}}=\mathrm{h}_{\mathrm{f}}=3.96 \mathrm{~m}$;
Final $f=0.022 ; V=1.24 \mathrm{~m} / \mathrm{sec} ; Q=1.56 \mathrm{~L} / \mathrm{sec}$

